

Article

Sustainable Restoration of Degraded Farm Land by the Sheet-Pipe System

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Abstract: For the sustainable restoration of wet farm land degraded by the climate change-induced rise of ground water level (GWL) and soil salinity etc., the sheet pipe system is one of the most useful technologies which reduces cultivation obstacles due to the poor drainage by controlling the rapid drainage function and enabling farmers to produce profitable crops. This system is characterized mainly as a perforated polyethylene rolled-band sheet 180 mm in width and 1 mm thick which is drawn into the subsurface layer while transforming a drainage pipe with $\phi = 50$ mm. The major advantage of this system is that since the sheet pipe is installed without trenching, the disturbance of land is minimized and the construction period can be shortened to about 1/4 (which reduces the cost approximately by 50%). In this study, by using the sheet pipe installed miniature-type model soil box, the drainage capacity of the sheet pipe was confirmed as being the same as the pipe-shaped standard drainage pipes. Based on the observations of the saturated–unsaturated flow and the maximum lowering rate of GWL was predicted. Finally, at the farm land wherein the free board of the adjoining canal was limited, the effectiveness of the sheet-pipe system was confirmed.

Keywords: sheet-pipe system; subsurface drainage; flow capacity; saturated-unsaturated flow; ground water level



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1. Introduction

For the sustainable restoration of farm land degraded by climate change-induced rise of ground water level (GWL) and soil salinity etc., the sheet pipe system is one of the most useful technologies. It reduces cultivation obstacles due to poor drainage by controlling the rapid drainage function and enables farmers to produce profitable crops [1–5]. Sheet-pipe-used drainage is also useful as a convenient way to improve the playgrounds, parks, golf courses, soft soil ground, etc.

Although subsurface drainage by the excavated mole drains is expensive and time consuming [6], the sheet-pipe system is characterized as another type of subsurface drainage in which the perforated high-density polyethylene rolled-band sheet with a width of 180 mm and a thickness of 1 mm (the length is 100 m/roll) is transformed to a drainage pipe with 50 mm in diameter just before being inserted into the subsurface soil layer by a ‘Mole Drainer’ (as shown in Figures 1 and 2). So, the advantage of this system is that since the sheet pipe is installed without trenching and backfilling, the disturbance of the land is minimized and the construction period can be shortened to about 1/4 of the conventional method (which reduces the cost approximately by 50%). During the last two decades, an abrupt climate change induced upward movement of water level in areas with shallow water tables and coastal areas with water intrusion has resulted in root zone salinity [2]. To cultivate problematic lands sustainably, a proper water management system is required which keeps water levels in a preferable range for optimum plant growth.

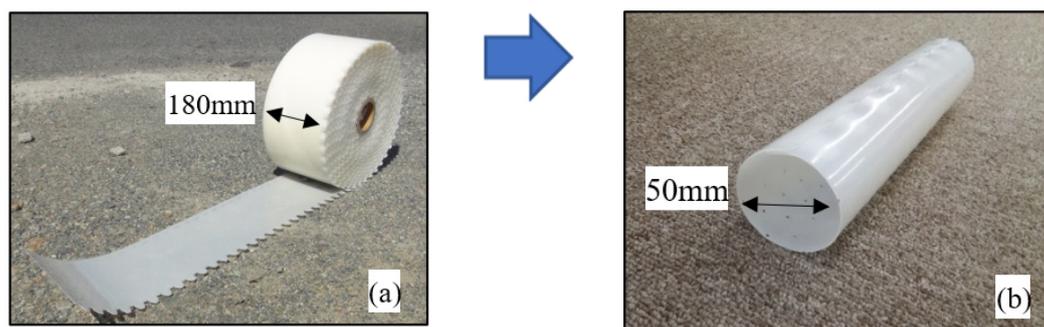


Figure 1. (a) Rolled-band sheet (100 m in length) with the perforated drainage holes, (b) sheet pipe transformed from the rolled-band sheet.

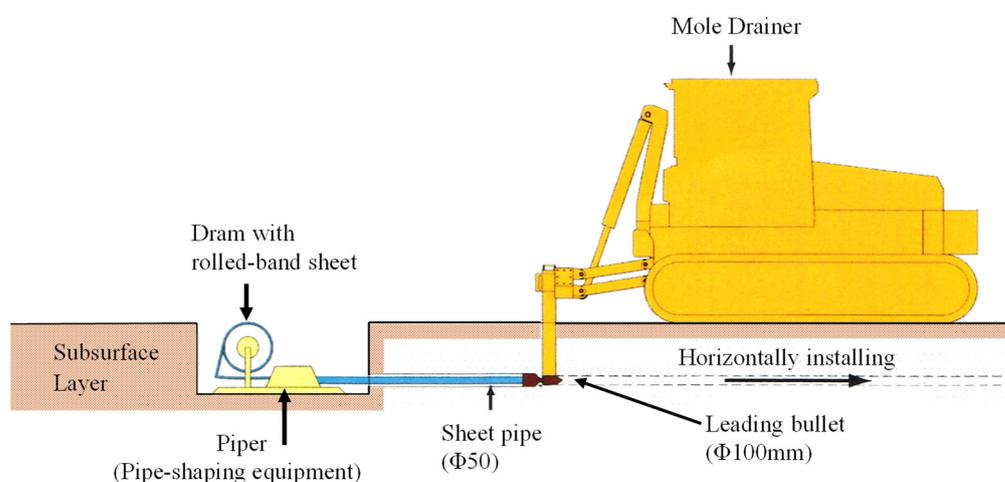


Figure 2. Process of insertion of the sheet pipe into the subsurface layer.

The subsurface drainage method by using sheet-pipe has been developed more than 40 years ago in Japan for the relatively smaller lot of farmlands with about 3000 m² as a standard size. After being certified as a new technology by the Japanese government in 2001, this method was gradually spread and adopted in the agricultural land improvement projects [4], and also by the observations at ten years after the installation of the sheet pipe, the deformation of pipe form and the significant clogging of the drainage holes were not confirmed [7]. Until now, however, and although the effectiveness of the sheet-pipe system was observed on the crop productivity in detail by measuring water depth, weather parameters such as solar radiation and temperature, etc. [8–12], reports on the differences of the flow capacity between the sheet-pipe and the standard slotted PVC or flexible pipes have been very limited [13] and the hydraulic properties of the sheet-pipe system have not been clarified satisfactorily, especially on the flow capacity of the sheet-pipe when installed horizontally in the ground.

In this study, to clarify the differences in the flow capacities between the sheet pipe and four other drainage pipes which are commercially supplied in Japan, miniature-type model soil box (hereinafter designated as MT-MSB), which is possible to measure not only the volume of water discharged through the sheet pipe but also the change of the hydraulic pressure in the soil layer, was newly developed [14], based on the assumption by Dupuit-Forchheimer (i.e., in a system of gravity flow toward a shallow sink, all of the flow is horizontal and that the velocity at each point is proportional to the slope of the water table but independent of depth) [15]. By the MT-MSB, the drainage capacity of the sheet pipe, the change of the water head, and the vertical distribution of matric suction were measured with time on natural fine sand (named Toyoura sand which is widely used in geotechnical engineering in Japan), and it is clarified that the flow capacity of the sheet pipe is almost the same as other drainage pipes and the storage coefficient was obtained.

Based on the experimental results, for the case that the sheet pipe is laid horizontally, the maximum lowering rate of GWL was predicted by the intake model. Finally, as a field case study, the effectiveness of the sheet-pipe system was investigated by focusing on the differences between the water level in the canal and GWL at a paddy field which includes both a horizontally installed sheet pipe area and no sheet pipe area.

