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Abstract: To understand the hydraulic vibration characteristics in a traditional hydropower system and identify possible exciting sources that may induce serious hydraulic vibrations in the flow passage, experimental tests and numerical calculations were conducted for different operating conditions. The experimental results show that the pressure fluctuations are mainly related to the vortex rope phenomena in the draft tube, and the dominant frequency of pressure fluctuation is $0.2 \sim 0.4$ times the runner rotational frequency (f_n). The numerical results show all the attenuating factors are negative, which indicates the system itself is stable on the condition that all the hydraulic elements have steady operating performance. The free vibration analyses confirm that the frequency range of the vortex rope in the draft tube partly overlaps the natural frequencies of the hydropower system. Apart from the vortex rope, the runner rotational frequency is another common frequency that is approximately equal to the frequency of the 10th vibration mode. From the vibration mode shapes, it is inferred that a small disturbance in its frequency close or equal to a specific natural frequency of the vibration mode could induce large pressure oscillations in the tail tunnel. In light of the system's response to different forcing frequencies, the vortex rope formed under offdesign conditions and runner rotational frequency is verified to be the potential exciting source of a traditional hydropower system, and the frequency $0.2 f_n$ is much more dangerous than other disturbances to the system.

Keywords: hydraulic vibration; pressure fluctuation; hydropower system; vortex rope

1. Introduction

Nowadays, the increasing global demands for electricity and environmental protections are gradually bringing about the transformation of energy structure such that clean and renewable energy covers an increasing proportion of energy consumption [1]. In this aspect, hydropower plays an irreplaceable role in the clean and renewable energy system, and attracts much attention [2]. Hydraulic turbines are designed for working at the rated head and rated discharge, which is defined as the best efficiency point (BEP), while the current tendency for hydropower plants is that they are undertaking increasing power frequency regulation tasks in the electric grid, and turbines are required to work at off-design conditions more than before [3]. Vibrations exist in various fields, including hydropower stations, and vibration mitigations have been extensively investigated by researchers [4,5]. Recently, more hydraulic vibration phenomena in hydropower systems have been reported accompanied by obvious pressure oscillations along with their hydro-mechanical systems, and even severe accidents, such as local structural damages and power swings, have happened [6].

When turbines operate under off-design conditions, pressure fluctuations due to rotorstator interaction (RSI), rotating stall, vortex rope, and other flow instabilities might be



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Copyright: © 2021 by the authors. Licensee MDPI, Basel, Switzerland. This article is an open access article distributed under the terms and conditions of the Creative Commons Attribution (CC BY) license (https:// creativecommons.org/licenses/by/ 4.0/). induced, and accompanying vibration phenomena may be generated [7,8]. Numerous studies, including model experimental tests and numerical simulations, have been conducted to investigate the pressure fluctuations under various off-design conditions. With an overall literature review, it was pointed out that pressure fluctuations in the turbine caused by transient processes would result in fatigue development in the runner, and then shorten the life span of the runner [9]. Based on CFD simulations and validated by experiments, rotor-stator interactions (RSI) were reported to be the primary cause of pressure fluctuation in the vaneless space, and the geometric and operating parameters of the unit were the main affecting factors [10–12]. Moreover, vortex rope in the draft tube was an unavoidable problem that formed under off-design conditions, and this would cause low-frequency but high-amplitude pressure fluctuations in the draft tube that propagate upstream and downstream [13,14]. Qin et al. discovered that the Thoma number influenced not only pressure fluctuation amplitude but also the distribution of the frequency components of pressure fluctuation in the draft tube [15]. Yu et al. discovered that there is a close relation between vortex rope and cavitation in the draft, such tube that the cavitation increases the vortex production as well as the pressure fluctuation frequency induced by vortex rope [16]. Apart from traditional hydraulic turbines, for a pump-turbine operating under off-design conditions, especially in S-shaped regions, pressure fluctuation in the flow passage is also attracting increasing attention [17-19]. Through numerical simulations of a pump-turbine working in an off-design way, it has been reported that the low-frequency pressure fluctuations originate from the rotation of vortices in the draft tube; further, the number of runner blades had an impact on the dominant frequency of pressure fluctuations in the draft tube [20,21]. The vibration affected the security of the hydropower station, and various strategies used to suppress vortex rope oscillation in the turbine flow passage have been explored [22,23]. In view of the pressure fluctuation caused by the vortex rope under off-design conditions, a novel passive control method using an adjustable diaphragm is introduced in decelerating swirling flow; further, the water injection method was proposed to mitigate the pressure fluctuation and change the velocity field in the flow passage, and correlation between the water jet discharge and vortex rope has been investigated [24,25]. However, the experimental method is restricted owing to its high costs and security risk, and the numerical simulation, including the one-dimensional method of characteristics (1D-MOC) and three dimensional (3D) numerical simulations [26] based on computational fluid dynamics (CFD), sometimes requires plenty of computation time, since the process of convergence to a stable state is very slow for long-distance water conveyance systems [27,28].

