

Article

Integrated Application of Remote Sensing, GIS and Hydrological Modeling to Estimate the Potential Impact Area of Earthquake-induced Dammed Lakes

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Abstract: Dammed lakes are an important secondary hazard caused by earthquakes. They can induce further damage to nearby humans. Current hydrology calculation research on dammed lakes usually lacks spatial expressive ability and cannot accurately conduct impact assessment without the support of remote sensing, which obtains important characteristic information of dammed lakes. The current study aims to address the issues of the potential impact area estimate of earthquake-induced dammed lakes by combining remote sensing (RS), a geographic information system (GIS), and hydrological modeling. The Tangjiashan dammed lake induced by the Wenchuan earthquake was selected as the case for study. The elevation-versus-reservoir capacity curve was first calculated using the seed-growing algorithm based on digital elevation model (DEM) data. The simulated annealing algorithm was applied to train the hydrological modeling parameters according to the historical hydrologic data. Then, the downstream water elevation variational process under different collapse capacity conditions was performed based on the obtained parameters. Finally, the downstream potential impact area was estimated by the highest water elevation values at different hydrologic sections. Results show that a flood with a collapse elevation of at least 680 m will impact the entire downstream region of Beichuan town. We conclude that spatial information technology combined with hydrological modeling can accurately predict and demonstrate the potential impact area with limited data resources. This paper provides a better guide for future immediate responses to dammed lake hazard mitigation.

Keywords: earthquake-induced dammed lakes; flood; potential impact area; estimate; hydrological modeling; RS; GIS

1. Introduction

Dammed lakes formed by earthquake landslides are an important secondary hazard induced by earthquakes. Damming objects, sliding from mountain slopes, comprise rocks and weathered soil, which have strong penetrability, low stickiness, and high instability. Damming objects easily collapse and threaten human beings who live downstream [1,2]. Rainfall usually occurs after an earthquake. Given that a high amount of rainfall falls upstream, the damming objects block the runoff and destroy the river function, thereby increasing the water level and the risk of the dam collapsing. A flood that develops under these extreme conditions after a dam collapses is more dangerous than the usual mountain torrent disasters. Such floods will generate a second calamity to the people who have just suffered from an earthquake.

Remote sensing techniques are widely applied for extracting water bodies and monitoring floods [3–11]. Using remote sensing images, digital elevation model (DEM), and geographic information system (GIS) can also generate flood mapping [12–15]. The earthquake-induced

dammed lake is formed suddenly with the occurrence of earthquakes. It is not long-standing. Therefore, there is no long-term historical data on the lake that can provide a reference for the study. In recent years, remote sensing of dammed lakes has been researched increasingly due to the frequent occurrence of serious earthquake disasters. People have paid more attention to dangerous lakes that continually threatening the earthquake disaster victims. For example, after an earthquake measuring 8.0 on the Richter scale struck Wenchuan of the Sichuan province of China on 12 May, 2008, and the Ya'an earthquake happened on 20 April 2013, many studies have focused on the interpretation of dammed lake properties by using remote sensing images such as DEM and digital orthophoto map (DOM). Many related studies have dynamically monitored dammed lakes forming in the earthquake and have acquired relative lake property information, such as the area, the backwater length, and the water volume, based on multi-platform remote sensing [16–22]. Very high-resolution (VHR) satellite imagery and the high-resolution synthetic aperture radar (SAR) imagery were also used to assess the effect of the Wenchuan earthquake as a complementary data source to aerial photography [23]. Remote sensing techniques are increasingly important in acquiring firsthand information on emergent dammed lake events.

Remote sensing (RS) was also used to survey the hydrological variations caused by earthquakes. Hsu et al. [24] studied an earthquake of 7.3 on the Richter scale whose epicenter occurred at Jiji, in the central of Taiwan. The effect of earthquake-induced dammed lakes on channel evolution and hydrological analysis of mobile beds was studied based on remote sensing surveys. The RS technique can provide the necessary parameters of the hydrological model. A hydrological model of the dammed lake in Santa Maria Basin, Argentina was established to estimate hydrological variations caused by the newly forming lake [25]. Carmelo et al. [26] combined hydraulic geometry, GIS, and RS into rainfall-runoff models to estimate the flood magnitude in ephemeral fluvial systems.

Meanwhile, several studies on dam-break have been promoted recently. They combined a hydraulic empirical model and a physical model to improve the forecasting accuracy of dam-break floods [27]. Methods on risk assessment related to dam-break floods have obtained increasing attention due to the increase in earthquake-induced dammed lake events [28–31]. Related emergency strategies and methods based on risk assessment were also promoted [32].

Summarizing the above studies, we conclude that many studies on dammed lakes only use the RS technology that focuses on the current situation, such as the location of the damming object and the current inundated area; these methods have failed to consider future trends. Several GIS studies about dammed lakes have an excellent spatial expressive ability to obtain the reservoir storage, the submerging depth, and the submerging area. However, most of them simply deal with water elevation problems wherein the water surface is assumed to be flat. The hypothesis is approximately appropriate to the upstream of the dammed lake, whereas the hypothesis is not obviously reasonable for area downstream. Hydraulic calculations can exactly forecast the future flood routing, but they are usually not combined with RS and GIS and lack a spatial expressive ability. Moreover, hydraulic calculation is time-consuming and requires massive boundary conditions, whereas the dammed lake formed by a sudden landslide usually lacks complete data to support the complex hydraulic calculations. Dammed lake disasters must take full advantage of limited data resources to conduct emergency response in a short time. The response must effectively and rapidly estimate the corresponding potential impact area based on different given dam-break conditions. Therefore, this study combines RS, GIS technologies, and hydrologic data to establish an effective method to estimate the impact risk of dammed lakes in the corresponding downstream regions under different water elevation conditions. The study in this paper can be widely applied to different earthquake-induced dammed lake cases because it only requires DEM, historical hydrological data, and current RS images that are easy to obtain.

2. Study Area and Dataset

On 12 May 2008, a magnitude 8.0 earthquake occurred in Wenchuan country, Sichuan province, China. The earthquake induced a massive mountain landslide that destroyed the natural river discharge function and formed 33 large dammed lakes. The largest dammed lake, Tanjiashan dammed lake, was selected as the study area. Tanjiashan is located at the upstream of the Xun River and is only six kilometers from Beichuan town, which is the largest town downstream from the dammed lake and was also severely afflicted by the Wenchuan earthquake disaster. Thus, Tanjiashan dammed lake has the highest risk level among all of the dammed lakes.