2. Materials and Methods

To find the differences in the drainage capacity between the sheet pipe and four other typical drainage pipes, MT-MSB (as shown in Figure 3) was developed. The dimensions of MT-MSB are $0.4\text{ mW} \times 0.4\text{ mL} \times 0.4\text{ mH}$. For the assumption by Dupuit-Forchheimer as mentioned above, to satisfy the layer below the drainage pipe as impermeable, the drainage pipe was placed as close to the base plate as possible and through small holes on the base plate which were placed at 0.03 m (PP3), 0.10 m (PP2) and 0.17 m (PP1) from the center of the drainage pipe, the hydraulic pressures and water heads were measured by using the pressure transducers (PGM02KG) and manometers. As for the change of matric suction during the drainage test, the porous cup column was set horizontally at the depth 0.10 m , 0.20 m , and 0.30 m from the top surface of MT-MSB as shown in Figure 4 and the positive and negative pressures were measured by the pressure transducer (PGM1KG).

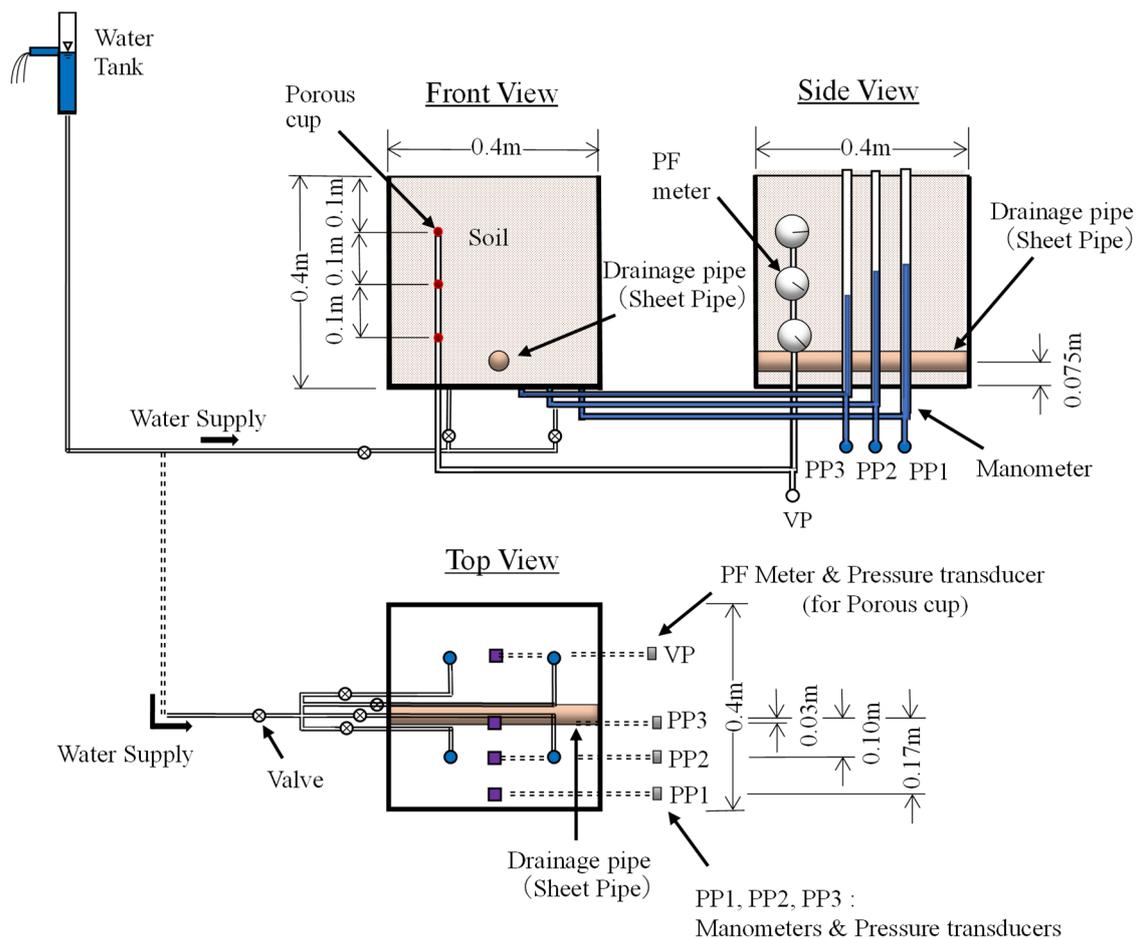


Figure 3. Outline of the miniature-type model soil box (MT-MSB).



Figure 4. Setup of the porous cup.

To fill the MT-MSB with soil sample and water, after deairing the predetermined weight of sample was poured into the MT-MSB with a sufficient depth of water step by step. To raise the water level in the MT-MSB, the water was supplied from the constant head water tank to the four small holes at the base plate in which porous stones are placed. MT-MSB filled with sand and water is shown in Figure 5. The volume of the water drained from the drainage pipe was measured with time by the volume-correlated load transducer (TCLA1KNB).

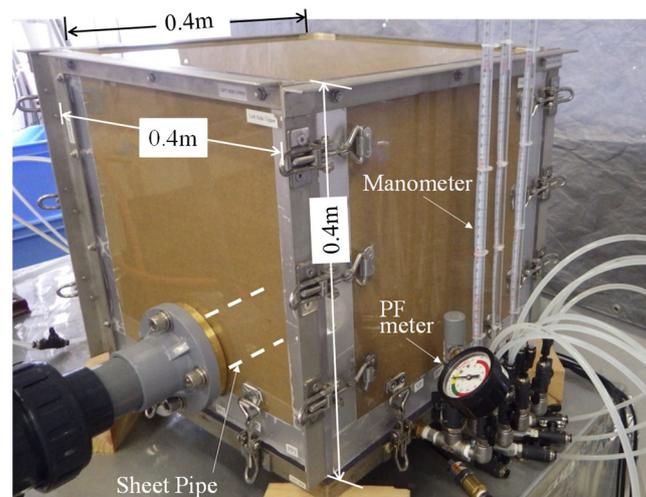
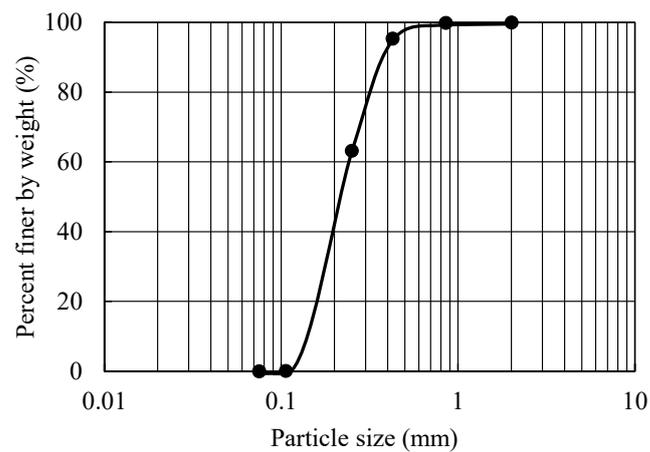


Figure 5. MT-MSB filled with sand and water.

Since one of the main objectives of this study is to investigate the maximum flow capacity of the drainage pipe, as a relatively higher permeable soil than farmland soil, Toyoura sand obtained in Yamaguchi prefecture, Japan was chosen, which has been widely used previously in geotechnical investigations [16–19]. Its physical properties, including the coefficient of permeability and the particle size distribution curve, are shown in Table 1 and Figure 6, respectively. In Table 1, ρ_s is the particle density, e_{\max} and e_{\min} are the maximum and minimum void ratio, k is the coefficient of permeability and D_r is the relative density.

Table 1. Physical properties of sample.

