

Article

Impact of the Depth of Diaphragm Wall on the Groundwater Drawdown during Foundation Dewatering Considering Anisotropic Permeability of Aquifer

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Abstract: Foundation dewatering combined with a waterproof curtain is widely applied to ensure the safety of the foundation pit in areas with multi-aquifer–aquitard alternative strata. The buried depth of the diaphragm wall can influence the environmental effect due to dewatering obviously. This paper investigates the impact of the buried depth of the diaphragm wall on the groundwater drawdown considering the anisotropic permeability of the dewatering aquifer. Numerical simulation is conducted based on an engineering case. The ratio of penetrating depth of diaphragm wall to thickness of dewatering aquifer (R_w) and the ratio of horizontal and vertical hydraulic conductivity of dewatering aquifer (R_c) are varied. The relationship between approximate hydraulic gradient (Δi) and R_w (or R_c) can be fitted by Boltzmann curve (or logarithmic curve). Effective, suggested and control values of R_w (or R_c) are proposed, of which the suggested value is recommended in practical engineering. The effective, suggested and control value of R_w can be calculated by logarithmic equation considering the value of R_c .

Keywords: diaphragm wall; anisotropic permeability; foundation dewatering; groundwater drawdown; penetrating depth

Citation: Wang, X.-w.; Xu, Y.-s. Impact of the Depth of Diaphragm Wall on the Groundwater Drawdown during Foundation Dewatering Considering Anisotropic Permeability of Aquifer. *Water* **2021**, *13*, 418. <https://doi.org/10.3390/w13040418>

Academic Editor: Maurizio Barbieri

Received: 26 December 2020

Accepted: 2 February 2021

Published: 5 February 2021

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1. Introduction

The foundation pit is one major kind of underground structure in the massive construction of underground space [1–4]. Gradually increased excavation area and depth with the development of urban construction may result in environmental effects. There are high risks during construction in multi-aquifer–aquitard alternative strata which is rich in groundwater [5–9]. To prevent water inrush accidents and ensure the safety of underground construction, groundwater control is essential and dewatering is a kind of common measurement [10–12]. However, inappropriate dewatering may result in some negative impacts, such as soil deformation [13–15], differential settlement [16–18] and sand flow or piping phenomenon [19]. To reduce the negative impacts of dewatering, waterproof curtains such as diaphragm walls [20,21], bored piles [22] and mixing piles [23] are constructed before the excavation. Waterproof curtains play a good role in blocking water seepage and extending the length of seepage distance [24,25], which can reduce the drawdown of groundwater level and ground settlement outside the pit.

The buried depth of the waterproof curtain is essential for the environmental effect outside the pit caused by foundation dewatering. Theoretically if the waterproof curtain fully cuts off the dewatering aquifer, which is called full penetrating waterproof curtain, the groundwater drawdown outside the pit is very small and the environmental effect

caused by the foundation dewatering is slight. However, with the increase of the excavation depth, the depth of the watertight curtain also increases. Full penetrating waterproof curtains are faced with high construction cost and construction difficulty. Therefore, waterproof curtains that partially cut off the dewatering aquifer, called partial penetrating waterproof curtains, are always chosen in practical engineering [26–30].

Anisotropic permeability widely exists in rock [31], coal mines [32] and soil layers [33]. The degree of anisotropic permeability of the aquifer can be reflected by the ratio of horizontal and vertical hydraulic conductivity of the aquifer. The anisotropic permeability of the aquifer is different in different fields. For example, the value of anisotropic permeability always ranges from 2 to 4 and even as high as 10 in Shanghai [26,34], from 3 to 6 in Tianjin [35] and from 4 to 10 in Ningbo [36,37]. The anisotropic permeability of the dewatering aquifer may influence the groundwater seepage inside and outside the pit during dewatering with a partial penetrating waterproof curtain. Some research also indicated that the buried depth of the waterproof curtain and the anisotropic permeability of the aquifer can both impact the environmental effect outside the pit [38,39]. The optimized method has been proved effectively to analyze the soil parameters [40,41], and it also can be used for considering the combination of these two factors in the design of dewatering. The objective of this paper is to evaluate the recommended depth of the waterproof curtain penetrating the dewatering aquifer considering the degree of anisotropic permeability based on an engineering case in Shanghai, China. Firstly, the project description is introduced. Secondly, a numerical model is established and the pumping test is used to correct the numerical model. Then, the effect of dewatering is investigated by considering the depth of waterproof curtain penetrating the dewatering aquifer and the degree of anisotropic permeability. Finally, the suggested depth of waterproof curtain is proposed via considering the anisotropic permeability.

