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Correlating Natural, Dry, and Saturated Ultrasonic Pulse Velocities with the Mechanical Properties of Rock for Various Sample Diameters

Hasan Arman *  and Safwan Paramban

Geology Department, College of Science, United Arab Emirates University, Al Ain P.O. Box 15551, UAE;
p.safwan@uaeu.ac.ae

* Correspondence: harman@uaeu.ac.ae; Tel.: +971-50-112-1905

Received: 26 October 2020; Accepted: 26 November 2020; Published: 21 December 2020



Featured Application: The ultrasonic pulse velocity test is a widely used, inexpensive, nondestructive, and modest test to define the dynamic properties of rocks in various geotechnical and mining engineering applications as well as in gas and oil explorations under various environmental conditions.

Abstract: P-wave velocity is employed in various fields of engineering to estimate the mechanical properties of rock, as its measurement is reliable, convenient, rapid, nondestructive, and economical. The present study aimed to (i) correlate natural, dry, and saturated P-wave velocities with the mechanical properties of limestone and (ii) investigate how the ultrasonic P-wave velocities and mechanical properties of limestone are affected by the sample diameter. This study reveals that P-wave velocities under different environmental conditions can be correlated with the mechanical properties of limestone. Further, the R-value variations with different P-wave velocities for a given sample diameter are (i) negligible in terms of the uniaxial compressive strength (UCS) excluding 63.2 mm, (ii) limited for the diametrical point load index (PLI^D) except for 53.9 mm, (iii) perceived in case of the axial point load index (PLI^A) for 47.7 mm, (iv) observed for the indirect tensile strength (ITS), but generally insignificant, and (v) detected in terms of Schmidt hammer value (SHV) except for 47.7 mm.

Keywords: correlation; ultrasonic pulse velocity; natural P-wave velocity; dry P-wave velocity; saturated P-wave velocity; mechanical property; sample diameter; limestone

1. Introduction

The ultrasonic pulse velocity (UPV) test is a convenient, rapid, and low-cost method for evaluating rock samples under different environmental conditions, both in the laboratory and in situ using portable equipment. Due to its nondestructive nature, simplicity, and reliability, the UPV test has been broadly accepted and used in several disciplines, such as civil engineering, mining, and geo-engineering. Several researchers have utilized this test to investigate a variety of phenomena and parameters, including microstructural material properties, grouting assessment, the efficiency of different applications, deformation, stress, weathering degrees in rock mass, detection of rock material variations, characterization of rock and rock mass, fracture zone extensions around underground openings, and the thermal conductivity of rocks [1–12]. Furthermore, ultrasonic propagation in fractured rock, geotechnical characterization of monument and building stones, and the relationships between the UPV and rock properties have been intensively studied and reported by various researchers [13–36]. These studies have established correlations between the UPV and rock properties for various types of rocks.

Rock seismic properties are greatly influenced by shape and grain size, rock type, porosity, density, pore water, anisotropy, temperature, clay content, confining pressure, weathering and alteration zones, bedding planes, joint properties (e.g., roughness), water, filling material, and strike and dip directions and angles [26,27,37]. The mechanical properties of rock must be defined under various environmental conditions for the planning and design of any civil, mining, or geotechnical application. Such works require great attention in sample preparation and testing, and test results must be reported within an acceptable accuracy level. The UPV test, which is a reliable and convenient technique, can be applied under various environmental conditions to predict the mechanical features of rock based on correlations. In addition, the expected effects of sample diameter on the ultrasonic P-wave velocity (V_p) and mechanical features of limestone can be investigated by using various sample diameters.

In the literature, few studies have directly correlated natural (V_p^n), dry (V_p^d), and saturated (V_p^s) ultrasonic P-wave velocities with the mechanical properties of rocks with various sample diameters. Kahraman [37] investigated the predictability of wet-rock V_p based on the dry-rock V_p on 41 various rock types. He reported that the wet-rock V_p can be assessed from the dry-rock V_p using derived equations. Karaman et al. [38] evaluated the possible influence of sample length on P-wave velocities for 200 dry and saturated core specimens of volcanic rocks and limestones. The authors found that the sample length strongly affected the P-wave velocities; moreover, they developed a method to determine the threshold specimen length of the studied rocks as 109 mm for limestones. Ercikdi et al. [39] examined the effect of core size on the dry and saturated UPV of limestone samples. They reported that the reduction in V_p and uniaxial compressive strength (UCS) might depend on the weakening of solid/solid interfaces between grains and variations in the chemical and mineralogical composition of limestone. A core length of 75 mm was found to be ideal for dry and saturated UPV tests. Chawre [40] conducted a correlative study on pulse wave velocities and quartz–mica schist properties. She noted that the physicomaterial properties of rocks may be associated with ultrasonic P- and S-wave velocities and suggested that proposed empirical equations be validated for a rock type from a project site before being used in designing and planning goals. Kumar et al. [41] assessed the geotechnical features of rocks in coalfields under dry, semi-saturated, and saturated environments using ultrasonic S- and P-wave measurements. Their study showed a strong correlation, which was verified by the coefficient values for correlations between the P-wave velocity and other geotechnical parameters under dry conditions compared with semi-saturated and saturated conditions. Gonzales et al. [42] reported on an analytical model to predict the UCS from V_p , density, and porosity for 13 saturated limestone specimens. They found that a multi-exponent model was appropriate for predicting the UCS and demonstrated that the UCS depends on V_p . However, a detailed study, which correlates natural, dry, and saturated P-wave velocities with the mechanical properties of rock, and investigates how the ultrasonic P-wave velocities and mechanical properties of rock are affected by the sample diameter, was missing in the field.