ρ_s (g/cm ³)	2.673
e_{\max}	0.991
e_{\min}	0.630
k (m/s) $D_r = 60\%$	2.246×10^{-4}
k (m/s) $D_r = 70\%$	1.700×10^{-4}
k (m/s) $D_r = 80\%$	1.629×10^{-4}

**Figure 6.** Particle size distribution curve.

To confirm the applicability of the sheet pipe, the flow capacity was compared with those of four other drainage pipes. In Table 2, dimensions of drainage pipes including the sheet pipe (designated as Pipe A or Sheet Pipe A) are shown for the outside diameter, hole size, pipe thickness, and length. In these drainage pipes, B is the perforated PVC pipe, C and D are the slotted PVC pipe and E is the hard polyethylene pipe.

Table 2. Dimensions of typical drainage pipes.

Drainage Pipe	Diameter (mm)	Drainage Hole Size (mm)	Thickness (mm)	Length (mm)
A (Sheet pipe)	50.5	$\varnothing 2$	1.0	400.0
B	60.0	$\varnothing 7$	4.1	400.0
C	59.6	8×1	5.0	400.0
D	60.4	-	-	400.0
E	54.0	$\varnothing 7$	2.0	400.0

In the drainage tests, all the drainage pipes shown in Table 2 were covered with the wire mesh filter as shown in Figure 7 in which the opening and the wire diameter are 0.109 mm and 0.06 mm, respectively. This is due to the fact that the size of the drainage holes for every pipe with the range from 2 to 8 mm is larger than the maximum particle size of Toyoura sand.

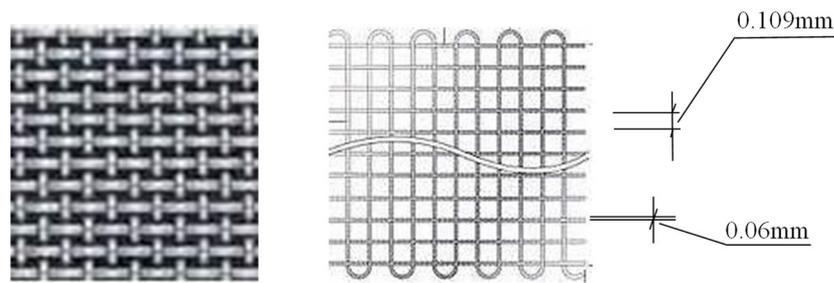


Figure 7. Wire mesh filter.

Thus, the effects of wire mesh filter on the flow capacity were confirmed by the constant head permeability tests and also by using MT-MSB as follows. Firstly, to measure the flow capacity only through the wire mesh filter, after placing the sheet pipe which was covered with the wire mesh, the sheet pipe was pulled out from the window on the side wall of the MT-MSB as shown in Figure 8a. Secondly, MT-MSB was filled with water as in Figure 8b and the drainage test was carried out. Thirdly, MT-MSB was filled with sand and water as shown in Figures 5 and 8c, then the water was drained out. Figure 8d shows the inside of the wire mesh filter after finishing the drainage test. It is seen that the shape is kept as those before the drainage and that no sand particles were confirmed.

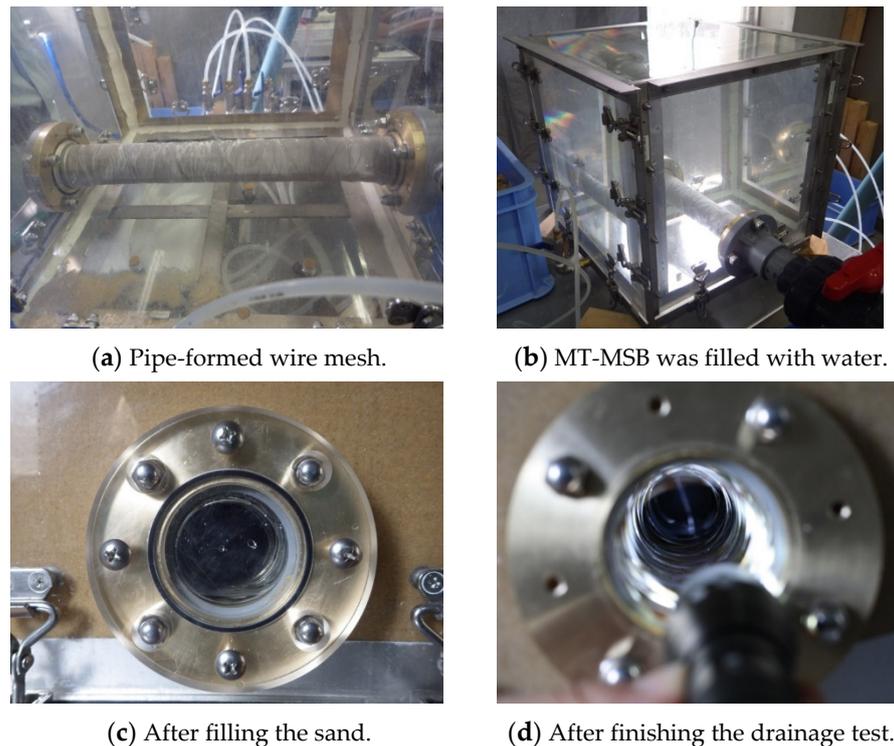


Figure 8. Setup of wire mesh filter in MT-MSB.

3. Results and Discussions

3.1. Flow Capacity of the Sheet Pipe

To compare the flow capacity of drainage pipes which are shown in Table 2, after placing the drainage pipe, a predetermined weight of Toyoura sand with a relative density $D_r = 60\%$ was poured into MT-MSB by the water pluviation method, and after initial settings of the data logging equipment, the drainage tests were started.

Figure 9 shows the relationships between the volume of water discharged from MT-MSB filled with only water and elapsed time. In the case of only mesh which is shown in Figure 8b, the volume of discharged water reaches $56 \times 10^{-3} \text{ m}^3$ at 100 s. For pipes C and

D, tendencies of water accumulation are similar to those of only mesh filter. Although the accumulations of the water for A, B, and E are delayed, Pipe A and E reach the same level as Pipe C and D at 220 s.

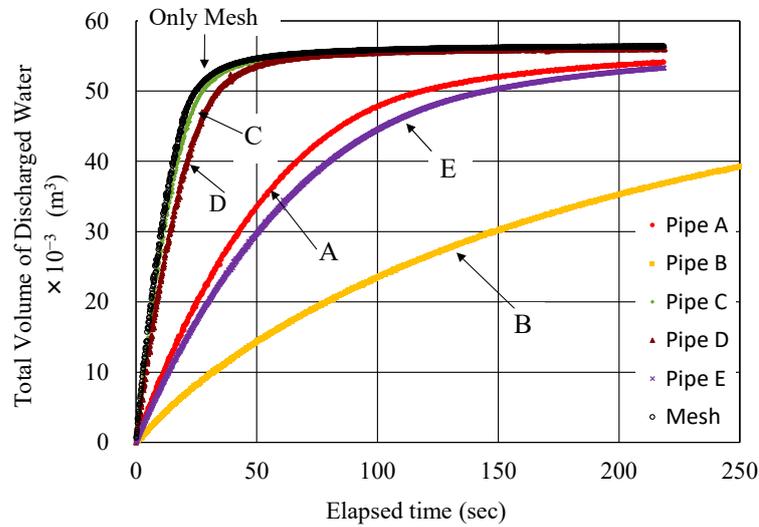


Figure 9. Drainage from MT-MSB filled with only water.

Figure 10 shows the results for the case that the MT-MSB was filled with sand and water. For all pipes except for pipe B, the volume of discharged water reaches the same level at 600 min.