The experimental data mainly include DEM, various satellite RS images, aerial photographs, and historical hydrologic data (observed at Tongkou and Xiangshui hydrologic stations) (Figure 1). All of the images employed in the study are referenced in the World Geodetic System (WGS84) datum, and are projected to the Universal Transverse Mercator system. Further information about the specifications of the data used in this study is provided in Table 1.

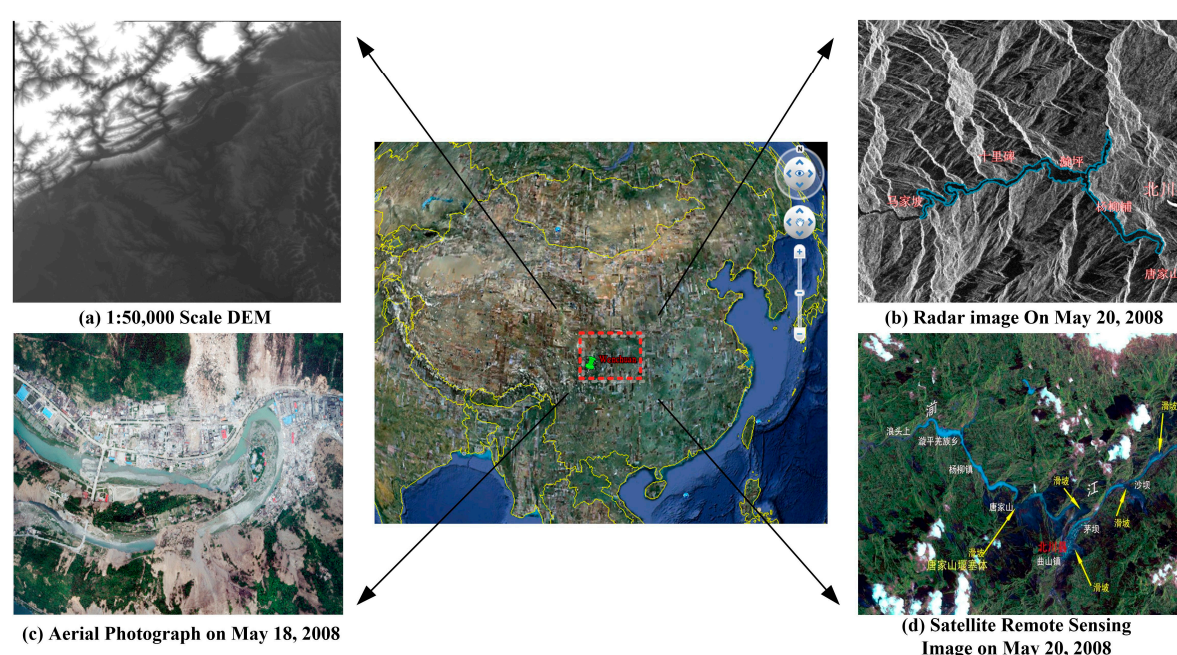


Figure 1. Study area and data.

Table 1. Specifications of the satellite remote sensing image, aerial photograph, and satellite radar data used in this study.

Data	Acquisition Date	Resolution	Format	Region
Formosat-2	20 May 2008	2 m	TIFF	Tanjiashan Dammed Lake
Aerial photograph	18 May 2008	0.15 m	TIFF	Beichuan Town
Envisat	20 May 2008	30 m	TIFF	Tanjiashan Dammed Lake
DEM	2003	1:50,000	TIFF	Xun River Basin

3. Methodology

The research process is demonstrated in Figure 2.

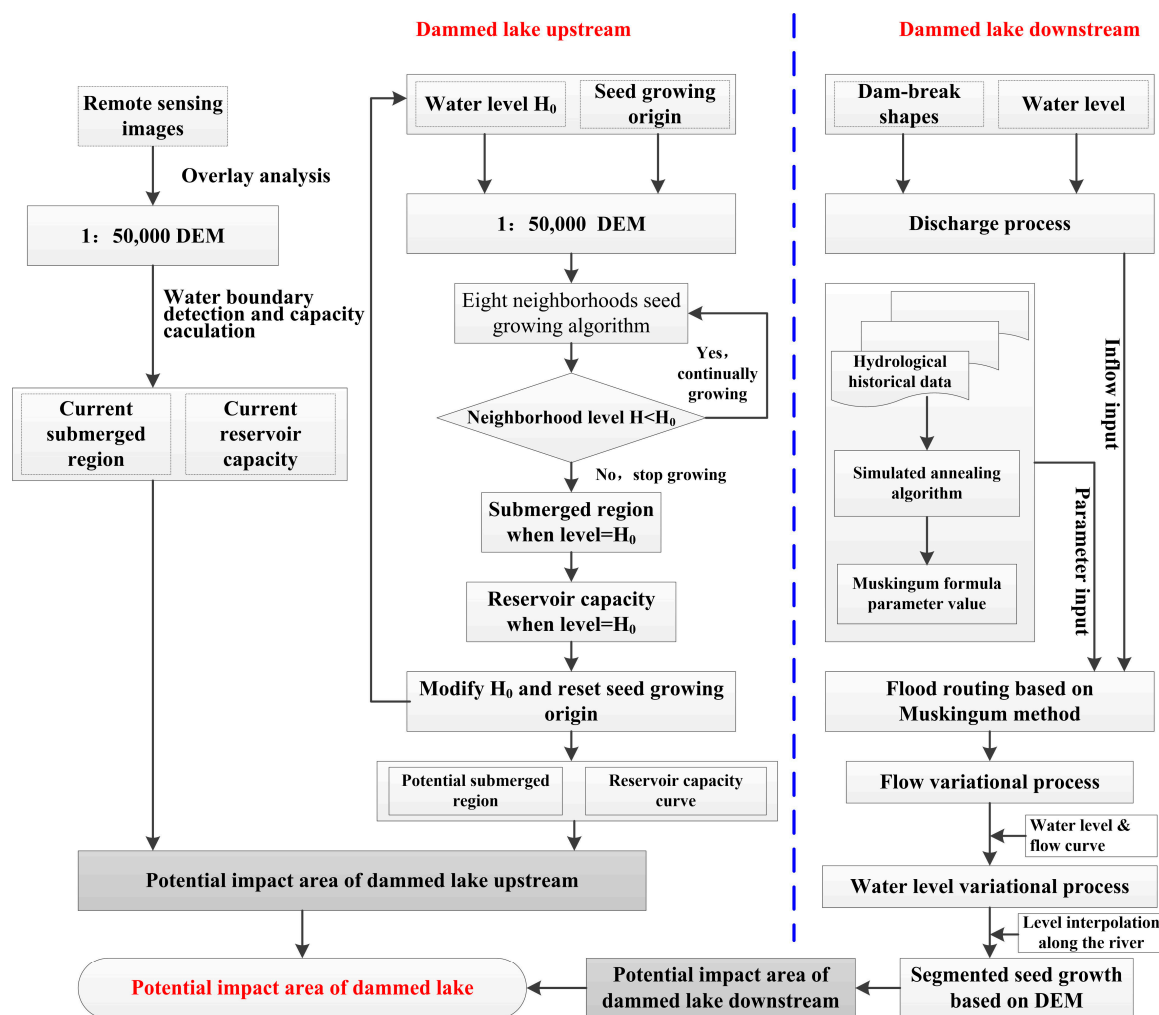


Figure 2. Research flow chart.