Hydraulic vibration is a periodic hydraulic transient that exists in various water conveyance systems and may result in instabilities and local destructions [29,30]. With complex condition switches under frequency regulation tasks, both the initial and boundary conditions become extremely complicated, especially for off-design conditions. If frequencies of exciting sources are coincident with the natural frequencies of the hydropower system and continuous working, hydraulic resonance will inevitably develop, and severe pressure oscillations may occur. Consequently, intense flow-induced pressure oscillations may lead to strong vibrations and structural damages [31]. As hydraulic turbines are subjected to increasing off-design operating conditions, the resonance may originate from the vortex rope in the draft tube when its frequency is approximately close to the intrinsic frequency of the water oscillation [32]. However, the exciting sources of hydraulic vibration in hydropower systems have not yet been revealed, and it is imperative to have a clear understanding of the exciting sources of hydraulic vibration in hydropower systems so as to ensure safety and stability during the operating process.

In this paper, pressure fluctuations in the turbine are analyzed, and possible exciting sources that may induce hydraulic vibration in hydropower systems are identified. Firstly, experimental tests were carried out for three off-design conditions by utilizing a reduced scale model runner to monitor pressure fluctuation characteristics in the spiral case, vaneless space, draft tube cone, and draft tube elbow. Secondly, based on transfer matrix and hydraulic impedance methods, free vibration analyses were performed to assess the vibration characteristics, including natural frequencies with attenuating factors of each order, and corresponding vibration mode shapes. Finally, the pressure fluctuations were analyzed in both the time domain and the frequency domain. Then, by comparing the frequency characteristics of pressure fluctuations with natural frequencies of the hydropower system, the correlations between flow instabilities in the hydraulic turbine and hydraulic vibration were revealed, and possible exciting sources were identified.

2. Research Object Description

The research project in this paper involved two turbines with a capacity of 1015 MW per unit. This set up consists of two parallel water diversion penstocks and tail branches, a downstream surge tank, and a D-shaped tail tunnel with a maintenance tail gate shaft located next to the tail water. Figure 1 is a sketch of the plan layout and detailed division information of the hydropower system.



Figure 1. Sketch of the plan layout of the hydropower system.

Table 1 lists the details of each pipe segment, including length and equivalent diameter, in the hydropower system.