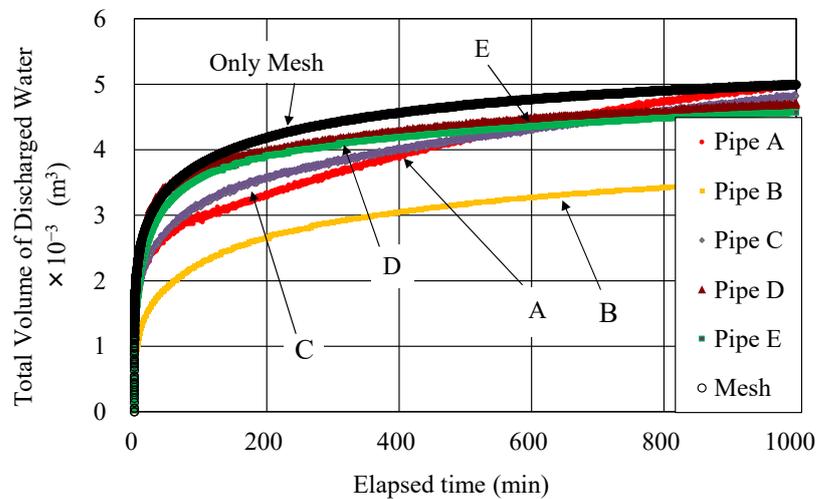


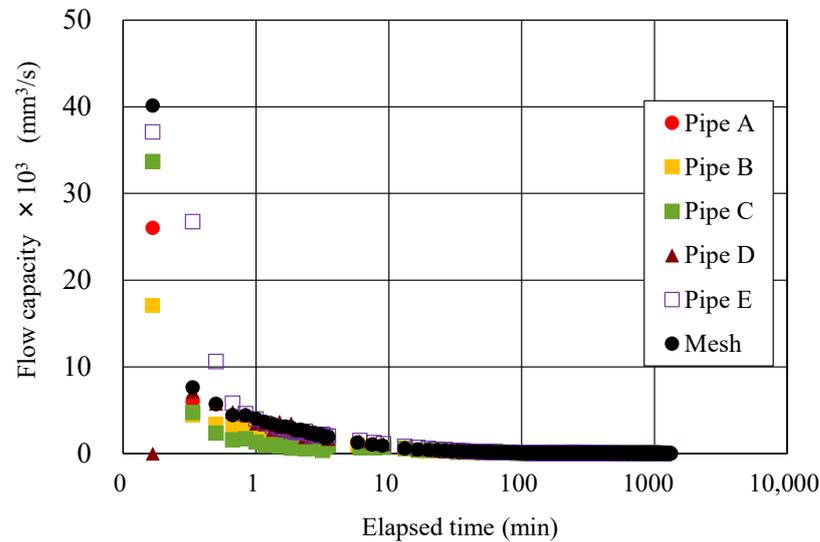
Figure 10. Drainage from MT-MSB filled with water and sand.

Based on the results as shown in Figure 10, the coefficient of flow capacity C_d which is defined as the ratio of the volume of discharged water through each drainage pipe to that through only the wire mesh filter was obtained. In Table 3, C_d for each drainage pipe is shown and it is seen that at the time of 600 min, C_d exceeds 90% for pipes A, C, D and E, and that at 1000 min, pipe A reaches 100%. In this research, since relatively high permeable soil compared with the farmland soil is used, the flow capacity of the sheet pipe is satisfied enough. This point is discussed later more in detail.

Table 3. Coefficients of drainage capacity for each drainage pipe.

Time (min)	Coefficient of Drainage Capacity				
	Pipe A	Pipe B	Pipe C	Pipe D	Pipe E
200	0.79	0.63	0.85	0.95	0.93
300	0.83	0.65	0.87	0.94	0.92
400	0.86	0.67	0.88	0.94	0.92
500	0.89	0.68	0.89	0.93	0.91
600	0.92	0.69	0.91	0.93	0.91
700	0.94	0.69	0.92	0.93	0.91
800	0.97	0.7	0.93	0.93	0.91
900	0.98	0.7	0.95	0.94	0.92
1000	1	0.71	0.97	0.94	0.92

Figure 11 shows the change of flow capacity with the logarithm of elapsed time. At 10 s after opening the drainage valve, the flow capacity reaches $40 \times 10^3 \text{ mm}^3/\text{s}$ and $26 \times 10^3 \text{ mm}^3/\text{s}$ for only wire mesh filter and Pipe A, respectively.

**Figure 11.** Change of flow rate with elapsed time.

3.2. Effects of Drainage by the Sheet Pipe on the Water Head, Matric Suction, and Storage Coefficient

As mentioned before, hydraulic pressures at small holes PP1, PP2, and PP3 on the base plate of MT-MSB were transferred to the pressure transducers. Based on the measured hydraulic pressures, the water head h_w for each point was obtained by $h_w = u/\gamma_w$, where u is the hydraulic pressure and γ_w is the unit weight of water. In Figure 12, changes of the water head are shown against the logarithmic time. It is seen that the decrease in the water head at PP1 and PP2, whose positions from the center of sheet pipe are farther than PP3, is apparently delayed. In Figure 13, changes of the approximated phreatic surface estimated by the hydraulic pressures with time are shown in which $t = 0$ means the initial value. After starting the drainage test, it is seen that the water head quickly drops and the gradient of the phreatic surface changes with the distance from the center of the drainage pipe. The approximated phreatic surface is curved near the sheet pipe. This means that the assumption by Dupuit-Forchheimer provides feasible solutions in a form simpler than that obtainable by rigorous analysis [20]. The continuity equation of water flow has been applied to the saturated porous medium to show the profiles of water level and their changes with time [6].

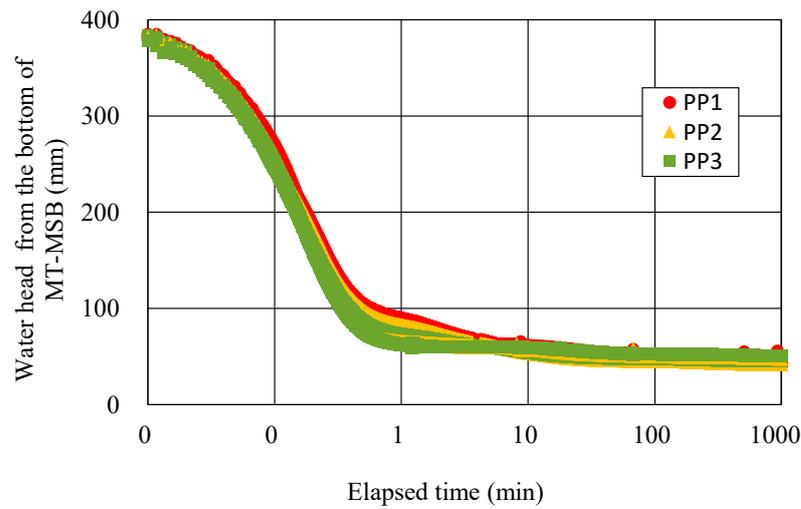


Figure 12. Changes of water head by drainage through the sheet pipe.

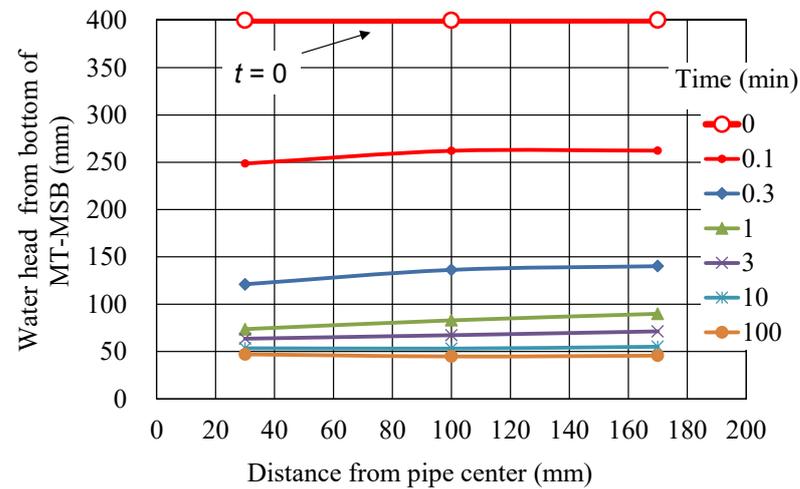


Figure 13. Distribution of water head.