2. Project Description

2.1. Engineering Overview

Figure 1 presents a plan view of the foundation pit of a certain metro line station in Shanghai. The pit is divided into two parts: the standard part (Zone-I) and the shield end well part (Zone-II). The length of Zone-I and Zone-II is 85.1 m and 16.3 m, and the width is 22.7 m and 27.1 m, respectively. The excavation depth (D_E) of Zone-I is 17.44 m and that of Zone-II is 19.50 m. A diaphragm wall with a thickness of 0.8 m is constructed as the waterproof curtain. The buried depth of the diaphragm wall (D_W) in Zone-I is 30.8 m, and 34.2 m in Zone-II. Four pumping wells inside the pit and two observation wells outside the pit are arranged, which is shown in Figure 1.

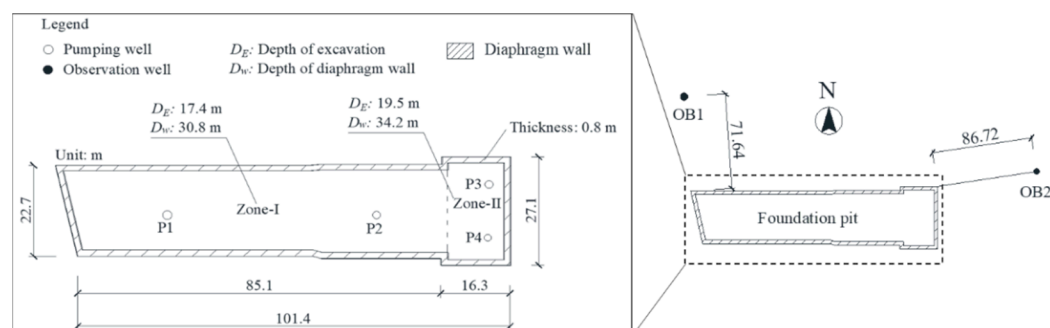
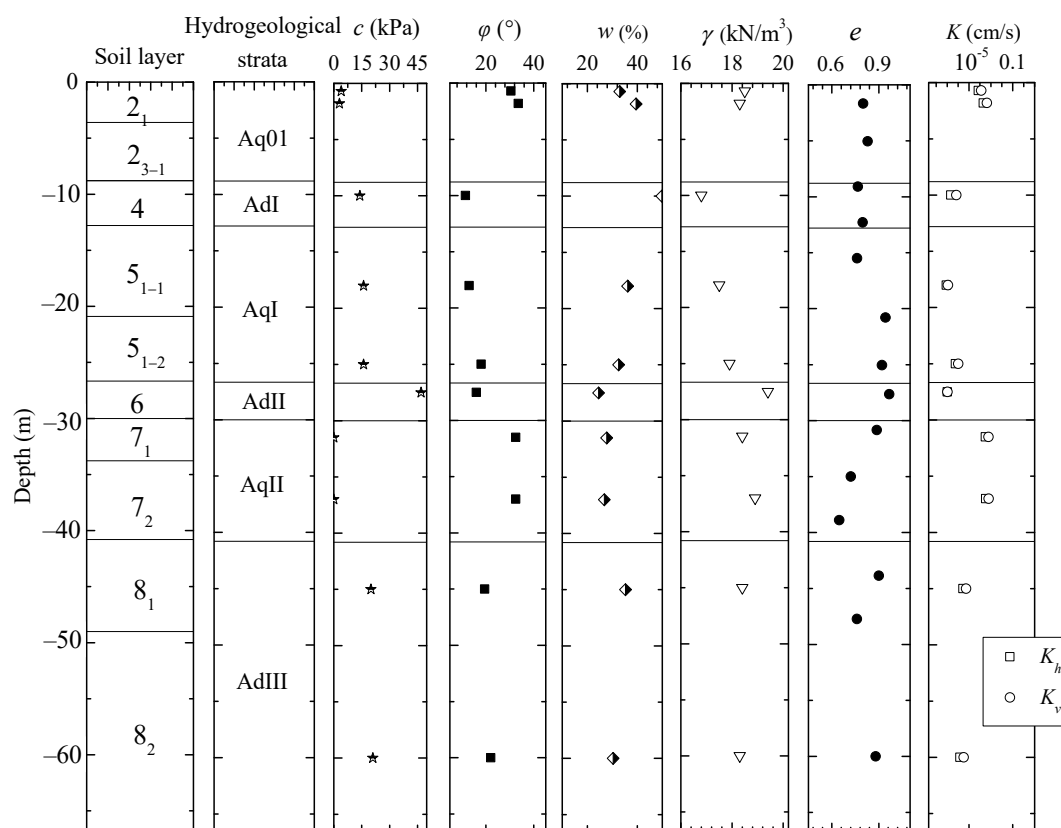