3.1. Extraction and Calculation of Dammed Lake Parameters

The dammed lake's characteristic parameters, including its location, current submerged area, storage volume, and other geometric characteristics, can be extracted based on the DEM data obtained before the earthquake and the RS images acquired after the earthquake. The dammed lake's geometric parameters, including the location, size, and area of the damming object, were acquired by visual interpretation from Formosat-2 images. The reservoir storage capacity curve was calculated based on the DEM when the location of the dammed lake was known. The elevation versus reservoir capacity curve is essential in forecasting the tendency of the downstream potential submerged area that affects the estimation of the submerging loss and risk avoidance decision. We applied the eight neighborhoods seed-growing algorithm [33] to calculate the reservoir storage capacity curve based on the DEM data.

The principle of the algorithm is to locate damming objects from RS images acquired after the earthquake, where a line segment across the river channel is set as a pretend dam body. A seed origin at the center of the upper dammed lake is selected to spread on the DEM below a given elevation. In designing the algorithm, the elevation value of the line segment across the river channel can be first set to an invalid value (Null) or a maximum (such as elevation = 99,999). When a seed grows nearby the dam site, it cannot cross the dam and proceed downstream because of the existence of the invalid values. This method is similar to the clustering method in the image process. It is an iterative or recursive judgment process for computer programming. It repeatedly judges whether the given elevation H_0 is higher than seed's eight neighborhoods. If the elevation of any neighborhood is lower than H_0 , then the neighborhood (pixel) would be viewed as a submerged grid

and pushed to the top of the stack. Then, the pixel on the top of stack will be selected as the current seed. If no neighborhood satisfies the condition (lower than H_0), then the current seed will be ejected from the top of the stack. The push and pop-up circle will repeat until all the seeds in stack are ejected. When finding a satisfied grid, the accumulated volume is added to the current volume, which can be expressed with the following equation:

$$\text{Volume} = [\text{given level } H_0 - \text{grid elevation}] \times \text{grid area} \quad (1)$$

Finally, the number of grids, once pushed into the stack, represents the submerged area. The accumulated volume is the reservoir capacity corresponding with the H_0 condition.

After a group of given water level values were assumed to calculate the corresponding water surface area and reservoir capacity with DEM data, the elevation versus area and reservoir capacity curve was established by the above calculation results.

3.2. Dam-Break Flow Calculation

The weir flow empirical formula (Equation (2)) was adopted to calculate dam-break flow:

$$Q = \varepsilon m b \sqrt{2g} H_0^{3/2} \quad (2)$$

where Q is outflow, g is the free fall acceleration, b is the overflow surface width, m is the flow coefficient, ε is the side contraction coefficient, and H_0 is potential head, including the elevation heads and velocity heads. Upon initiating the dam-break mode, the break shape and time step are given, and the outflow process can be calculated. The break shapes can be divided into several typical styles, such as a triangle or trapezoid. The dam-break modes can also be an entire break, a half break, and a 1/3 break. In fact, the real time of dam-break is unknown. People usually assume several typical time schemes of dam-break, such as 1 h entire break, 2 h entire break, and 6 h entire break. The continuity Equation (3) is expressed as follows:

$$\sum_{i=1}^n (I_i - Q_i) \Delta t = \Delta W \quad (3)$$

where Δt is the time step; ΔW is the reservoir capacity variation; I_i and Q_i are the inflow and outflow of reservoir, respectively; and, n is the length of the time series. The water elevation H determines H_0 and b . All of the parameters in Equation (3) are known except H . According to the Equations (2) and (3), storage capacity versus elevation curve and the break shape, represented by the dynamic coefficient H , can be solved. The dam-break flow is the initial condition for the downstream flood routing. The potential impact area is formed by the largest flood; thus, we select the most dangerous combination, in which entire break mode consumes 1 h.

3.3. Flood Routing with an Improved Muskingum Method

The Muskingum flood routing method was adopted in this paper to calculate the potential impact area downstream because of its accuracy and simplification.

3.3.1. Muskingum Method

The principle of the Muskingum method can be described by the difference form of continuous equation and the channel storage equation [34]. The equations are expressed as follows:

$$\left(\frac{I_1 + I_2}{2} \right) \Delta t - \left(\frac{O_1 + O_2}{2} \right) \Delta t = \Delta W \quad (4)$$

$$W = k [xI + (1-x)Q] = kQ' \quad (5)$$

Through simultaneous of Equations (4) and (5), the Muskingum flood routing equation is defined as follows:

$$O_2 = C_0 I_2 + C_1 I_1 + C_2 O_1 \quad (6)$$

where,

$$\begin{cases} C_0 = \frac{0.5\Delta t - kx}{0.5\Delta t + k - kx} \\ C_1 = \frac{0.5\Delta t + kx}{0.5\Delta t + k - kx} \\ C_2 = \frac{-0.5\Delta t + k - kx}{0.5\Delta t + k - kx} \end{cases}, C_0 + C_1 + C_2 = 1 \quad (7)$$

where I (input) and O (output), respectively, represent the inflow rate and outflow rate to the reach, W is the water storage between the upper river section and the lower section, and k and x are equation parameters. Equation (6) is the basic equation. If the parameters, namely, C_0 , C_1 , and C_2 are known, then the outflow sequences can be calculated according to Equation (6) when the inflow sequences and initial outflow O_1 are given.

3.3.2. Parameter Optimization with Simulated Annealing

The key of the Muskingum flood routing method is determining the equation parameters. The traditional method of solving equation parameters is a tentative calculation algorithm with low efficiency and low precision. Simulated annealing (SA) [35], one of the intelligent algorithms, is used to solve the Muskingum parameters in this paper. SA, which is derived from thermodynamics theory to simulate the annealing process, is a universal probability algorithm. The algorithm simulates the slow temperature descent based on the initial given temperature parameter and searches the optimal solution of a subject in the entire solution space.