In the drainage tests, the change of hydraulic pressure was also measured through the porous cup column which was set horizontally at the depth 0.10 m, 0.20 m, and 0.30 m from the top surface of MT-MSB by the pressure transducer with the pressure capacity from -100 kPa to 100 kPa. Although the porous cup was placed at the depth $Z = 0.1$ m, 0.2 m or 0.3 m from the top surface of MT-MSB, for the precise measurement of the matric suction under the limited space in MT-MSB, the depth of the porous cup was set individually for each test. This means that a series of drainage tests under the same condition consists of three tests with the depth of porous cup as $Z = 0.1$ m, 0.2 m, and 0.3 m. Relationships between the hydraulic pressure u and logarithm of elapsed time are shown in Figure 14, in which the changes of matric suction ψ_m are also shown; ψ_m is the negative capillary pressure and defined as $\psi_m = u_a - u$, where u_a is the pore air pressure. When u_a is assumed to be negligible, $\psi_m = -u$ is obtained [21].

For every depth, it is seen that immediately after the start of drainage, the hydraulic pressure shows the hydrostatic pressure for each depth and decreases to zero after about 20 s. Thereafter, the hydraulic pressures show negative values, the matric suctions increase and after passing the bending point of the curve, the gradient of the curve ϵ which is defined as $\epsilon = \Delta S_u / \Delta \log t$ gradually decreases. For the time between 100 min and 1000 min, however, it is seen that ϵ is almost constant but increases with the depth [14]. As an influence of suction on the time-dependent compression behavior of fill material, matric suction can increase the compression stiffness [22]. This means that at the surface of wet

farm land, by lowering the GWL the shear strength increases with time even after the GWL becomes constant.

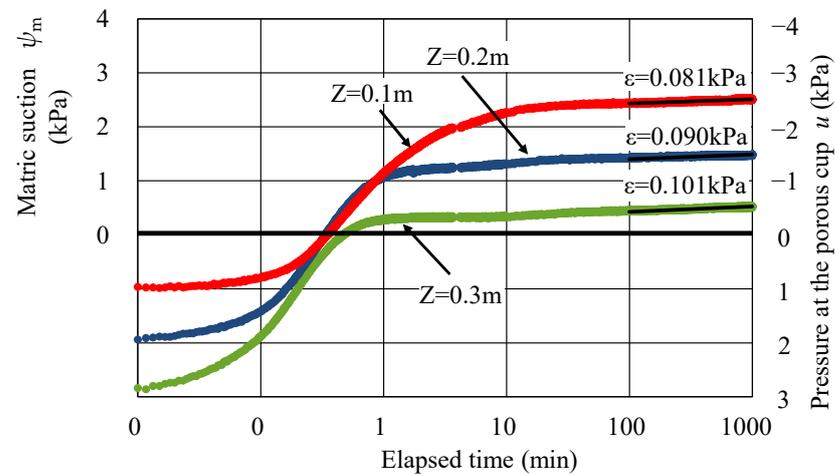


Figure 14. Change of the matric suction.

Although results obtained in this study might be important, to clarify this point more in detail, further experiments are required. However, such experiments exceed our current research objectives.

After finishing the drainage test, the water contents were measured at the depths $Z = 0.0\text{ m}$, 0.1 m , 0.2 m and 0.3 m with the horizontal distance $l = 0.03\text{ m}$, 0.10 m , and 0.17 m from the center of the sheet pipe.

Figure 15 shows the distributions of the volumetric water content θ with the depth, where θ is derived from the water content as follows: $\theta = V_w/V = \gamma_d/\gamma_w \times w$, where V_w is volume of water, V is the volume of soil and water, γ_d is the dry unit weight of soil, γ_w is the unit weight of water, and w is the water content. At $Z = 0.1\text{ m}$, there is a tendency that the closer the sampling position to the sheet pipe, the lower the volumetric water content. At $Z = 0.2\text{ m}$ and 0.3 m , however, θ is almost the same.

Two broken lines in Figure 15 show the initial distribution θ_i and the average value θ_t after drainage tests. At the beginning of the test, since the soil is saturated, θ_i is obtained as $\theta_i = \gamma_d/\gamma_w \times e/G_s = 43.6\%$.

For the case of unconfined aquifer, the storage coefficient S is obtained as a change of the volumetric water content by the following equation [23], and for Figure 15, $S = 9.4\%$ was obtained.

$$S = \left| \int d\theta \right| = |\theta_i - \theta_t| \tag{1}$$

3.3. Drainage Capacity by the Sheet Pipe

3.3.1. Maximum Drainage Capacity through the Horizontally Installed Sheet Pipe

Based on the above-mentioned results, the drainage capacities by the sheet pipe are discussed. Figure 16 shows the intake model for the subsurface drainage by the sheet-pipe system, in which A_s is the intake area, W is the space of installed sheet pipe, L is the length of sheet pipe, D_p is the depth of sheet pipe installation, S is the storage coefficient, and Δz is the lowering plan of GWL.

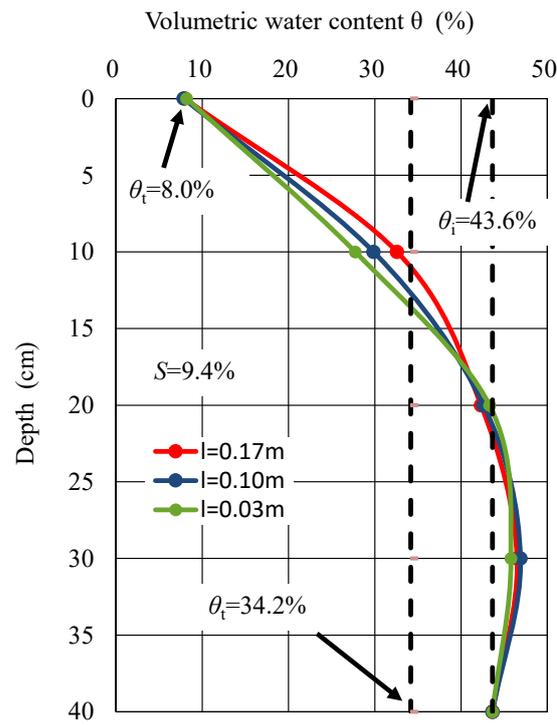


Figure 15. Distributions of volumetric water content.

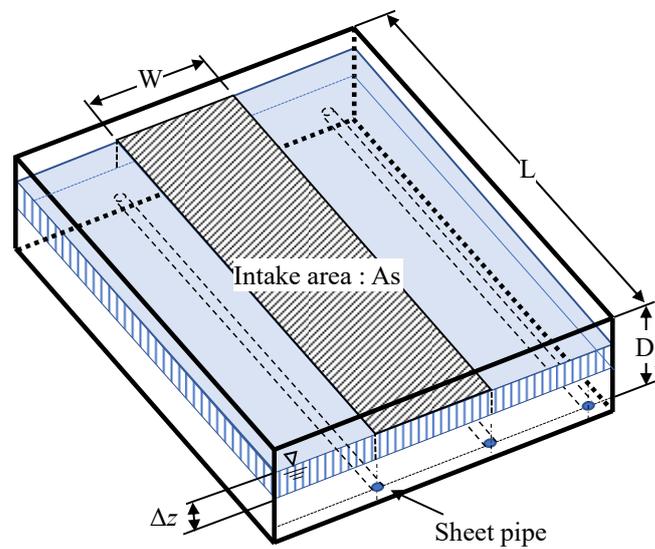


Figure 16. Intake model by the sheet-pipe system.