Tab	le 1	. E	Basic paran	neters of t	the p	ipe in	the	hyċ	lropowe	r system.
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No.	L(m)	D(m)	No.	L(m)	D(m)	No.	L(m)	D(m)
1	41.0	15.925	11	24.55	17.145	21	61.1	11.460
2	166.446	10.999	12	93.25	16.456	22	60.85	17.145
3	47.124	10.203	13	41.0	15.925	23	24.55	17.145
4	104.50	10.203	14	167.938	10.999	24	74.0	16.456
5	47.124	10.203	15	47.124	10.203	25	103.745	17.628
6	31.88	9.053	16	104.50	10.203	26	972.805	17.628
7	42.4	8.600	17	47.124	10.203	27	100.0	17.605
8	20.0	11.460	18	31.88	9.053	28	122.166	17.605
9	61.1	11.460	19	42.4	8.600	29	397.048	17.605
10	60.85	17.145	20	20.0	11.460			

The specific parameters of the prototype turbine are as follows: the rated output $P_r = 1015$ MW, the rated head $H_r = 202$ m, the rated discharge $Q_r = 545.49$ m³/s, and the rated rotational speed $n_r = 111.1$ r/min. The maximum and minimum head of the prototype turbine are $H_{max} = 243.1$ m and $H_{min} = 163.9$ m, respectively. The test rig consists of a spiral casing, a scaled model turbine runner with 15 blades, and a draft tube.

Figure 2 shows the comprehensive characteristic curves of the turbine. Here, Q_{11} and n_{11} stand for the unit discharge and unit speed, respectively, which are defined as $Q_{11} = Q/(D_1^2\sqrt{H})$ and $n_{11} = nD_1/\sqrt{H}$. From Figure 2, the optimal operating conditions for the turbine are guide vane opening $a = 18.8^\circ$, unit speed $n_{11} = 53.07$ r/min, unit flow rate $Q_{11} = 0.579$ m³/s, and peak efficiency = 95.07%.



Figure 2. Comprehensive characteristic curves of the turbine.

3. Materials and Mathematical Model

3.1. Governing Equations

For a single pressurized pipe, as shown in Figure 3, the simplified governing equations for the internal steady-oscillatory flow, including the momentum equation and continuity equation, can be deduced by simplifying differential equations of transient flow [33]. The two governing equations are as below,

$$\frac{\partial H(x,t)}{\partial x} + \frac{1}{gA} \frac{\partial Q(x,t)}{\partial t} + \frac{fQ^2(x,t)}{2gDA^2} = 0$$
(1)

$$\frac{\partial Q(x,t)}{\partial x} + \frac{gA}{a^2} \frac{\partial H(x,t)}{\partial t} = 0$$
⁽²⁾

where $H = \overline{H} + h$, $Q = \overline{Q} + q$, H is the instantaneous pressure head, Q is the instantaneous discharge, \overline{H} and \overline{Q} are the average value, and h and q are the fluctuation value from the average. x is length to the inlet, t is the time, g is the gravity acceleration, f is the friction factor, A is the cross-sectional area of the pipe, D is the diameter of the pipe, and a is the wave speed.



Figure 3. Single characteristic computing pipe.

By substituting the instantaneous items into Equations (1) and (2), and removing the average items, the following form of equations can be obtained and written as

$$\frac{\partial \Phi(x,t)}{\partial x} + B \frac{\partial \Phi(x,t)}{\partial t} + G \Phi(x,t) = 0$$
(3)

where $B = \begin{pmatrix} 0 & L \\ C & 0 \end{pmatrix}$, $G = \begin{pmatrix} 0 & R \\ 0 & 0 \end{pmatrix}$, $\Phi(x,t) = \begin{cases} h(x,t) \\ q(x,t) \end{cases}$, L = 1/(gA), $C = Ag/a^2$, $R = f\overline{Q}/(gDA^2)$, L, C, and R are defined as the hydraulic inductance, hydraulic capacitance, and hydraulic resistance of the fluid in the pressurized pipe, respectively.

Application of the Laplace transformation yields the following subsidiary equation,

$$\frac{d\hat{\Phi}(x,s)}{dx} + (Bs+G)\hat{\Phi}(x,s) = 0 \tag{4}$$

where $\hat{\Phi}(x,s) = \{\hat{h}(x,s) \ \hat{q}(x,s)\}^T$ is the transformation of $\Phi(x,t)$, $\hat{\Phi}(x,s) = \int_0^\infty \Phi(x,t)e^{-st}dt$. $s = \sigma + i\omega$, *s* is the complex frequency, σ is coefficient of attenuation, and ω is angular frequency.