In the present paper, we aimed to correlate the natural (V_p^n), dry (V_p^d), and saturated (V_p^s) P-wave velocities with mechanical features such as the UCS, the axial and diametrical point load indices (PLI-Point Load Index, PLI^A and PLI^D), indirect tensile strength (ITS), and Schmidt hammer value (SHV) of limestone. Furthermore, we examined how the P-wave velocities and mechanical properties of limestone are affected by the sample diameter.

2. Materials and Methods

All rock blocks were collected from the outcrops of sedimentary rock sequences of limestone, Dammam Formation, Middle to Late Eocene, aged 49–34 Myr, at Jabel Hafit Mountain in Al Ain City. Each rock block was prudently inspected for macroscopic flaws such as fractures, cracks, bedding planes, and alteration zones in order to attain standard testing samples for laboratory studies. As the UPV is greatly affected by nonhomogeneous and anisotropic features of testing samples, this task is critical for rock block assortments. All rock blocks were kept under the laboratory condition (around 25 ± 1 °C) until sample preparation and testing.

To avoid natural occurrence effects, all test samples were inspected prior to testing to ensure that they were free of any macroscopic or visible defects such as fractures or cracks. For the UPV test, core samples with diameters of 24.9, 38.1, 47.7, 53.9 (NX size), and 63.2 mm and various lengths ranging from 78.9 to 108.3 mm were acquired from the selected rock blocks perpendicular to bedding plane using a core drill and trimming machine. For good coupling, both end surfaces of each core sample were smoothed for standard testing, and the diameter and length of all core samples were measured and recorded. A total of 190 P-wave velocity tests were performed on the prepared core samples, with 39 24.9-mm-diameter samples, 36 38.1-mm-diameter samples, 29 47.7-mm-diameter samples, 63 53.9-mm-diameter samples, and 23 63.2-mm-diameter samples.

An accurate P-wave measurement requires perfect matching between the transducers and the end surfaces of the test samples, which enables good coupling. The UPV tests were conducted on the core samples using a Pundit Lab (Proceq, PL02-002-0276 B0) pulse generator unit with two transducers (54 kHz, 38.6 mm diameter). Each sample was evaluated under different environmental conditions, i.e., natural, dry, and saturated conditions, according to the American Society for Testing Materials (ASTM) standard [43]. To define the natural P-wave velocities, V_p^n , 190 samples were tested under the assumption of in situ conditions. The dry P-wave velocities, V_p^d , of the samples were measured after the samples were dried in an oven for 12–16 h at 105 °C; subsequently, all samples were cooled under laboratory conditions for approximately 48 h. For the saturated P-wave velocity, V_p^s , the samples were saturated with distilled water for 48–50 h by soaking them in distilled water tank prior to P-wave velocity measurements.

As UPV tests are nondestructive, all samples used for UPV tests were further evaluated to determine the mechanical properties of limestone after being maintained under laboratory conditions for approximately 2 weeks. In total, 87 UCS, 50 PLI^A, 45 PLI^D, 102 ITS, and 190 SHV tests were performed following the suggested ASTM standards [44–47], (see Table 1, second column).

Table 1. Representative empirical relation and correlation coefficient values of mechanical properties and natural, dry, and saturated P-wave velocities of limestone samples with various diameters.