The intake volume of water V_A and the flow capacity Q are obtained as follows [14,24],

$$V_A = A_s \times \Delta z \times S \tag{2}$$

$$Q = v \times A \tag{3}$$

$$v = \left(\frac{1}{n}\right) \times R^{(2/3)} \times i^{(1/2)} \tag{4}$$

where v is the flow rate and obtained by the Manning’s equation which is widely used to determine the flow capacity for the gravity flow, i is the hydraulic gradient, and A is the flow area inside the sheet pipe. When the pipe is fully filled with water, then $A = \left(\frac{\pi}{4}\right) \times D^2$. D is diameter of the sheet pipe, and R is the hydraulic mean depth. R is obtained by $R = A/P$,

where P is the wetted perimeter, and n is Manning’s n value. In this study, $n = 0.010$ was used for the sheet pipe, based on Figure 17 [25].

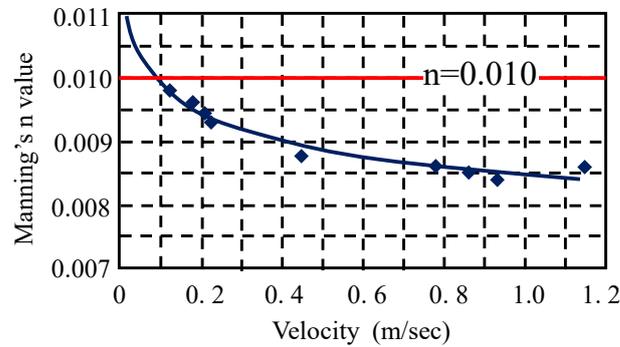


Figure 17. Manning’s n value for the sheet pipe [25].

As mentioned before, since the sheet pipe is installed horizontally in the subsurface layer, a method to predict the maximum lowering rate of the ground water level was developed [14]. When considering that the pipe is fulfilled with water at the downstream end of the sheet pipe, then the hydraulic gradient is named as the critical hydraulic gradient of the sheet pipe i_{cr} and obtained from Equations (2)–(4) as follows.

$$i_{cr} = \left[Q / \left\{ \left(\frac{1}{4} \right)^{5/3} \times \pi \times D^{8/3} \times (1/n) \right\} \right]^2 \tag{5}$$

When the sheet pipe is laid horizontally in the subsurface layer, if the following equation is satisfied, the sheet pipe is supposed not to be fulfilled with the water. Here, i_{max} is the maximum gradient of the pipe.

$$i_{cr} \leq i_{max} = D/L \tag{6}$$

Then the maximum flow capacity Q_{max} is given as follows.

$$Q_{max} = (1/4)^{5/3} \times \pi \times D^{8/3} \times \left(\frac{1}{n} \right) \times (i_{max})^{1/2} \tag{7}$$

When the storage coefficient S is obtained or predicted, the maximum lowering rate of GWL Δz_{max} is obtained as,

$$\Delta z_{max} = Q_{max} / (A_s \times S) \tag{8}$$

To confirm the applicability of this method, firstly the critical hydraulic gradient i_{cr} was calculated for the condition as $\Delta z = 100$ mm/day, $D = 50$ mm, $L = 100$ m, $W = 4.0$ m, $n = 0.010$ and $S = 9.4\%$ which was obtained in the above-mentioned experiments on Toyoura sand. Figure 18 shows the results on the flow capacity of the sheet pipe for $\Delta z = 100$ mm/day on Toyoura sand. In the same figure, a result for Soil1 which was obtained by the hollow-cylinder used drainage tests with 82.5 cmH and 5.2 cmD [23] is shown. For Toyoura sand ($S = 9.4\%$) and Soil1 ($S = 20.0\%$), as a flow capacity, $0.044 \times 10^{-3} \text{m}^3/\text{s}$ and $0.093 \times 10^{-3} \text{m}^3/\text{s}$ were obtained, respectively.

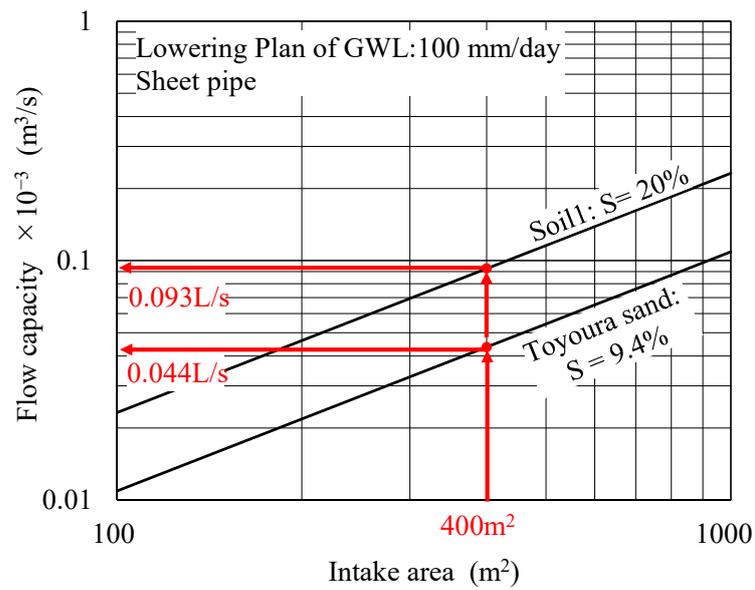


Figure 18. Flow capacity of the sheet pipe.

In Figure 19, the critical hydraulic gradient of the sheet pipe i_{cr} on two soils were obtained as $i_{cr} = 0.0017\%$ for Toyoura sand and $i_{cr} = 0.0077\%$ for Soil1 as follows.

$$i_{cr} = 0.0017\%, 0.0077\% \ll \frac{50}{100,000} = 0.05\% \tag{9}$$

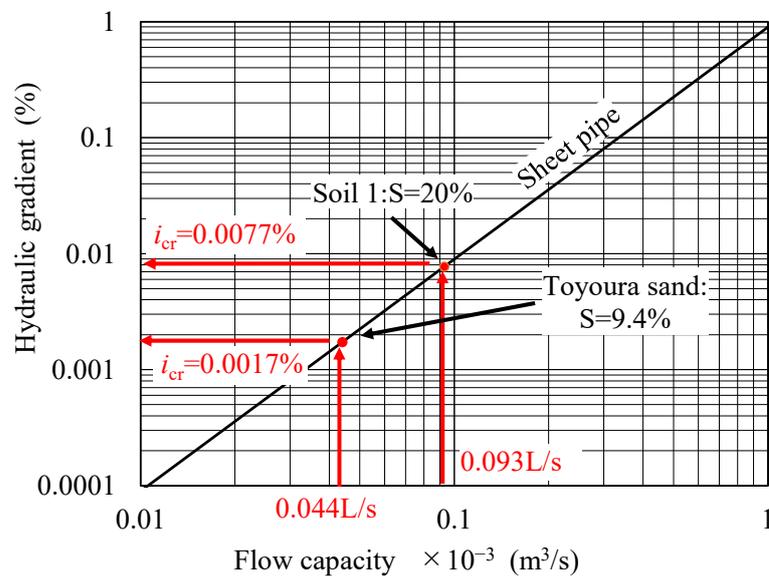


Figure 19. Critical hydraulic gradient.

It is apparent that these results satisfy Equation (6) and this means that even if the ground water level is lowered as $\Delta z = 100$ mm/day, the sheet pipe would not be fulfilled with water.

Maximum flow capacity Q_{max} and maximum lowering rate of GWL Δz_{max} are obtained by Equations (7) and (8) and results on two soils are summarized in Table 4. For the maximum flow capacity of the sheet pipe, $Q_{max} = 0.237 \times 10^{-3} \text{ m}^3/\text{s}$ was confirmed less than $Q = 1.14 \times 10^{-3} \text{ m}^3/\text{s}$ which was obtained at 10 s after starting in the direct drainage test through the sheet pipe in the MT-MSB without the soil and mesh filter.