Assuming the oscillatory pressure head and discharge at the upstream end are known, the complex oscillatory pressure head and discharge are expressed as

$$\left\{ \begin{array}{c} \hat{h}(x,s)\\ \hat{q}(x,s) \end{array} \right\} = \left(\begin{array}{c} \cosh\gamma x & -Z_{C}\sinh\gamma x\\ -\frac{1}{Z_{C}}\sinh\gamma x & \cosh\gamma x \end{array} \right) \left\{ \begin{array}{c} \hat{h}(0,s)\\ \hat{q}(0,s) \end{array} \right\}$$
(5)

where $Z_C = \gamma/(Cs)$ is the characteristic impedance of the pipe, $\gamma = \sqrt{Cs(R + sL)}$ is the complex propagation constant, and *x* is the length from the inlet.

3.2. Transfer Matrix and Hydraulic Impedance Methods

The transfer matrix is a square matrix that relates two state vectors at any point of the pipe by introducing matrix form. Hydraulic impedance is defined as the ratio of the complex head to the complex discharge at the same cross-section. Expressions of some commonly used hydraulic elements have already been established [33,34]. Generally, for various complicated pressurized water conveyance systems, hydraulic vibration analyses are realized by combining the transfer matrix and hydraulic impedance. Two kinds of matrices are commonly used.

3.2.1. Field Matrix

The field transfer matrix connects state vectors at adjacent cross-sections in the same pipe, and as previously deduced, the field matrix of a single pressurized pipe with a length of *l* is

$$F = \begin{pmatrix} \cosh \gamma l & -Z_C \sinh \gamma l \\ -\sinh \gamma l / Z_C & \cosh \gamma l \end{pmatrix}$$
(6)

3.2.2. Point Matrix

The point transfer matrix relates the left and right state vectors of local discontinuity, such as a junction, valve, turbine, etc. The point matrices of several common hydraulic elements are presented below.

For a series junction connecting two pipes, neglecting the minor losses and obeying the relationship $Q_{D1} = Q_{U2}$, $H_{D1} = H_{U2}$, the point matrix is,

$$P_S = \left(\begin{array}{cc} 1 & 0\\ 0 & 1 \end{array}\right) \tag{7}$$

The equations for a hydraulic machine operating at a fixed speed can be expressed in a simplified matrix form if there is no excitation pressure head and flow rate at the turbine point,

$$P_T = \left(\begin{array}{cc} 1 & -M\\ 0 & 1 \end{array}\right) \tag{8}$$

where *M* is the slope of the head–discharge curve, and the value of *M* is usually assumed to be a real constant for pumps and turbines if the opening of guide vanes remains constant in the transient regime.

For a throttled surge tank installed in the system shown in Figure 4, the equation of motion is applied to the fluid in the pipeline between the upstream and downstream ends, and the friction term is linearized while the mean flow conditions are subtracted.

$$h_u(t) - h_d(t) - 2kq(t) = \frac{l}{gA_s} \frac{dq(t)}{dt}$$
(9)

where k, A_s , and l are the head loss coefficient, cross-sectional area, and water depth in the surge tank, respectively.



Figure 4. Sketch of the throttled surge tank.