	Mechanical Property (Number of Samples)	Natural P-Wave Velocity (V_p^n)		Dry P-Wave Velocity (V_p^d)		Saturated P-Wave Velocity (V_p^s)	
		Empirical Relation	R-Value	Equation	R-Value	Equation	R-Value
Diameter = 24.9 (mm)	UCS (MPa) (28)	$UCS = 14.63V_p^n + 10.64$	0.23	$UCS = 19.98V_p^d - 17.45$	0.27	$UCS = 17.20V_p^s - 4.3$	0.25
	PLI ^D (MPa) (10)	$PLI^D = 1.92V_p^n - 5.08$	0.69	$PLI^D = 2.45V_p^d - 7.92$	0.71	$PLI^D = 2.32V_p^s - 7.1$	0.69
	PLI ^A (MPa) (19)	$PLI^A = 1.93V_p^n - 3$	0.51	$PLI^A = 2V_p^d - 3.16$	0.49	$PLI^A = 1.96V_p^s - 3.14$	0.52
	ITS (MPa) (25)	$ITS = -0.38V_p^n + 9.24$	-0.17	$ITS = -0.32V_p^d + 8.86$	-0.12	$ITS = -0.31V_p^s + 7.84$	-0.05
	SHV (N) (39)	$SHV = 2.79V_p^n + 36.19$	0.48	$SHV = 3.07V_p^d + 34.92$	0.47	$SHV = 3.13V_p^s + 34.43$	0.53
Diameter = 38.1 (mm)	UCS (MPa) (17)	$UCS = 28.37V_p^n - 45.14$	0.74	$UCS = 31.53V_p^d - 62.4$	0.77	$UCS = 31.27V_p^s - 59.5$	0.76
	PLI ^D (MPa) (7)	$PLI^D = 2.35V_p^n - 7.75$	0.97	$PLI^D = 2.33V_p^d - 7.47$	0.93	$PLI^D = 2.28V_p^s - 7.24$	0.98
	PLI ^A (MPa) (7)	$PLI^A = 1.31V_p^n - 1.04$	0.74	$PLI^A = 1.36V_p^d - 1.43$	0.83	$PLI^A = 1.6V_p^s - 2.58$	0.8
	ITS (MPa) (31)	$ITS = 2.18V_p^n - 4.4$	0.62	$ITS = 2.13V_p^d - 4.17$	0.62	$ITS = 2.49V_p^s - 6$	0.66
	SHV (N) (36)	$SHV = 3.23V_p^n + 26.8$	0.61	$SHV = 3.6V_p^d + 24.8$	0.65	$SHV = 3.5V_p^s + 25.47$	0.61
Diameter = 47.7 (mm)	UCS (MPa) (14)	$UCS = 49.7V_p^n - 171.8$	0.84	$UCS = 52V_p^d - 180.73$	0.83	$UCS = 52.68V_p^s - 188.73$	0.81
	PLI ^D (MPa) (3)	$PLI^D = 2.38V_p^n - 8.63$	0.91	$PLI^D = 2.036V_p^d - 6.45$	0.95	$PLI^D = 2.42V_p^s - 8.94$	0.95
	PLI ^A (MPa) (7)	$PLI^A = 2.26V_p^n - 5.05$	0.92	$PLI^A = -0.83V_p^d + 13.14$	-0.25	$PLI^A = 2.02V_p^s - 3.29$	0.82
	ITS (MPa) (11)	$ITS = 2.46V_p^n - 6.8$	0.65	$ITS = 2.77V_p^d - 8.08$	0.82	$ITS = 3.23V_p^s - 10.93$	0.78
	SHV (N) (29)	$SHV = 4.02V_p^n + 26.9$	0.68	$SHV = 2.04V_p^d + 38.04$	0.34	$SHV = 4.37V_p^s + 25.12$	0.67
Diameter = 53.9 (mm), NX	UCS (MPa) (22)	$UCS = 44.4V_p^n - 153$	0.76	$UCS = 51.17V_p^d - 184.72$	0.8	$UCS = 46.5V_p^s - 161.57$	0.79
	PLI ^D (MPa) (17)	$PLI^D = 1.18V_p^n - 1.80$	0.67	$PLI^D = 1.07V_p^d - 1.16$	0.58	$PLI^D = 1.3V_p^s - 2.46$	0.73
	PLI ^A (MPa) (13)	$PLI^A = 0.41V_p^n + 2.57$	0.21	$PLI^A = 0.27V_p^d + 3.26$	0.13	$PLI^A = 0.11V_p^s + 4.05$	0.1
	ITS (MPa) (24)	$ITS = 1.89V_p^n - 2.08$	0.62	$ITS = 2.45V_p^d - 4.97$	0.67	$ITS = 2.25V_p^s - 3.93$	0.69
	SHV (N) (63)	$SHV = 5.4V_p^n + 18.68$	0.55	$SHV = 6.7V_p^d + 12.34$	0.6	$SHV = 6.57V_p^s + 12.67$	0.66
Diameter = 63.2 (mm)	UCS (MPa) (6)	$UCS = 64.5V_p^n - 262$	0.95	$UCS = 4.58V_p^d + 67.46$	0.04	$UCS = 67.39V_p^s - 269.83$	0.87
	PLI ^D (MPa) (8)	$PLI^D = 0.87V_p^n - 270$	0.77	$PLI^D = 1.00V_p^d - 1.67$	0.8	$PLI^D = 0.84V_p^s - 0.88$	0.75
	PLI ^A (MPa) (4)	$PLI^A = -0.55V_p^n + 7.7$	-0.29	$PLI^A = -0.16V_p^d + 5.40$	-0.1	$PLI^A = 0.23V_p^s + 3.14$	0.13
	ITS (MPa) (11)	$ITS = 1.93V_p^n - 4.32$	0.83	$ITS = 2.04V_p^d - 4.77$	0.81	$ITS = 1.77V_p^s - 3.37$	0.73
	SHV (N) (23)	$SHV = 6.2V_p^n + 12.67$	0.71	$SHV = 5.55V_p^d + 16.13$	0.58	$SHV = 6.44V_p^s + 12.05$	0.68

V_p^n , V_p^d , V_p^s = natural, dry and saturated P-wave velocity, UCS = uniaxial compressive strength (MPa), PLI^D = point load index, diametrical (MPa), PLI^A = point load index, axial (MPa), ITS = indirect tensile strength (MPa), SHV = Schmidt hammer value (N).