Table 4. Maximum drainage capacity Q_{max} and maximum lowering rate of GWL.

Sample	S (%)	L (m)	W (m)	D (mm)	Δz (mm/d)	$Q (\times 10^{-3})$ (m ³ /s)	i (%)	$Q_{max} (\times 10^{-3})$ (m ³ /s)	Δz_{max} (mm/d)
Toyoura sand	9.4	100.0	4.0	50.0	100.0	0.044	0.017	0.237	543.4
Soil1	20.0	100.0	4.0	50.0	100.0	0.093	0.077	0.237	255.4

3.3.2. Limitations and Constraints

The shape of the water table between parallel drain pipes is generally described as elliptical or the fourth-degree parabola [5,26]. In the above mentioned intake model, however, to find the drainage capacity of the sheet pipe, the ground water table was assumed to drop down keeping the water table horizontal as an average.

For these water tables, to find the drain spacing there are two types of formula commonly used. These are the steady state formula and the non-steady state formula. As a steady state formula, the Hooghoudt formula, and as a non-steady state formula, the Glover-Dumm drainage formula have both been used for determining the desirable spacing and depth of drains to maintain the water table below a certain level [20,26,27]. In these two types of formula, since the Glover-Dumm drainage formula is based on the schematization $h < d < W$, the pipe spacing should be much larger than h and d , where h is the height of the ground water table at the midway point between drains and d is the equivalent depth to the impermeable substratum [26]. In this study, since the impermeable layer in the MT-MSB was placed at almost the same level as the drain and in the sheet pipe system, the drain spacing is set empirically as short as about 4 m, the Hooghoudt formula as shown below was used.

$$W^2 = \frac{8k_2dh}{q} + \frac{4k_1h^2}{q} \quad (10)$$

where k_1 and k_2 are the hydraulic conductivity for the upper and lower layer, respectively, (m/d) and q is the discharge rate per unit horizontal area (m/d). Table 5 shows the results obtained by Equation (10) under the condition of the soil which is isotropic and homogeneous for the water flow. As for the thickness of equivalent layer below the drains was set as 0.7 m and the height of GWL at the center between drains was changed as 0.1, 0.2 m and 0.3 m. For these conditions, the spacing of the drainage pipe was obtained as 19.9 m, 15.8 m and 10.9 m, respectively. These drain spacings are larger than $W = 4.0$ m. This means that the maximum flow capacity obtained in this study is applicable as a safe side.

Table 5. Pipe spaces calculated by Hooghoudt's equation.

Sample	Toyoura Sand		
h (height of GWL at the center above pipe) m	0.30	0.20	0.10
$\Delta Z = q$ (lowing rate of GWL) m/d	0.10	0.10	0.10
W (drain spacing) m	19.90	15.76	10.79

Note: Relative density $D_r = 60\%$. Coefficient of permeability for above and below drainage pipe $k_1 = k_2 = 2.246 \times 10^{-4}$ = m/s. Thickness of equivalent Layer $d = 0.7$ m.

3.4. A Case Study on the Drainage by Using the Sheet-Pipe System

The first application of the sheet-pipe system was performed at the Indonesian Center for Rice Research, Sukamandi, West Java, Indonesia in 2018 as a pilot project [12,14]. The tested site is a paddy field with an area of 10,000 m² and the field was divided into two areas (i.e., as a sheet pipe installed area (SP-area) and non sheet pipe area (NSP-area)). In SP-area, the sheet pipe was installed by the Mole Drainer at a depth of $D_p = 0.45$ m and the sheet pipe spacing was set as 4.0 m. The pilot project area and outline of the test field are shown in Figures 20 and 21. Figure 22a shows a mole drainer which is drawing the sheet

pipe into the paddy field and Figure 22b shows the cross section of the sheet pipe installed in the subsurface layer.

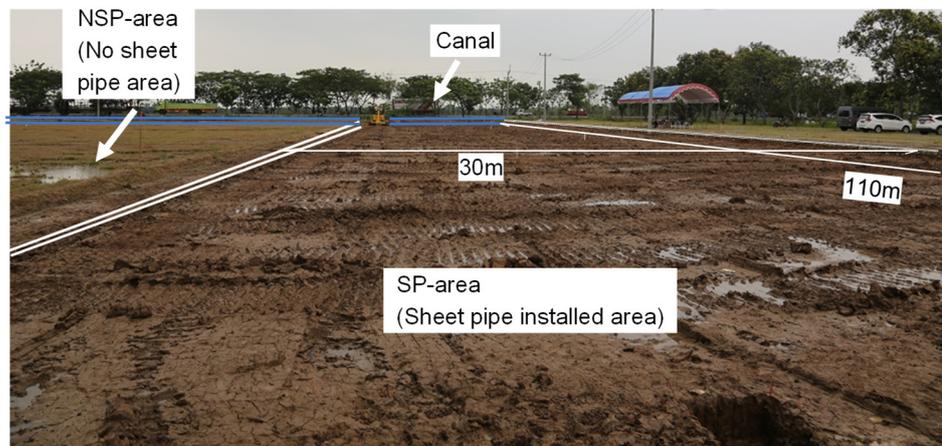


Figure 20. Pilot project area of 1000 m².

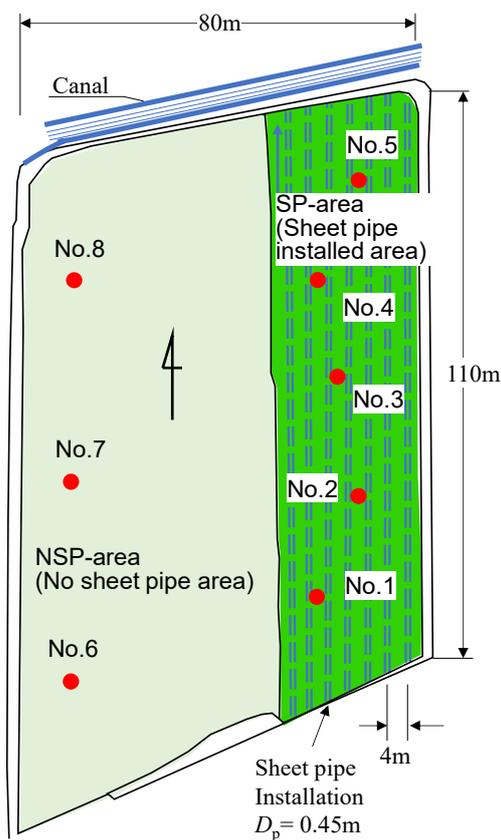


Figure 21. Outline of the test site.

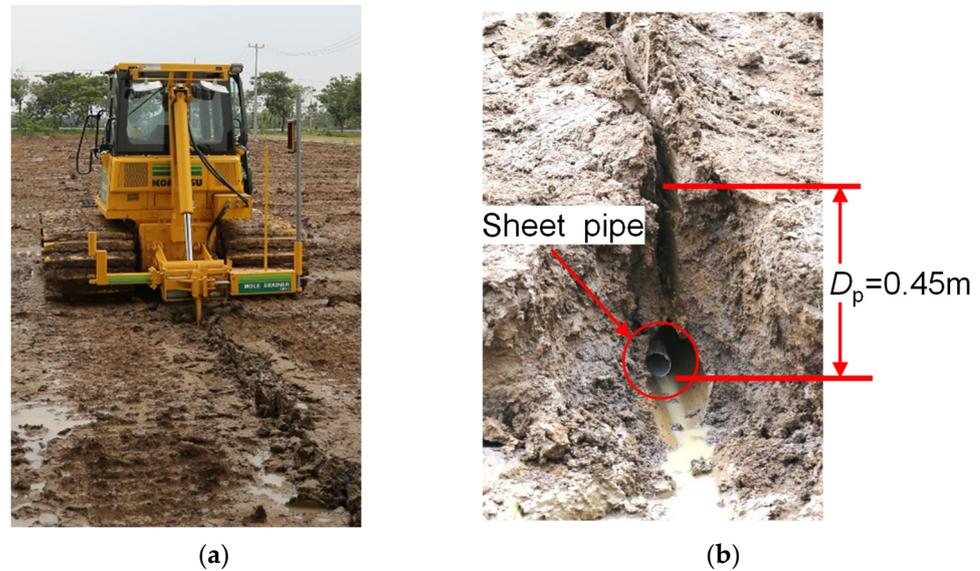


Figure 22. (a) Sheet pipe is under drawn into the ground, (b) cross section of subsurface layer after the insertion of sheet pipe.