Based on the continuity equation $\frac{dh_d(t)}{dt} = \frac{q}{A_s}$, the Laplace transformation of h_d can be derived, $\hat{h}_d(s) = \frac{\hat{q}(s)}{sA}$. Applying the Laplace transformation to Equation (9), the impedance formula can

be wrtiten,

$$Z_{u} = \frac{h_{u}(s)}{\hat{q}(s)} = \frac{1}{sA_{s}} + 2k + \frac{sl}{gA_{s}}$$
(10)

Therefore, the matrix of a throttled surge tank is

$$P_{st} = \begin{pmatrix} 1 & 0\\ -\frac{1}{Z_u} & 1 \end{pmatrix}$$
(11)

For the parallel system in Figure 5 with no forcing function, the field matrix of path jis expressed as $[F]_j = \begin{bmatrix} e_{11} & e_{12} \\ e_{21} & e_{22} \end{bmatrix}_j$. Then, the overall field transfer matrix of the whole parallel system is

$$[F]_{\text{loop}} = \begin{bmatrix} \frac{\zeta}{\eta} & \frac{1}{\eta} \\ \frac{\zeta\xi}{\eta} - \eta & \frac{\zeta}{\eta} \end{bmatrix}$$
(12)

where
$$\eta$$
, ζ and ξ are $\eta = \sum_{j=1}^{n} \left(\frac{1}{e_{12}}\right)_{j}$, $\zeta = \sum_{j=1}^{n} \left(\frac{e_{11}}{e_{12}}\right)_{j}$, and $\xi = \sum_{j=1}^{n} \left(\frac{e_{22}}{e_{12}}\right)_{j}$, respectively.



Figure 5. Parallel system.

3.3. Hydraulic Vibration Analysis

3.3.1. Free Vibration Analysis

Free vibration is the residual oscillation in the absence of an external disturbance, and the aim of conducting free vibration analysis is to obtain the complex frequencies of a given system, as well as the corresponding vibration mode shapes. With some commonly used hydraulic elements, the characteristic equations of the whole pressurized water conveyance system can be derived, and the overall matrix that relates the boundary data at two terminal points of the system is expressed as

$$\left\{ \begin{array}{c} \hat{h}(l,s)\\ \hat{q}(l,s) \end{array} \right\} = \left[\begin{array}{c} u_{11} & u_{12}\\ u_{21} & u_{22} \end{array} \right] \left\{ \begin{array}{c} \hat{h}(0,s)\\ \hat{q}(0,s) \end{array} \right\}$$
(13)

For the condition wherein the upstream is a reservoir and the outlet is a tailwater, and water elevation remains constant, $H_U = 0$, $H_D = 0$, which means the impedance values at the inlet and outlet are both zero. Hence, the characteristic equation of the system is written as below,

$$\begin{cases} Z_D(s) = u_{12}/u_{22} = 0 \\ u_{12} = 0 \end{cases}$$
(14)

By solving Equation (14), we can obtain the complex frequency s_k (k = 1, 2, 3, ...).

3.3.2. Forced Vibration Analysis

A periodic external disturbance that continues acting on a certain boundary, and which can generate steady oscillatory flow, is recognized as forced vibration. The purpose of forced vibration analysis is to investigate the system's response to a known forcing function that existed all the time. In a fully developed forced vibration, the whole system oscillates with the frequency of the forcing function, and the complex frequency only contains an imaginary part, $s = i\omega_k$, the attenuate factor, while $\sigma = 0$ means the vibration amplitude was independent of time.

3.4. Mathematical Model of Pressurized Flow in the System

For the hydropower system shown in Figure 1, based on the transfer matrix and hydraulic impedance method, the overall transfer matrix of the entire system was built. The format of the overall transfer matrix connecting the oscillatory pressure head and discharge at the upstream and downstream end of the system is,

$$U = \begin{bmatrix} u_{11} & u_{12} \\ u_{21} & u_{22} \end{bmatrix} = [F]_{29}[P]_{wz}[F]_{28}[F]_{27}[F]_{26}[F]_{25}[P]_{st}[F]_{loop}$$
(15)

where $[F]_i$ (i = 25, 26, 27, 28, 29) is the field matrix of the pipe numbered i in the main tail tunnel, $[P]_{wz}$ is the point matrix of the gate shaft, $[P]_{st}$ is the point matrix of the surge



tank located at the branch, and $[F]_{loop}$ is the field matrix of two parallel branches. The corresponding hydraulic vibration analysis process is presented in Figure 6.