3. Results and Discussion

Linear regression analyses were performed to assess the relationships between the ultrasonic P-wave velocities (V_p^n , V_p^d , and V_p^s) and mechanical properties (UCS, PLI^A , PLI^D , ITS, and SHV) of the limestone samples. The numbers and diameters of the rock samples for each test are given in Table 1. The best-fit line equation and the correlation coefficient, R, were also obtained, as displayed in Table 1.

In Figures 1–5, the best-fit lines for each data set are shown. In all cases, linear regression was considered as the foremost representative for the best-fit relationships. The P-wave velocity values were highly scattered with respect to all mechanical properties of limestone, as shown in Figures 1–5. This trend could be due to the anisotropic and nonhomogeneous nature of the rock samples and the influence of the sample diameter.

The relationships between the UCS and V_p^n , V_p^d , and V_p^s for various sample diameters are displayed in Figure 1a–c. The correlation coefficient values have a range of 0.04–0.95 (Table 1). The maximum R-value was 0.95, indicating a strong correlation between the UCS and V_p^n for a rock sample diameter of 63.2 mm. In contrast, no correlation was observed for V_p^d at the same diameter. This result was unexpected and may be due to either invisible natural features of the tested samples or human error during the experiments. In addition, the International Society for Rock Mechanics (ISRM) [48] standard states that the specimen diameter for the UCS test should not be less than the NX core size (approximately 54 mm). Therefore, the 53.9-mm sample diameter could be considered to provide the best correlation between the P-wave velocities and UCS.

Figure 2a–c displays the relationships between PLI^D and V_p^n , V_p^d , and V_p^s for different sample diameters. The R-values ranged from moderate to strong (0.58–0.98) (Table 1). In general, the correlations between PLI^D and the P-wave velocities were relatively strong. The maximum R-value was observed for PLI^D and V_p^s at a sample diameter of 38.1 mm.

The values of PLI^A are plotted as a function of V_p^n , V_p^d , and V_p^s for various sample diameters in Figure 3a–c. The correlations between PLI^A and the P-wave velocities exhibit strong variations, ranging from negative to positive (weak to strong) with R-values of -0.29 to 0.92 (Table 1). The maximum and minimum correlation coefficient (R) values were acquired for V_p^n and V_p^s at sample diameters of 47.7 and 63.2 mm, respectively.

For different sample diameters, the relationships between the ITS and V_p^n , V_p^d , and V_p^s are presented in Figure 4a–c. Except for the 24.9-mm diameter samples, the R-values indicated moderate to strong correlations, with a range of 0.62–0.83 (Table 1). The minimum and maximum R-values were observed for V_p^s and the V_p^n at sample diameters of 24.9 and 63.2 mm, respectively.

Figure 5a–c illustrates the relationships between the SHV and V_p^n , V_p^d , and V_p^s for various sample diameters. The R-values exhibited weak to strong correlations, ranging from 0.34 to 0.71 (Table 1). The maximum and minimum correlation values (R) were obtained for V_p^n and V_p^d at sample diameters of 63.2 and 47.7 mm, respectively.

The previous researchers commonly focused on correlations between the UPV and rock properties, mechanical and physical, for various types of rocks [13–36]. Only few researchers investigated the predictability of wet-rock V_p based on the dry-rock V_p , correlated the physicomaterial properties of rocks with ultrasonic P- and S-wave velocities, assessed some geotechnical parameters under various environmental conditions using ultrasonic S- and P-wave measurements, and suggested a multi-exponent model to predict the UCS for various rock types [37,40–42]. Moreover, limited researchers studied the effects of sample length on P-wave velocities of dry and saturated core samples. They reported that the sample length strongly affected the P-wave velocities and suggested 75 or 109 mm as the ideal or the threshold specimen length of core specimen for limestones. The variation in the suggested sample length and the threshold could be due to sample origin, chemical characterization, texture, etc., since samples were collected from different locations [38,39].