As shown in Figures 20 and 21, a narrow canal is located at the north end of the test site into which the water was discharged through the sheet pipe. Table 6 shows the physical properties of samples obtained at Nos.1–5 in Figure 21 and the coefficients of permeability observed by the tube method. It is seen that all the soils are classified as CH (Inorganic clay with high plasticity) and the permeability is very low compared with the soil used in the MT-MSB tests as mentioned before.

Table 6. Physical properties of sample at SP-area (0.0–0.5 m).

Bore Hole	No.1	No.2	No.3	No.4	No.5
Clay(%)	69.10	80.75	67.06	61.82	57.77
<0.005 mm					
Silt(%)	29.76	19.25	29.72	36.02	39.43
<0.075 mm					
Fine sand(%)	1.14	0.00	3.00	2.16	2.12
<0.420 m					
w (%)	29.85	37.45	36.14	39.52	34.37
G_s	2.540	2.535	2.544	2.531	2.493
PI	32.77	45.37	44.18	32.97	37.80
Void ratio	0.76	0.96	0.93	1.01	0.87
k (m/s)	3.75×10^{-8}	1.98×10^{-7}	1.47×10^{-7}	5.9×10^{-9}	3.09×10^{-7}

Note: w , water content; G_s , specific gravity; PI , plasticity index; k , coefficient of permeability.

At the SP-area, the sheet pipe was installed on 12 and 13 March, 2018. Figure 23 shows the changes of GWL observed at the bore holes in SP-area and NSP-area (red and black lines show the average of GWL for SP-area and NSP-area, respectively) and the water level at the canal.

In the same field, the evaluation of the sheet-pipe system on the crop productivity was conducted based on the field investigation from 5 May to 6 August, 2018 as one rice season. At the SP-area, the grain yield with more than 13.36% was confirmed [12].

When focusing on from 4 to 12 April, 2018 in Figure 23, GWL is affected by the sheet pipe used drainage, rain, and the water level at the canal. Especially, by quick dropping of the water level in the canal, definite differences in GWL between SP and NSP areas are seen, e.g., on 6 April, GWL at the SP area is 0.073 m and at the NSP area is 0.002 m, so GWL at the SP area is 0.071 m lower than the NSP area. This means that although GWL is

affected by the rain and water level at the adjoining canal, even in such a case as that the free board in the canal is limited, the sheet pipe system functions effectively.

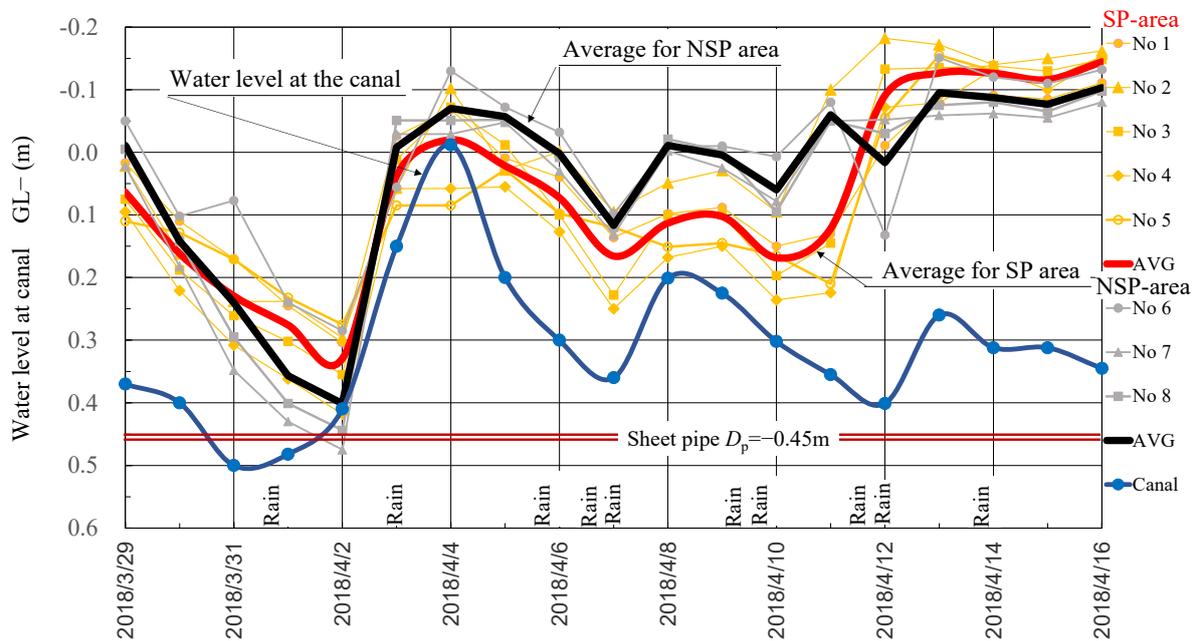


Figure 23. Effect of sheet pipe drainage on the ground water level lowering at the test site.

By applying Equation (10) to this site q is obtained for the parameters as the coefficient of permeability $k_1 = k_2 = k_{av} = 1.39 \times 10^{-7}$ m/s, where k_{av} is the average coefficient of permeability of those shown in Table 6 and the pipe spacing $W = 4.0$ m. Then since no information about the layer below the sheet pipe was obtained, the thickness of equivalent layer was set as $d = 0.7$ m. As a result, $q = 1.5$ mm/d, 0.96 mm/d and 0.45 mm/d for $h = 0.3$ m, 0.2 m, and 0.1 m were obtained, respectively. The average of daily drop of GWL measured at the site between 5 and 7 April is 93 mm, and this value is significantly larger than the calculated ones. The reasons why these differences occur are mainly considered due to that the available equations including the Equation (10) are based on assumptions such as isotropic and homogeneous of soil to idealize and simplify the flow system [15].

4. Conclusions

In this study, to clarify the applicability of the sheet-pipe system to a wet land which is affected easily by the ground water level rising, firstly, the differences in the flow capacity between the sheet pipe and another type of commercially used pipes were observed. Secondly, in the sheet-pipe system, when the sheet pipe is drawn horizontally into the subsurface layer where the free board of the adjoining canal is limited, the maximum lowering rate of the ground water level was derived based on the storage coefficient obtained by MT-MSB tests. Thirdly, as a case study, the sheet pipes were laid at the level farm land close to the canal. The efficiencies of the sheet-pipe system were confirmed by comparing the change of the ground water level at the farmland with a sheet pipe and without one (concerning the rain, water level at the adjoining canal, drainage capacity of the sheet pipe, and the pipe spacing).

These results mean that at a wet farmland such as one near the seacoast with a relatively smaller block in area, subsurface drainage is possible and the sheet-pipe system would especially contribute to the sustainable restoration of degraded farm land due to climate change-induced rise of GWL or salinization.

When spreading the subsurface drainage at a wetland, as an advantage, the agricultural mechanization might develop further by the improvement of trafficability due to the suction-induced increase of the bearing capacity at the ground surface.

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