Figure 6. Hydraulic vibration analysis method.

4. Results and Discussions

In the experiment, twelve pressure-monitoring points, designated as CH0~CH12, were evenly arranged in the flow passage as shown in Figure 7. In detail, CH0 and CH1 were located in the spiral casing, CH2~CH5 were located in the vaneless space, CH6~CH9 were located in the draft tube cone, and CH10~CH11 were located in the draft tube elbow.



Figure 7. Locations of pressure-monitoring points.

The fluctuation peak $\Delta H/H$ was calculated by processing the pressure signal at the confidence level of 97%, and its definition is

$$\frac{\Delta H}{H} = \frac{p_i - \overline{p}}{1000\rho_g H} \times 100\% \tag{16}$$

where *H* is the rated head of the turbine, p_i is the instantaneous pressure, and \overline{p} is the average pressure.

Three off-design operating conditions are selected in this paper to study the regularity of the pressure fluctuation. Details of operating points for the model runner are listed in Table 2.

Table 2. Details of operating conditions.

Mode	a (°)	<i>n</i> ₁₁ (r/min)	Q ₁₁ (L/s)	Н _т (m)	P (MW)	Pr (%Pt)
S01	7.88	60.21	145.99	29.91	301.11	29.67
S02	12.63	60.15	249.77	30.32	607.72	59.87
S03	14.12	73.16	260.64	29.88	346.43	48.32

4.1. Pressure Fluctuation in the Flow Passage

The pressure fluctuations at points CH0~CH11 under condition S01, S02, and S03 were monitored and recorded. Table 3 lists the pressure fluctuation peaks in the time domain at points CH0~CH11, and it is clear that the pressure fluctuations in the spiral case are relatively small compared to other points. The corresponding dominant frequencies are listed in Table 4, and $0.2f_n$ is the frequency with the greatest number of occurrences in the vaneless space and draft tube. Consequently, the pressure fluctuations in the vaneless space and draft tube are analyzed in the following section.

 Table 3. Pressure fluctuation peaks in time domain under different operating conditions.

Mode	CH0	CH1	CH2	CH3	CH4	CH5	CH6	CH7	CH8	CH9	CH10	CH11
S01	1.88	1.86	1.84	1.78	1.51	1.74	3.95	3.53	3.12	3.13	1.81	2.86
S02	1.91	1.88	2.67	3.02	2.14	2.54	2.68	2.86	2.63	3.36	1.54	2.28
S03	1.27	1.25	2.60	2.50	1.71	2.38	4.34	3.69	3.78	4.33	2.28	4.14

Table 4. f/f_n in frequency domain at different monitoring points.

Mode	CH0	CH1	CH2	CH3	CH4	CH5	CH6	CH7	CH8	CH9	CH10	CH11
S01	0.1	0.1	15	15	0.2	0.2	1.6	1.5	0.2	0.2	0.2	0.2
S02	0.0	0.1	0.2	0.2	1.0	0.2	0.2	0.2	0.4	0.4	0.2	0.2
S03	0.0	0.0	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2

4.1.1. Pressure Fluctuation in Vaneless Space

Figure 8 shows the pressure fluctuation characteristics in the vaneless space (CH2 and CH4). The time domain characteristics show that the fluctuation peak monitored under mode S02 was the largest compared to S01 and S03. The frequency domain characteristics indicate that under mode S01 with smaller guide vane openings, a blade frequency of $15f_n$ was manifested, caused by rotor–stator interaction, and with guide vane openings increasing, the runner rotational frequency reached $1.0f_n$ at point CH4 under S02. A frequency of $0.2f_n$, affected by vortex rope in the draft tube, persists in all operating modes.