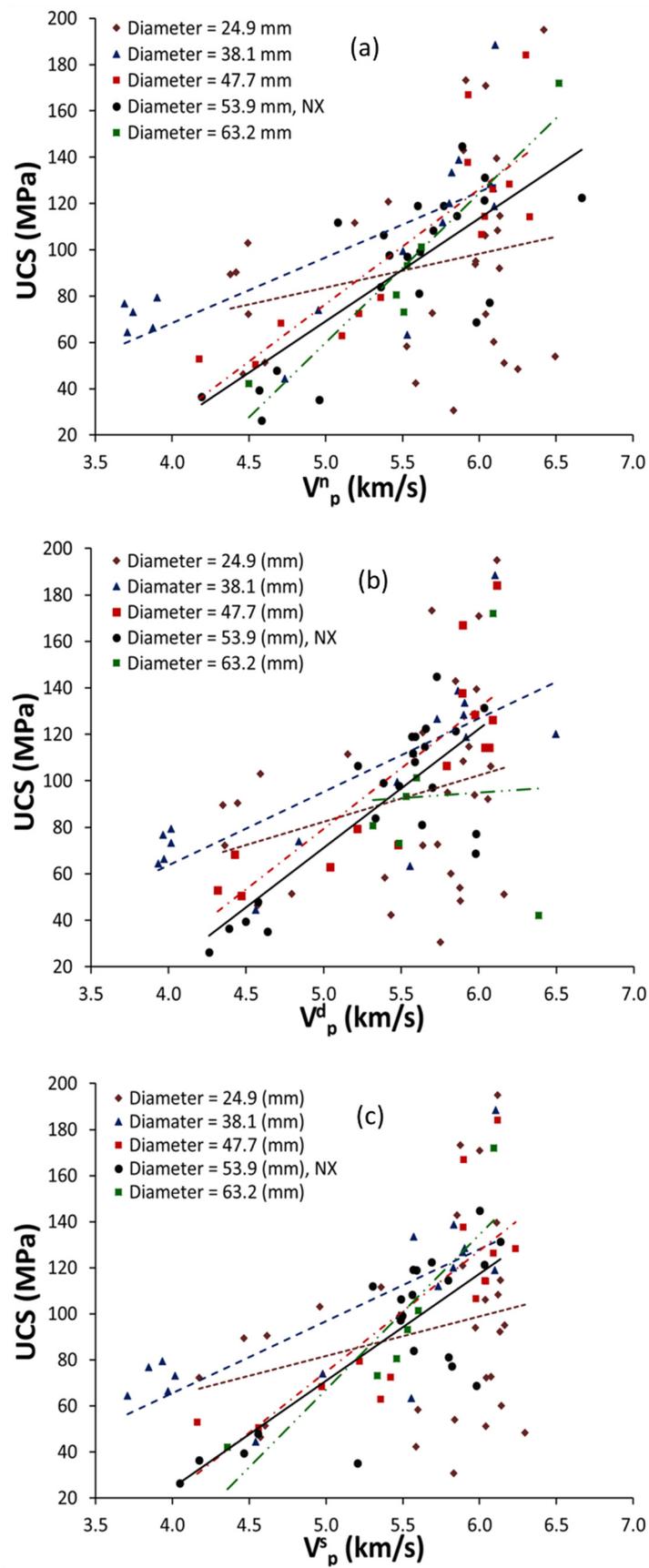


Figure 1. Variation in uniaxial compressive strength (UCS) values. UCS plotted as a function of (a) V_p^n , (b) V_p^d , and (c) V_p^s for different sample diameters.