The algorithm structure is shown in Figure 3. After the initialization setting step, the SA procedure generates random equation parameters, $C_0, C_1 \in (-1, 1)$, as the current location. The last parameter, C_2 , can be used instead of $1.0 - C_0 - C_1$. Then, the other group of C parameters is generated and considered as a disturbance neighborhood near the current location. The program uses the Muskingum flood routing equation (Equation (6)) to calculate both outflows ($Q(i), Q(i+1)$) according to the mass hydrological historical data. The program also applies a cost function (Figure 3) to check which group of C parameters is better. The cost function comprises a mean square deviation formula that judges the difference between calculated outflow and practical outflow Q' acquired from hydrological observation stations. If $dE = J(Q(i+1)) - J(Q(i)) \leq 0$ (Figure 3), signifying that $C(i+1)$ is better than $C(i)$, then the neighborhood will be accepted to be current solution. Otherwise, some acceptance probability ($\exp(-dE/T) > \text{random}(0, 1)$) is used to accept the worse neighborhood. The purpose of acceptance probability is to avoid falling into a local minimum instead of global optimization at the beginning of calculation. According to the instance, a higher T indicates that searching time is not yet long and is likely to decrease. Thus, the probability $\exp(-dE/T)$ deciding to jump the circle of local optimization is greater than that during the late cooling stage. Likewise, a smaller dE illustrates that the difference between the current location and the neighborhood is little and the program can accept a worse neighborhood in a certain probability to extend the beneficial searching scope.

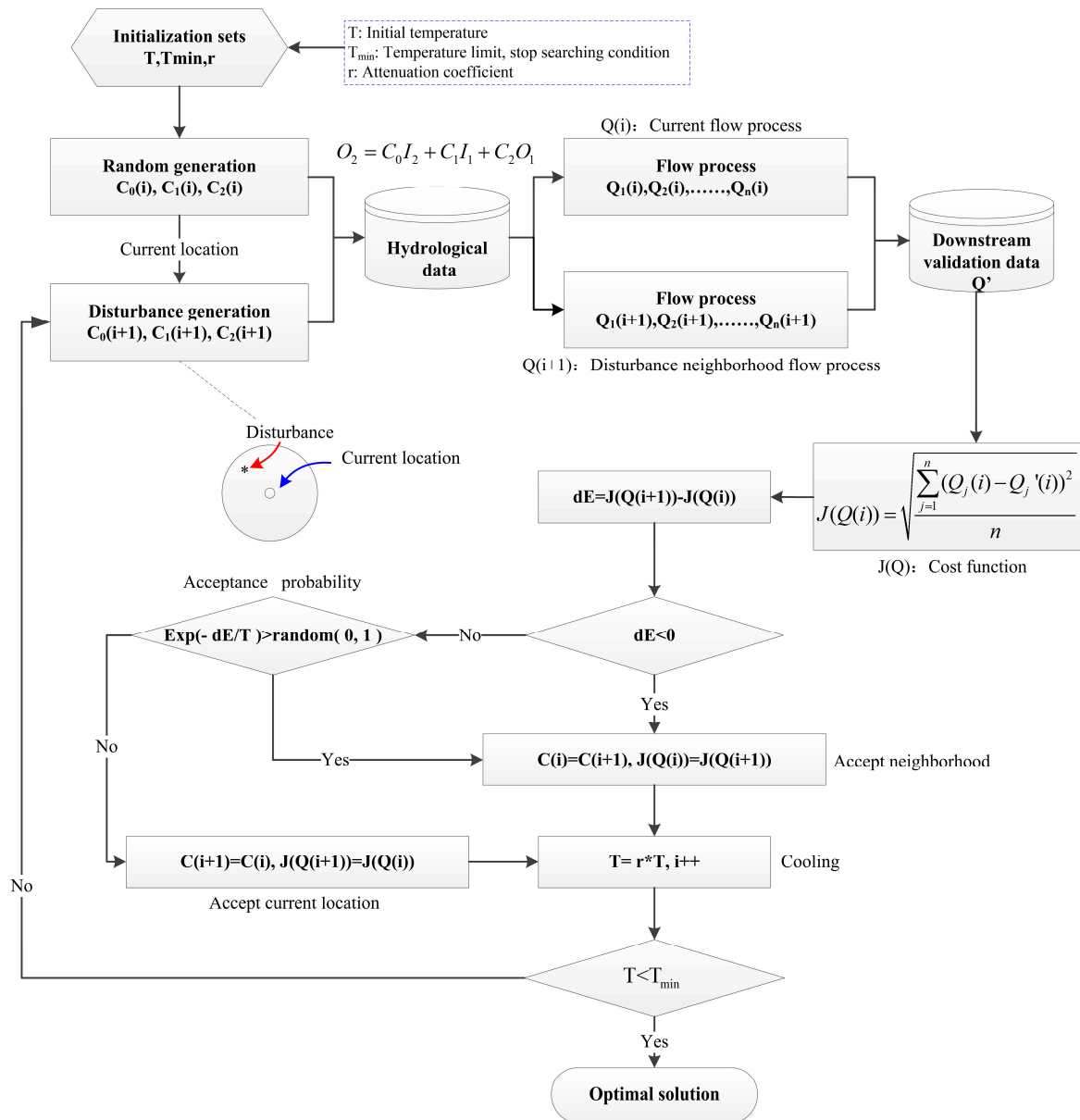


Figure 3. Simulated annealing algorithm flow chart.

The SA method is independent of an initial value and asymptotic convergence. It improves on the traditional Muskingum tentative calculation and avoids solving middle parameters (k, x) , and judging the $W \sim Q'$ curve regardless of whether a line is approached or not (ending condition). Its advantage is its direct adjustment of equation parameters C_0, C_1, C_2 .

3.4. Estimation for Potential Impact Area

3.4.1. The Runoff vs. Elevation Curve

When the runoff peak spreads through downstream hydrologic stations, the submerged area achieves a maximum (potential impact area). The Muskingum flood routing result is the indirect runoff information, not the elevation information itself. Thus, the flow must be translated into a corresponding elevation by the runoff versus elevation curve acquired from survey data polynomial fitting. The polynomial fitting on every hydrologic station section represents the relationship between the runoff with the water level based on long-term observation data. The quadratic or cubic equations usually fit the runoff versus elevation curve very well for mountainous rivers, most of them having simple regular fluctuations.

3.4.2. Estimation for Dam-Break Damage based on Remote Sensing Image and DEM

Through the Muskingum flood routing method and the runoff versus elevation curve, we obtained all of the maximum water levels at every hydrologic station section. However, the hydrologic stations were limited, and the downstream water elevation was descending along the river channel. Therefore, this study adopted the distance interpolation method to deal with the maximum water level of the control sections (including dam, hydrologic stations) and generated a series of dense elevations in all of the segments along the river. The river channel was divided into many segments. All of the segments were manually divided by approximately equal distances and were vertical to the river channel. The maximum water level at each section was interpolated by distance and the known water level.