4.1.2. Pressure Fluctuation in the Draft Tube

When turbines work under off-design conditions, the rotational velocity component of fluid in the runner outlet will probably cause vortex rope accompanied by low-frequency and large-amplitude pressure fluctuations in the draft tube. According to the empirical formula, the estimated frequency range of vortex rope is $f = (0.167 \sim 0.5) f_n$, and the frequency of $0.2 \sim 0.4 f_n$ captured in the experiment is in the range of the estimated frequency. Figure 9 shows the pressure fluctuations in the draft tube (CH6) under S01, S02, and S03. The time domain characteristics illustrate that the pressure fluctuation in the draft tube cone is the highest compared to other regions. From the frequency domain characteristics, we can infer that $0.2 f_n$ is the leading frequency that occurred in all conditions. Additionally, frequencies of $1.6 f_n$ and $1.5 f_n$, measured at the inlet of the draft tube only, occurred in S01, and these may propagate from the upstream region affected by RSI.



Figure 8. Pressure fluctuations in vaneless space.

4.2. Numerical Computation

4.2.1. Natural Frequencies

On the derivation of the mathematical model used for hydraulic vibration analysis, as mentioned above, further free vibration analyses were performed. The complex frequency mainly depends on the length of the system and the wave speed; as a result, for a traditional pressurized hydropower system, the hydraulic vibration characteristics can be revealed at a typical designed operating point. For this research project, the boundary conditions were $H_u = 806.8$ m and $H_d = 597.42$ m, and both turbines worked at the rated head and discharge. Then, by solving the characteristic Equation (14), the complex frequencies were calculated and are listed in the Case 1 column in Table 5. Taking the effect of wave speed on numerical computation into consideration, the column Case 2 lists the results by adding a 10% increase to the wave speed.

The data in Table 5 show that the frequencies of the first three orders have few relationships to wave speed, whereas for orders higher than 3, the frequencies are affected by the wave speed distinctly, and with increasing orders, the deviation between two frequencies corresponding to the same order becomes obvious. For the frequency 7.4851 of 8 order in Case 1 is approximately equal to the frequency of 7 order in Case 2 if ignoring the difference after the decimal point, so it is unnecessary to perform free vibration analysis on higher orders because of the clearly error induced by value of wave speed, and herein, only frequencies lower than f_n should be reserved in Table 5. Further, all attenuating factors in Table 5 are negative, which means that all vibration modes would attenuate with time until a steady state is achieved, and the possibility of self-excited resonance can be excluded.



Figure 9. Pressure fluctuations in the draft tube.

Table 5. Complex frequencies of the hydropower system.

Order kth	Cas	se 1	Cas	e 2
Oldel kui	σ	ω	σ	ω
1	-0.0046	0.0284	-0.0046	0.0284
2	-0.0048	0.2597	-0.0048	0.2599
3	-0.0025	0.3640	-0.0025	0.3640
4	-0.0038	2.3428	-0.0038	2.6017
5	-0.0036	4.6627	-0.0036	5.1799
6	-0.3048	5.5526	-0.3041	6.1735
7	-0.0044	6.8170	-0.0044	7.5732
8	-0.0055	7.4851	-0.0055	8.3165
9	-0.0265	9.4424	-0.0276	10.4924
10	-0.0034	11.6914	-0.0034	12.9902

4.2.2. Mode Shapes

The vibration mode shapes are calculated with an assumed initial oscillatory discharge value $Q_U = 1.0 \text{ m}^3/\text{s}$ at the upstream end. The rotational frequency of the prototype

runner is $f_n = 1.852$ Hz, and $\omega = 11.63$ rad/s. According to the empirical formula, the estimated frequency range of vortex rope was $f = (0.167 \sim 0.5) f_n = 0.309 \sim 0.926$ Hz, and $\omega = 1.94 \sim 5.82$ rad/s. Based on the above analysis, only orders whose angular frequency is in the range of vortex rope frequency (4th, 5th, 6th) and close to the runner rotational frequency (10th) are selected for plotting in Figure 10, including oscillatory discharge and oscillatory pressure head, and the abscissa is the distance from the inlet, while the black line is the head and the red line is the flow rate. For frequencies approximately equal to those of the 4th, 5th, and 10th orders, a small disturbance could cause intense pressure oscillation in the tail tunnel. However, for the sixth order, there was no obvious pressure fluctuation is at a minimum at the node and a maximum at the antinode. The locations of the node and antinode of the flow are opposite to those in the head. Since the upstream and downstream are reservoirs with a constant water level, the oscillatory head is zero at both ends and in all vibration modes.