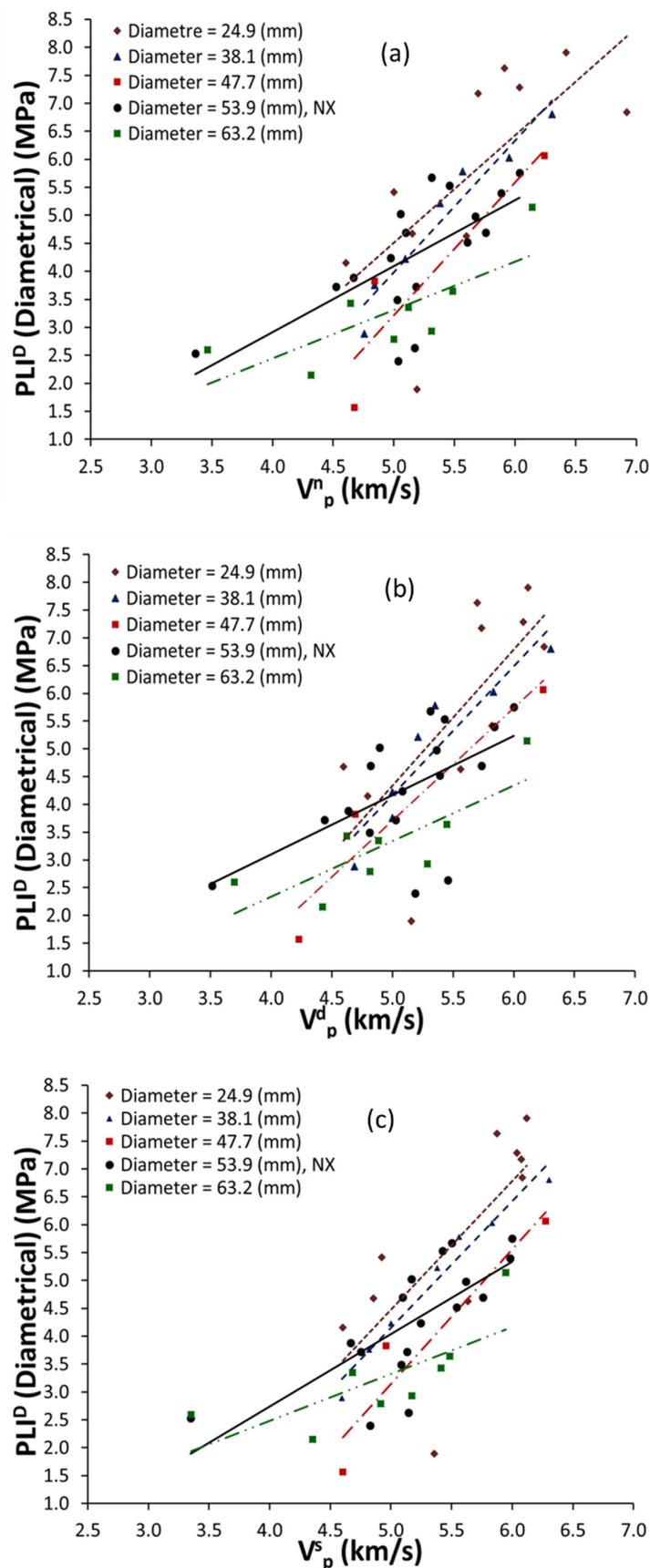


Figure 2. Variation in PLI^D values. PLI^D plotted as a function of (a) V_pⁿ, (b) V_p^d, and (c) V_p^s for different sample diameters.

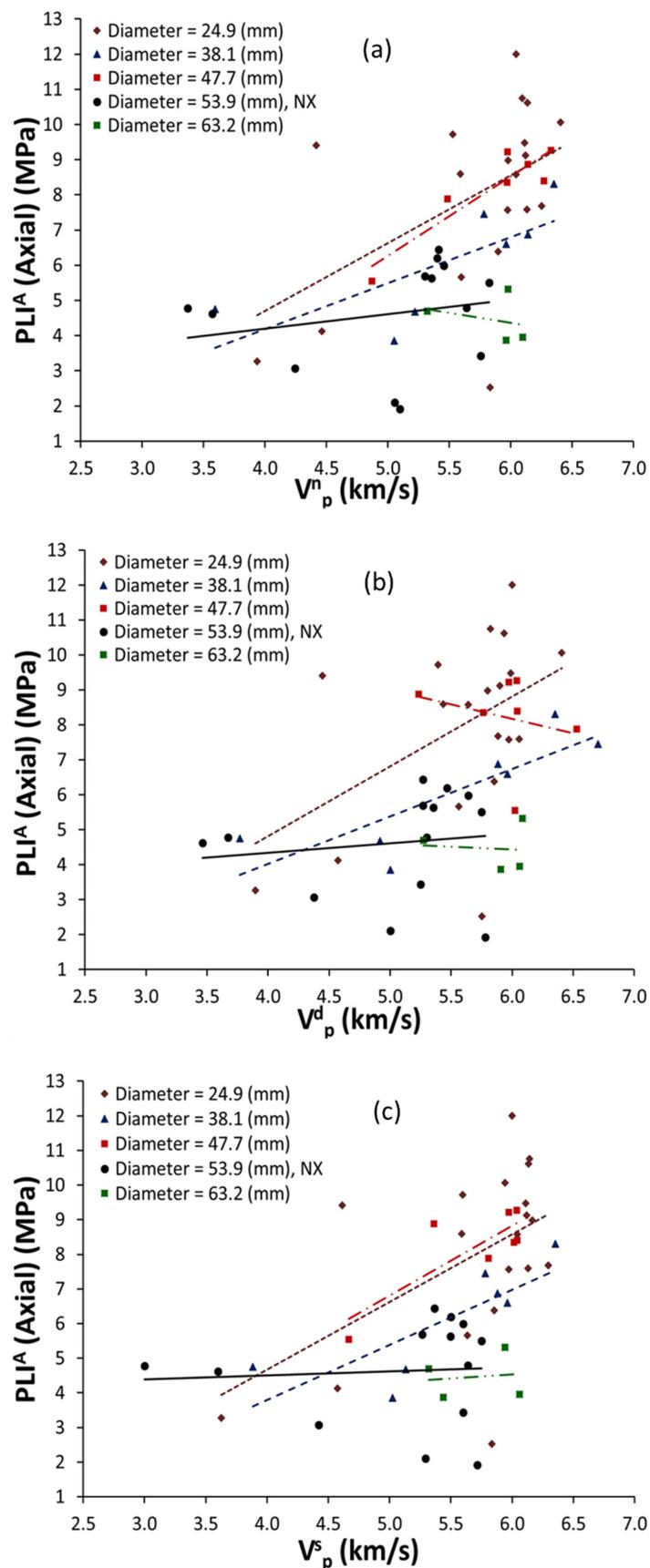


Figure 3. Variation in PLIA values. PLIA plotted as a function of (a) V_p^n , (b) V_p^d , and (c) V_p^s for different sample diameters.