The potential impact area at every segment was generated by the dense elevation values and the DEM data. We used the seed growing algorithm identical to the one used when calculating the elevation versus reservoir capacity curve. The calculation formed a series of segment documents. The entire impact area downstream was generated by overlaying all of the segment documents. The submerged elevation at every segment was viewed as flat. Thus, the junction about the adjacent segments was not smooth. Sometimes we could use enveloping line or smooth algorithm to obtain the smooth submerging boundary. Once the impact area was known, we could generally analyze the submerging depth and area and evaluate the disaster population, economy loss, and other essential damage information by overlaying the impact area, DEM, and RS image.

4. Calculation Example and Analysis

4.1. Dammed Lake Parameters based on DEM and Remote Sensing Images

According to the results interpreted by the multisource remote sensing images (Formosat-2, Aerial photograph.), the location of Tangjiashan dammed lake is approximately 104°24'27" E and 31°51'30" N. The shape of dammed objects is rectangular. The length along the river is approximately 803 m, and the width is 611 m. Combining RS images with the DEM shows that the top of the damming objects is 775.0 m on the right bank of the river and 793.9 m on the left.

When basic spatial information about Tangjiashan dammed lake is known, we assume a group of characteristic water level values and apply the eight neighborhoods seed growing algorithm to calculate the reservoir storage capacity curve based on the DEM data. The elevation-area-reservoir capacity curve and submerged range are shown in Table 2 and Figure 4.

Table 2. Elevation-area-reservoir capacity curve.

Water Level (m)	Reservoir Capacity (10 ⁸ m ³)	Area (m ²)
690	0.248	1.591
695	0.338	1.958
700	0.445	2.430
705	0.572	2.680
710	0.714	2.975
715	0.872	3.380
720	1.076	4.538
725	1.318	5.077
730	1.587	5.671
735	1.888	6.276
740	2.229	7.223
745	2.607	7.883
750	3.018	8.564

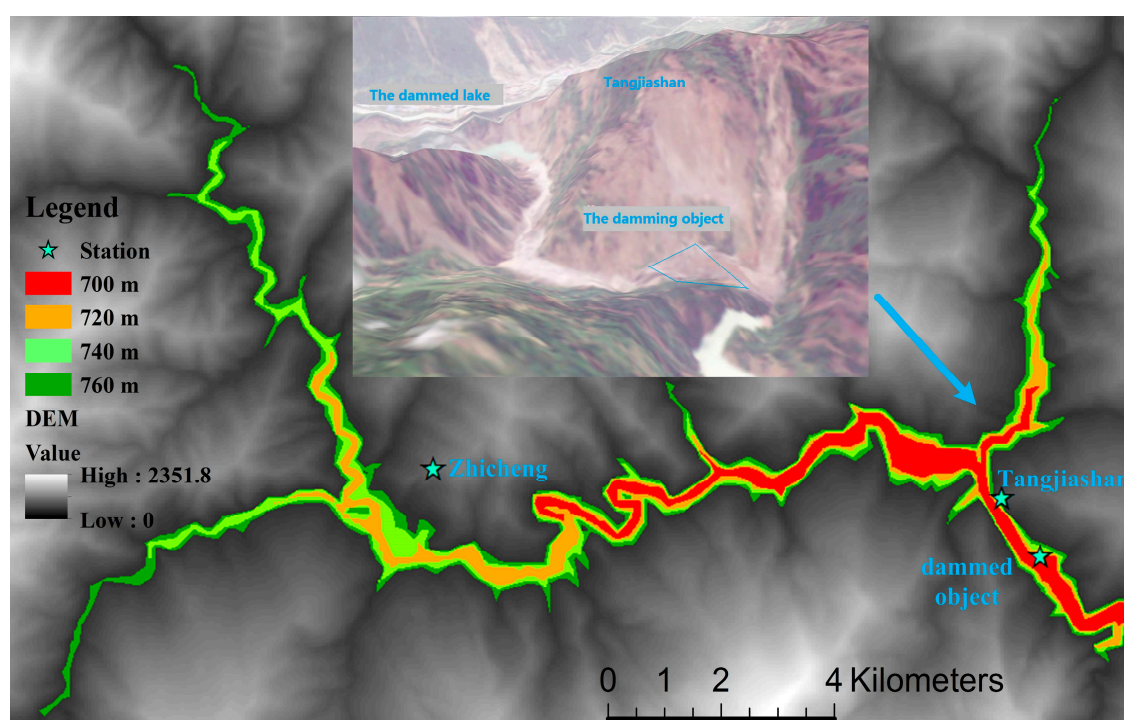


Figure 4. Dammed lake area in different water level.

4.2. Dam-Break Flow Calculation Result ($H = 700$ m)

The weir flow empirical formula (Equation (2)) and continuity equation (Equation (3)) were adopted to calculate dam-break flow. In this calculation example, the water elevation H was 700 m, and the weir bottom elevation was 600 m. We adopted an entire break mode and selected a trapezoidal break shape. The hydraulic calculation result corresponding with the above conditions is shown in Table 3.

Table 3. Dam-break flow calculation result ($H = 700$ m).

Dam-Break Time	Flood Peak	Peak Appearance Time	Ending Flow
1 h	83,620 m ³ /s	17 min	76 m ³ /s

The result demonstrates that the entire flood process finished in a short time (approximately 60 min), and that the flow suddenly decreased to the upstream natural inflow whose value adopted the observation average (76 m³/s). The dam-break period (approximately 17 min) can be neglected when compared with the time when the flood spreads in the channel. Meanwhile, the potential impact area concerns the flood peak; thus, we can select the dam-break flow maximum ($Q_0 = 83,620$ m³/s) as the initial inflow.

4.3. Flood Routing Result ($H = 700$ m) with an Improved Muskingum Method

4.3.1. Muskingum Parameters Calculation with the Simulated Annealing Method

The Muskingum Parameters were trained by abundant historical data acquired from hydrological observation stations (Tongkou and Xiangshui stations). When the cost function convergence error in the SA method was stable, the Muskingum parameters in Tongkou and Xiangshui achieved optimization solutions (Table 4).

Table 4. Muskingum parameters.