Figure 10. Vibration mode shapes.

4.3. Comparative Analysis

The estimated frequency range of the vortex rope overlaps the 4th, 5th, and 6th orders, and the rotational frequency of the prototype runner is close to the frequency of the 10th mode. For the prototype turbine, the frequencies of $0.2 \sim 0.4 f_n$ measured for the vortex rope and the runner rotational frequency are emphasized here. Forced vibration was performed at three frequencies ($0.2f_n$, $0.4f_n$, and f_n) of oscillating discharge at the turbine point as the forcing function, with an expression $q' = 0.2 \sin \omega t$, and the system's responses are shown in Figure 11.

It is shown in Figure 11 that if the frequency of disturbance close is to the 4th, 5th, or 10th mode, a small disturbance would cause intense pressure oscillation in the tail tunnel, and the response to $0.2f_n$ is the highest. On the contrary, the response to the runner rotational frequency is the lowest. It is clear that the oscillatory crest value of both pressure and discharge decreases as the disturbance's frequency increases, which indicates

that the lower frequency vibration is more severe and should be avoided, because severe pressure oscillation can burst or collapse the pipe due to pressure in excess of the designed pressure. The pressure fluctuation in the tail tunnel shown in Figure 11 is much higher than that in the upstream part of the system. For the blade frequency of $15f_n$ with an extremely short exciting period, it is unnecessary to carry out targeted analyses, since the natural frequency of the higher order is not exact owing to the error in the estimated wave speed, and the corresponding higher-order vibration usually manifests as energy dissipation. It is confirmed that the leading frequencies of the vortex rope and the runner rotational frequency are closely related to natural frequencies, which may induce huge pressure fluctuations and even resonance along the water conveyance line. Under the actual operating conditions, real-time monitoring should concentrate on the frequency characteristics of vortex rope in the draft tube and pressure fluctuation in the vaneless space, in case these equal the natural frequency of the system.



Figure 11. System's response to the forcing function.

5. Conclusions

In this paper, the pressure fluctuations in a reduced-scale model turbine test rig under three sets of off-design conditions are tested. According to the time domain and frequency domain analysis, the leading frequencies of pressure pulsation throughout the flow passage are obtained. Then, the natural frequencies are calculated, and the vibration mode shapes corresponding to various frequencies are revealed for the whole hydraulic system, based on detailed free vibration analysis. It was found that the leading frequencies of the vortex rope in the draft tube partially overlap the natural frequencies, including the fourth, fifth, and sixth modes. It is concluded from the forced vibration analysis that if the frequency of the vortex rope is close to these modes and continues acting as a forcing disturbance, intense pressure fluctuation inevitably occurs, and a frequency of $0.2f_n$ in the vortex rope is the most dangerous disturbance, as this will cause huge pressure fluctuations in the water conveyance line. Besides this, the runner rotational frequency cannot be ignored either, as this may cause severe pressure fluctuations in the tail tunnel. According to the computation results, when the disturbance frequencies are similar to certain natural frequencies, vibration mitigations actions should be taken during the operating stage.

Hydraulic vibration analysis can provide a reference to recognize disturbances during the design stage in order to avoid severe pressure fluctuation, and even resonance, during the operation stage. A future study will focus on the exploration of vibration reduction methods, and on exactly identifying the disturbance by decomposing the pressure signal in the flow passage into synchronous and asynchronous parts. The study will also focus on the wide application of hydraulic vibration theory to various kinds of water conveyance systems.

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