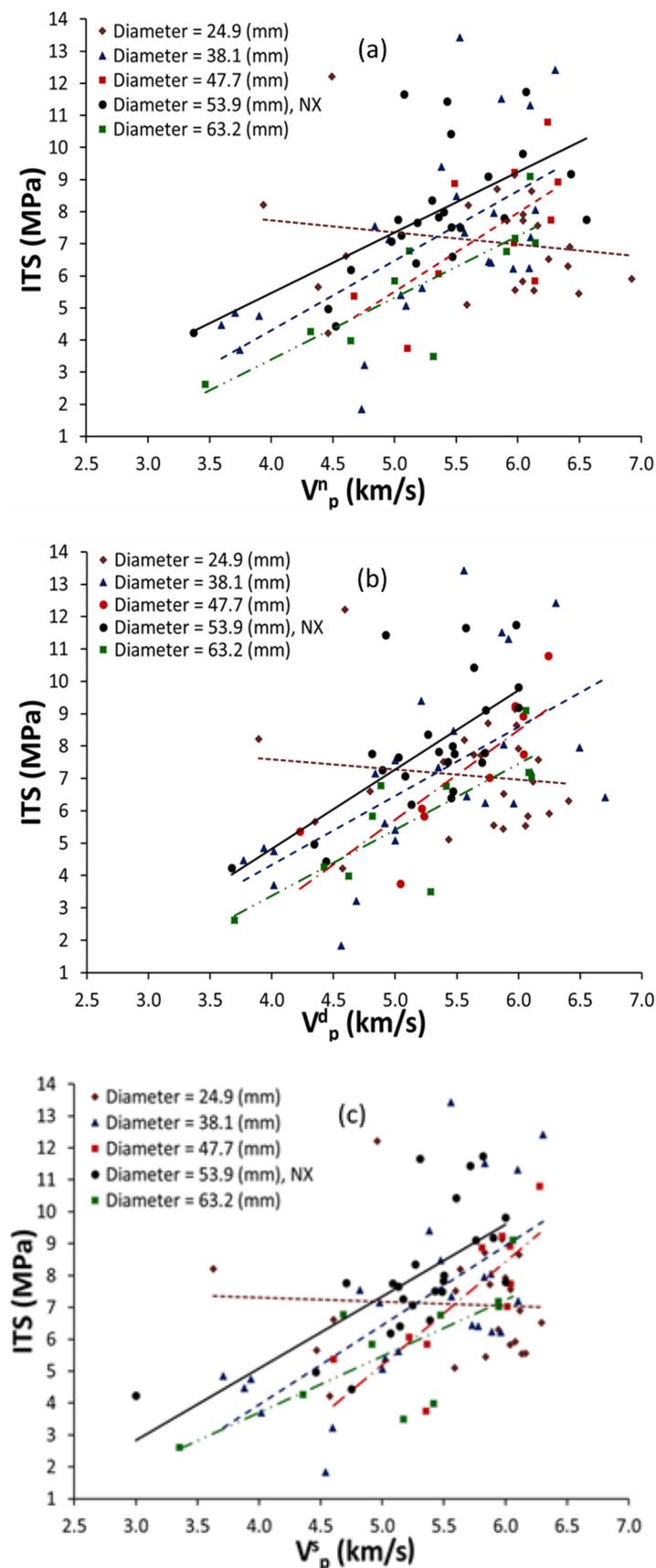


Figure 4. Variation in indirect tensile strength (ITS) values. ITS plotted as a function of (a) V_p^n , (b) V_p^d , and (c) V_p^s for different sample diameters.