Reach	C ₀	C ₁	C ₂
Tangjiashan to Tongkou	0.1722	0.1615	0.6663
Tongkou to Xiangshui	0.5197	0.2819	0.1984

4.3.2. Validation of the Muskingum Parameters

The measuring runoff data after the earthquake was selected to validate the Muskingum parameters. The flood that occurred on 10 June 2008 was used as a test sample to check the accuracy of the Muskingum parameters. The Muskingum flood routing equation (Tangjiashan to Tongkou) is expressed as follows:

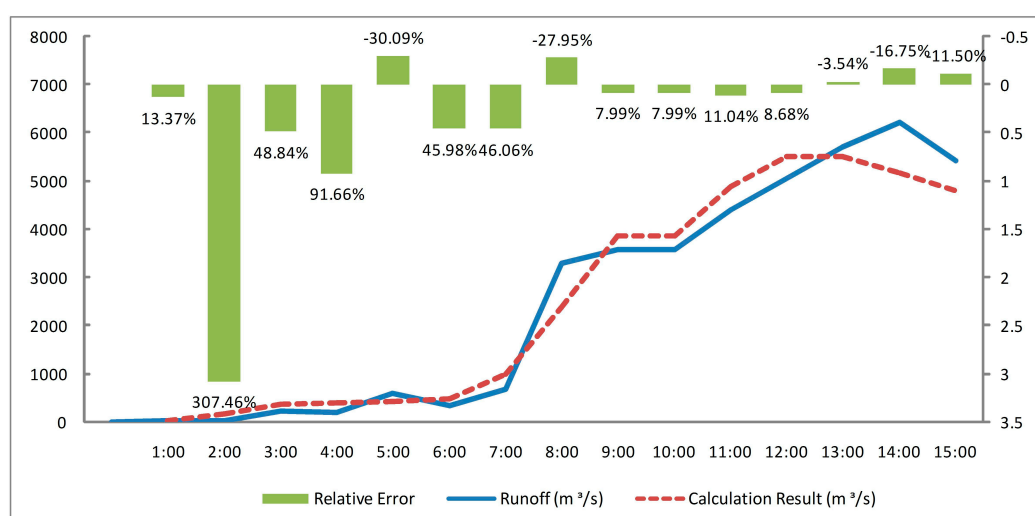
$$O_2 = 0.1722I_2 + 0.1615I_1 + 0.6663O_1 \quad (8)$$

The field survey data Q measuring at Tangjiashan station was considered as known inflow condition (I_2, I_1). The 23 m³/s value is viewed as the initial outflow condition (O_1) at Tongkou station. The outflow sequences could be calculated according to the Equation (8). Comparing outflow sequences with field survey data Q' measuring at Tongkou station, the coefficient test result is shown as below (Table 5 and Figure 5).

Table 5. Coefficient test result between Tangjiashan and Tongkou section.

Time (h) Tangjiashan	Water Level (m)	Runoff Q (m ³ /s)	Time (h) Tongkou	Water Level (m)	Runoff Q' (m ³ /s)	Flood Routing Result	Relative Error %
0:00	620.90	33.1	0:00	535.31	23		
1:00	621.05	64.3	1:00	535.36	28	31.743	+13.37%
2:00	622.75	858	2:00	535.52	44	179.282	+307.46%
3:00	622.25	576	3:00	537.19	240	357.210	+48.84%
4:00	621.93	415	4:00	537.1	210 *	402.496	+91.66%
5:00	622.03	465	5:00	538.27	594	415.279	-30.09%
6:00	622.62	780	6:00	537.33	333	486.114	+45.98%
7:00	625.90	3240	7:00	538.57	690	1007.800	+46.06%
8:00	629.54	6870	8:00	545.05	3300	2377.770	-27.95%
9:00	629.54	6870	9:00	545.65	3590	3876.827	+7.99%
10:00	629.54	6870	10:00	545.65	3590	3876.827	+7.99%
11:00	629.59	6930	11:00	547.16	4400	4885.981	+11.04%
12:00	629.23	6530	12:00	548.25	5060	5499.190	+8.68%
13:00	627.34	4640	13:00	549.20	5720	5517.713	-3.54%
14:00	626.94	4320	14:00	549.73	6210	5169.716	-16.75%
15:00	626.47	3800	15:00	548.80	5420	4796.622	+11.50%

Note: * Interpolation.

**Figure 5.** Tongkou station coefficient test result.

Similarly, the flood that occurred on 12 June 2008 was used to check the Muskingum parameters of Xiangshui section. The Muskingum flood routing equation (Tongkou to Xiangshui) is expressed as follows:

$$O_2 = 0.5197I_2 + 0.2819I_1 + 0.1984O_1 \quad (9)$$

The process of calculation is the same as the above. The data table is abbreviated because of the length of paper. The coefficient test result at Xiangshui station is shown in Figure 6.

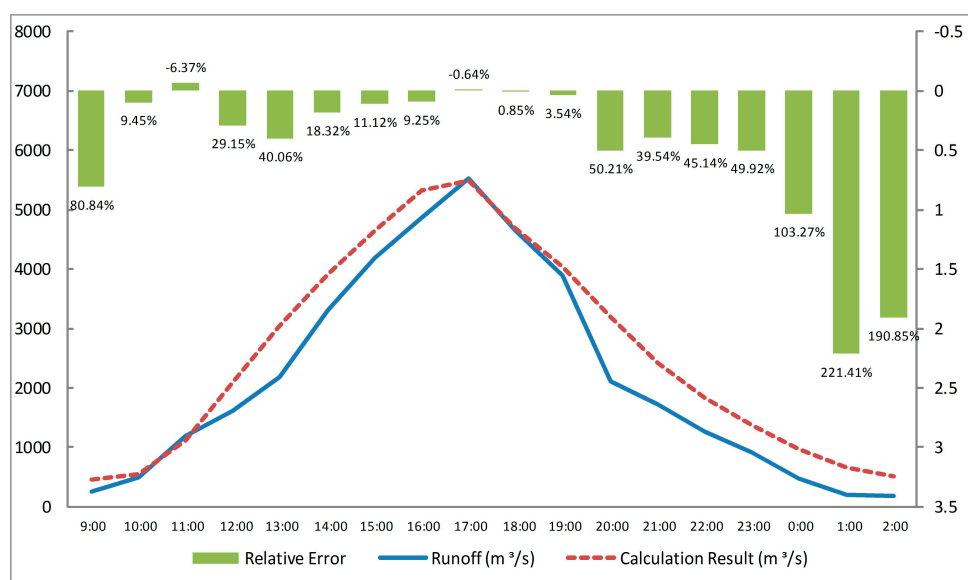


Figure 6. Xiangshui station coefficient test result.

Table 5 and Figures 5 and 6 show that the training parameters can ensure that total forecasting error is least when the cost function comprises a mean square deviation formula. The strategy of error control determines that a larger flow correlates with a higher weight impacting on the cost function. Thus, the peak flood flow is forecasted more exactly than the bottom of the flow. For example, the relative error of the peak flood flow is -0.64% , and is obviously more exact than the small flow in Figure 6. In this paper, our goal is to analyze the widest submerging area that corresponds with the peak flood flow. The outflow simulation is therefore appropriate for this study.

4.3.3. Flood Routing Result ($H = 700$ m)

Once a dammed lake is generated, it blocks the surface runoff, thus leaving only a small base flow in the river. The base flow primarily generated by groundwater is a small, stable value. It can be acquired by observation after the formation of a dammed lake (dammed lake $3.2 \text{ m}^3/\text{s}$, Tongkou $4 \text{ m}^3/\text{s}$, and Xiangshui $2 \text{ m}^3/\text{s}$). They are the initial inflow and outflow in Muskingum flood routing. The flood routing result ($H = 700$ m) is given in Tables 6 and 7 below.

Table 6. Muskingum flood routing between Tangjiashan and Tongkou.

Reach Parameters	I_2 (m^3/s)	I_1 (m^3/s)	O_1 (m^3/s)	O_2 (m^3/s)
Tangjiashan to Tongkou	83,620	3.2	4	14,402.6
$C_0 = 0.1722$	76	83,620	14,402.6	23,114.1
$C_1 = 0.1615$	76	76	23,114.1	15,426.3
$C_2 = 0.6663$	76	76	15,426.3	10,303.9
	76	76	10,303.9	6890.9
	76	76	6890.9	4616.7
	76	76	4616.7	3101.5
	76	76	3101.5	2091.9
	76	76	2091.9	1419.2

Table 7. Muskingum flood routing between Tongkou and Xiangshui.

Reach Parameters	I_2 (m ³ /s)	I_1 (m ³ /s)	O_1 (m ³ /s)	O_2 (m ³ /s)
Tongkou to Xiangshui	14,402.6	4	2	7486.6
$C_0 = 0.5197$	23,114.1	14,402.6	7486.6	17,557.8
$C_1 = 0.2819$	15,426.3	23,114.1	17,557.8	18,016.4
$C_2 = 0.1984$	10,303.9	15,426.3	18,016.4	13,278.1
	6890.9	10,303.9	13,278.1	9120.2
	4616.7	6890.9	9120.2	6151.3
	3101.5	4616.7	6151.3	4133.7
	2091.9	3101.5	4133.7	2781.6
	1419.2	2091.9	2781.6	1879.1

4.4. Estimation for Potential Impact Area ($H = 700$ m)

The runoff is plotted in Figure 7a according to the Muskingum flood routing results. When the runoff peak spreads through the Tongkou and Xiangshui stations, the submerged area achieves the maximum. The flow can be converted to a corresponding elevation by the runoff-versus-elevation curve (Table 8) acquired from survey data polynomial fitting. The conversion result shows that the highest impact elevation is 565.89 m in Tongkou and 537.47 m in Xiangshui.

Table 8. Runoff versus elevation curve in Tongkou and Xiangshui.

Hydrologic Stations	The Runoff (y) vs. Elevation (x) Curve	Flood Peak	Elevation Maximum
Tongkou	$y = 20.9186473861676x^2 - 22,283.2134587244x + 5,934,150.94321418$ $R^2 = 0.986247730187266$	23,114.1 m ³ /s	565.89 m
Xiangshui	$y = 7.26568228353629x^3 - 11,429.4504515713x^2 + 5,993,229.82457408x - 1,047,570,044.59771$ $R^2 = 0.997800010971559$	18,016.4 m ³ /s	537.47 m

Finally, the study adopted the distance interpolation method to deal with 700 m in dammed lake, 565.89 m in Tongkou, and 537.47 m in Xiangshui and to generate a series of dense elevations in all the segments along the river (Figure 7). All of the sections are manually divided by approximately equal distances and are vertical to the river channel. The potential impact area at every segment was generated by interpolating elevation values using the seed growing algorithm. The entire impact area was obtained by overlaying those segment results. To obtain a smooth junction, we used a smooth algorithm to deal with the entire impact area.

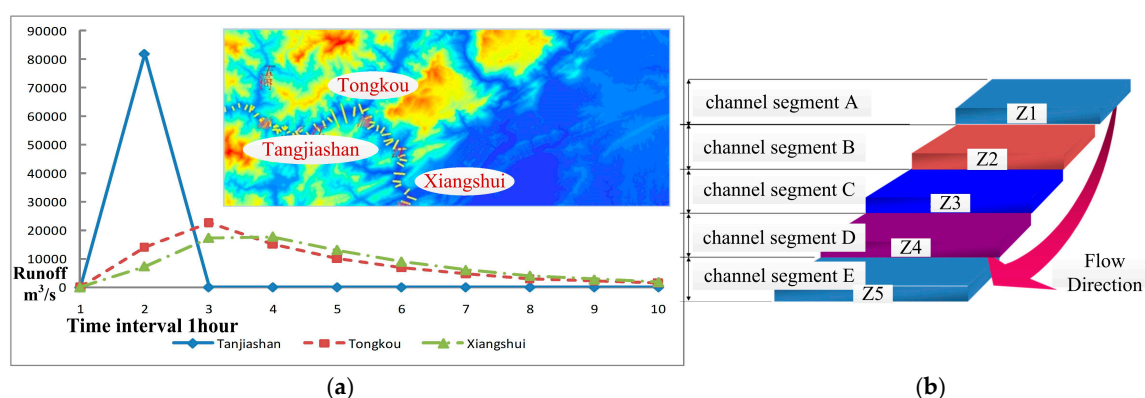


Figure 7. Muskingum flood routing results and elevation interpolation method. (a) Flood routing results based on Muskingum method and segments division; (b) The elevation interpolation along the river.

When the water elevation H was 700 m and the weir bottom elevation was 600 m, Figure 8 shows the potential impact area (smooth result) when adopting an entire break mode and selecting a trapezoidal break shape.