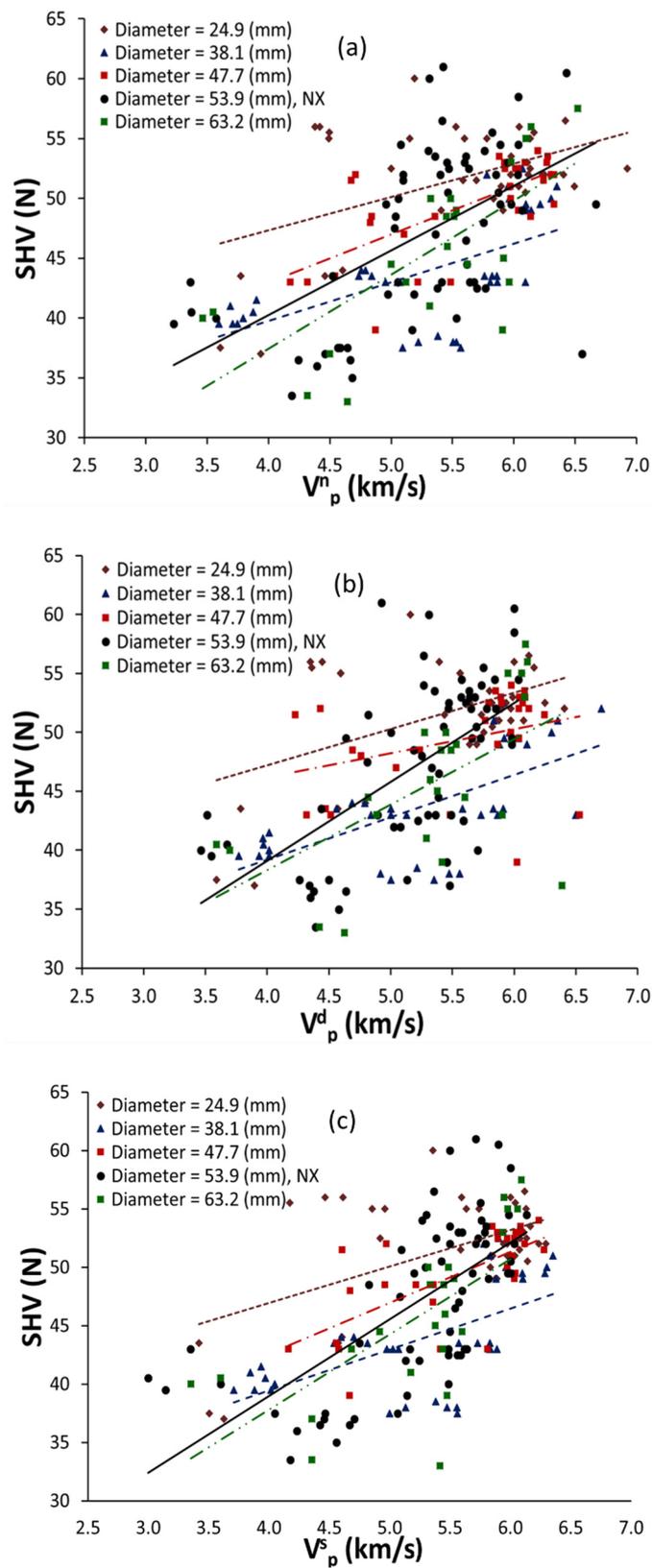


Figure 5. Variation in Schmidt hammer value (SHV). SHV plotted as a function (a) V_p^n , (b) V_p^d , and (c) V_p^s for different sample diameters.

In the present study, beside the correlation of the natural, dry, and saturated P-wave velocities with the mechanical properties of limestone, the effects of sample diameter on the ultrasonic P-wave velocities and mechanical properties of limestone, which were not addressed before in literature, were studied in detail. There are no similarities in terms of the main concept of this study with the previous investigations. As it was used in some previous studies, limestone was selected as a rock material for the UPV tests under various environmental conditions and mechanical tests due to availability and accessibility in the study area. Nevertheless, the study provides a new perspective especially on how the ultrasonic P-wave velocities and mechanical properties of limestone are affected by the core sample diameter.

4. Conclusions

In this study, P-wave velocities under natural, dry, and saturated conditions, as well as the UCS, PLI^A and PLI^D , the ITS, and the SHV, were measured for 190 limestone core samples with various sample dimensions. The P-wave velocities and the mechanical properties of limestone samples with various diameters were correlated by means of linear regression analyses, and the equation of the best-fit line and the correlation coefficient, R , were determined and interpreted. Furthermore, we investigated the effect of the sample diameter on the P-wave velocities and mechanical features of limestone. Based on the test results, the following conclusions were attained:

1. In general, a notable increasing trend of R -values for the UCS and P-wave velocities of limestone was observed with increasing sample diameter, with the exception of a correlation coefficient value of 0.04 for V_p^d and the 63.2-mm sample diameter. This result indicates that the UCS and P-wave velocities are affected by the sample diameter. Moreover, the variations in R -values with different P-wave velocities were negligible for a given sample diameter excluding 63.2-mm.
2. The R -values for the PLI^D and P-wave velocities of limestone showed fluctuations for various sample diameters. Nevertheless, an effect of sample diameter on the P-wave velocity and PLI^D was still visible. Furthermore, the variations in R -values for different P-wave velocities at a constant diameter were limited, except for the NX sample diameter.
3. Due to data fluctuations, there was no specific trend in R -value for the PLI^A and P-wave velocities of limestone for different sample diameters. Thus, it was difficult to interpret the direct influence of specimen diameter on the P-wave velocity and PLI^A . Moreover, variations in R -values for different P-wave velocities at a given diameter were observed, particularly for a sample diameter of 47.7 mm.
4. There was a notable trend in correlation coefficient values, R , for the ITS and P-wave velocities of limestone for various sample diameters, except for the 24.9-mm sample diameter. Thus, an influence of specimen diameter on the P-wave velocity and ITS was observed. In addition, variations in R -values for different P-wave velocities at a constant diameter were observed, but were generally insignificant.
5. An increasing trend of P -values for the SHV and P-wave velocities of limestone for different sample diameters was observed. Accordingly, the SHV and P-wave velocity were visibly affected by the sample diameter. Furthermore, variations in R -values for different P-wave velocities at a constant diameter were observed, except for the 47.7-mm-diameter samples.

Author Contributions: Conceptualization, methodology, investigation, data acquisition, validation, supervision, writing—original draft, writing—review and editing, H.A. and investigation, data acquisition, validation, S.P. Both authors have read and agreed to the published version of the manuscript.

Funding: This study was financially supported in part by the United Arab Emirates University, Research Affairs under a SURE 2016 grant.

Acknowledgments: The laboratory work was conducted at the Environmental and Engineering Laboratory at the Department of Geosciences, the United Arab Emirates University. The authors would like to thank the United Arab Emirates University, Research Affairs, for their generous support and encouragement.

Conflicts of Interest: The authors declare no conflict of interest.

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