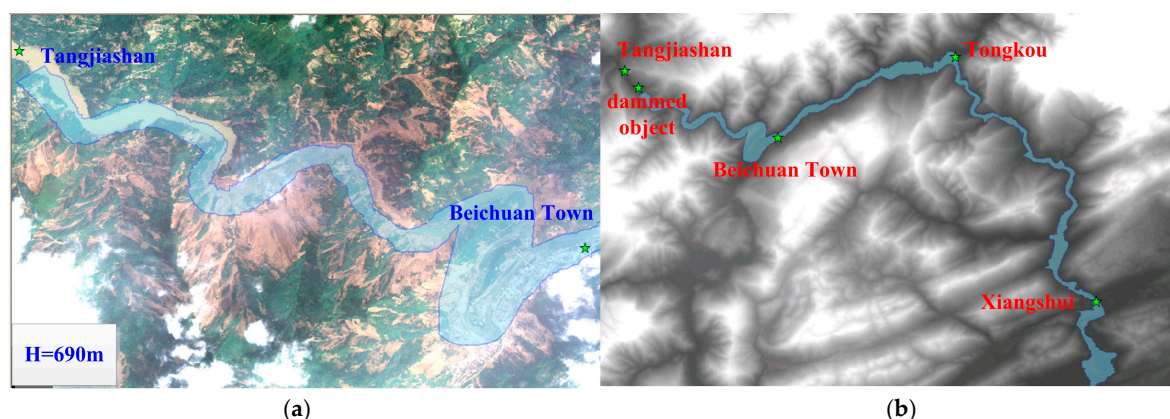


Figure 8. Potential impact area of the dammed lake downstream ($H = 700$ m). (a) Partial view of the submerged area; (b) Global view of the submerged area.

From the submerging result combining the DEM and RS images, we find that a given dam-break flood forming after an earthquake can destroy the entire Beichuan Town, which was already severely afflicted by the Wenchuan earthquake disaster. Based on downstream impact research, quickly estimating the loss of dam-break flood disasters is possible by using a GIS overlay analysis function that can generally analyze the population, economy, submerging depth, and other essential information.

5. Discussion

Usually, flood routing using hydraulics methods requires complicated grid calculation and various boundary conditions. Consequently, the hydraulics methods are time consuming and cannot provide a series of submerged areas corresponding to different water levels in a short time. The study in this paper obtained potential impact areas with scarce data resources (DEM, historical hydrological data, and current RS images). Furthermore, the improved Muskingum method could simplify the calculation process and significantly improve efficiency. It could rapidly forecast the downstream submerged area corresponding with different dam-break elevations.

According to the similar dam-break mode and calculation method, we assumed a group of dam-break elevations ($H = 700, 680, 660, 640$, and 620 m) in calculating the corresponding submerged area. The group of calculation results is shown in Figure 9. The result shows that the flood with the 680 m collapse elevation will impact the entire downstream region of Beichuan town. Several islands remained for the transfer route when the 660 m flood occurred.

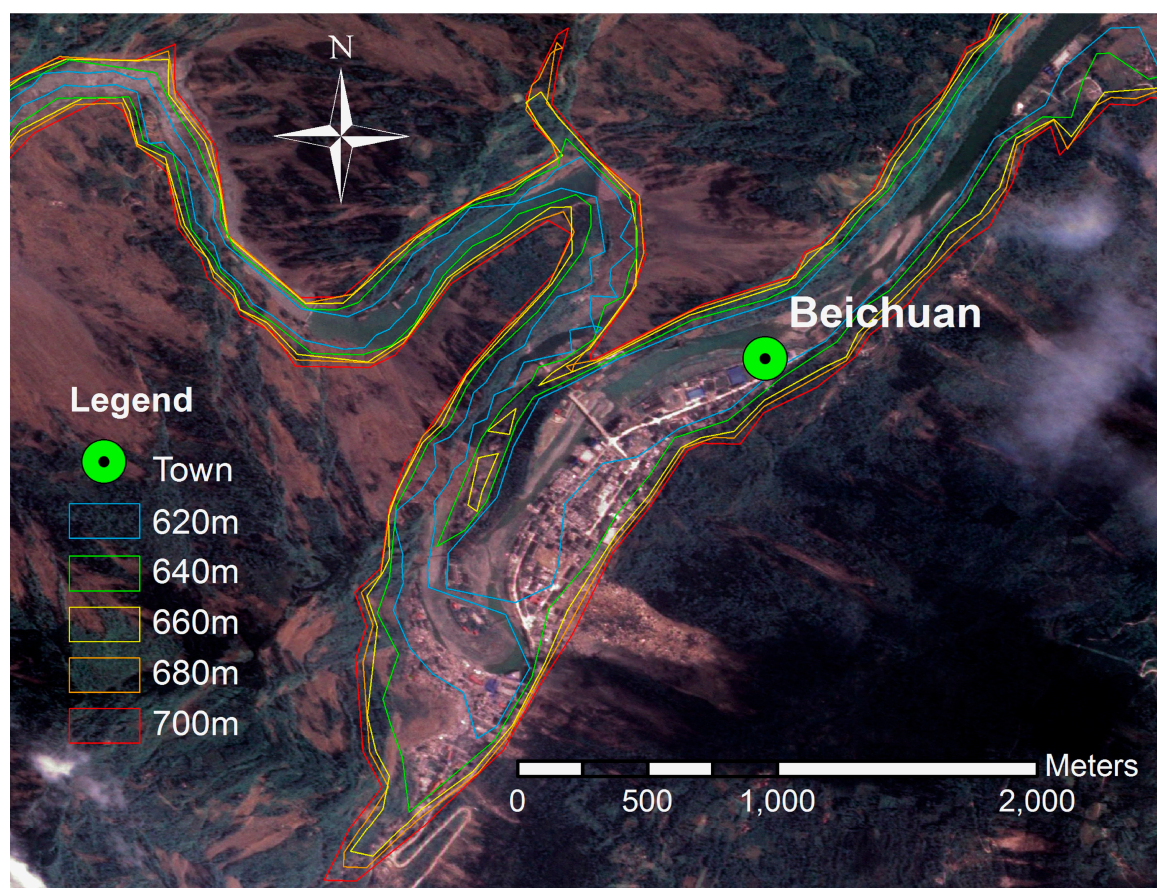


Figure 9. Potential impact area of dammed lake downstream ($H = 700, 680, 660, 640, 620$ m).

6. Conclusions

After an earthquake, the communication and transportation in a disaster region is cut off. Traditional land measurement cannot immediately provide first-hand estimation under such conditions. Accompanying the development of spatial techniques, rapidly estimating the impact of dammed lakes, one of the secondary disasters induced by earthquake, is possible. This paper proposed a method to respond immediately to dammed lakes and obtain their corresponding potential impact area by using a DEM, historical hydrological data, and current RS images. The proposed method can be used to survey the location, quantity, and scale of dammed lakes, and to estimate the potential submerging loss. The proposed method can forecast the upstream potential impact area according to different water levels and downstream potential impact areas according to different dam-break styles without requiring a field survey. Its time-efficiency is important in selecting transfer routes and generally assessing disaster loss. It is also helpful in providing rapid decision support to choose reasonable schemes and to effectively decrease the harm of dammed lakes. Research on rapid estimation for potential impact areas of earthquake-induced dammed lakes can satisfy the requirements of dammed lake management.

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