Figure 1. Plan view of foundation pit and layout of pumping and observation wells (recreated based on [42]).

2.2. Engineering Geology and Hydrogeology

The soil layers from top to bottom are sandy silt with silty clay (labelled as 2_1), sandy silt (2_{3-1}), mucky clay (4), silty clay (5_1), silt (5_2), silty clay (6), mealy sand (7_1), fine sand (7_2), silty clay with mealy sand (8_1) and silty clay (8_2). The soil profile and properties of soil layers are presented in Figure 2.



Note: c = cohesion; ϕ = internal friction angle; γ = unit weight; e = void ratio; w = water content; K_h , K_v = Horizontal and vertical hydraulic conductivity; Aq = Aquifer; Ad = Aquitard

Figure 2. Soil profile and properties of soil layers.

The hydrogeology of the project is multi-aquifer–aquitard alternative strata. The aquifer system of the project includes a phreatic aquifer (labelled as Aq01), and a confined aquifer I (labelled as AqI) and II (AqII). The aquitard layer is labelled as AdI to AdIII. Detailed information of the aquifers is described as follows:

(1) Aq01: the burial depth of the groundwater level of Aq01 is -0.5 to -1.0 m (a negative value implies that it is below ground surface); it lies within the Layers 2_1 and 2_{3-1} and it is affected by rainfall, spring tides and surface water. The water inflow rate of a single well is 0.48 – 30 m³/d, and the hydraulic conductivity is 0.01 – 0.85 m/d;

(2) AqI: the burial depth of the groundwater level of AqI layer is -10.0 to -3.0 m and it lies within the Layers 5_1 and 5_2 . The water inflow rate of a single well is 5 – 15 m³/d, and the hydraulic conductivity is 0.26 – 0.78 m/d;

(3) AqII: the burial depth of the groundwater level of AqII layer is -6.0 to -4.0 m; it lies within the Layers 7_1 and 7_2 . The water inflow rate of a single well is 36 – 48 m³/d, and the hydraulic conductivity is 0.18 – 4.1 m/d [43].

2.3. Pumping Test

To prevent uprush by the underlying confined aquifer, groundwater level inside the pit should be lowered which can be calculated by the following equation [44]:

$$\frac{P_s}{P_w} = \frac{\sum h_i \times \gamma_{si}}{h_p \times \gamma_w} \geq F_s \quad (1)$$

where P_s is the overburden pressure between the bottom surface of the foundation pit and the top surface of the underlying confined aquifer (kPa); P_w is the uplift force of artesian water in the initial state (kPa); h_i is the thickness of each layer of soil between the bottom surface of foundation pit and top surface of the underlying confined aquifer (m); h_p is the difference between the groundwater level and top surface of the confined aquifer (m); γ_{si} is the unit weight of each soil layer between the bottom surface of the foundation pit and the top surface of the underlying confined aquifer (kN/m³); γ_w is the unit weight of water (kN/m³); and F_s is the safety coefficient, which is considered as 1.10 in this study [44].

In this case, drawdown of AqII in Zone-I and II should be larger than 3.38m and 7.14m. To ensure the safety of construction and evaluate the dewatering effect on environment, the pumping test was conducted after the construction of the diaphragm wall. The placement of the pumping and observation wells are shown in Figure 1 and the structure and burial depth of the wells are shown in Figure 3. The pumping test contains two steps of dewatering, of which the pumping time and discharge rate is shown in Table 1.

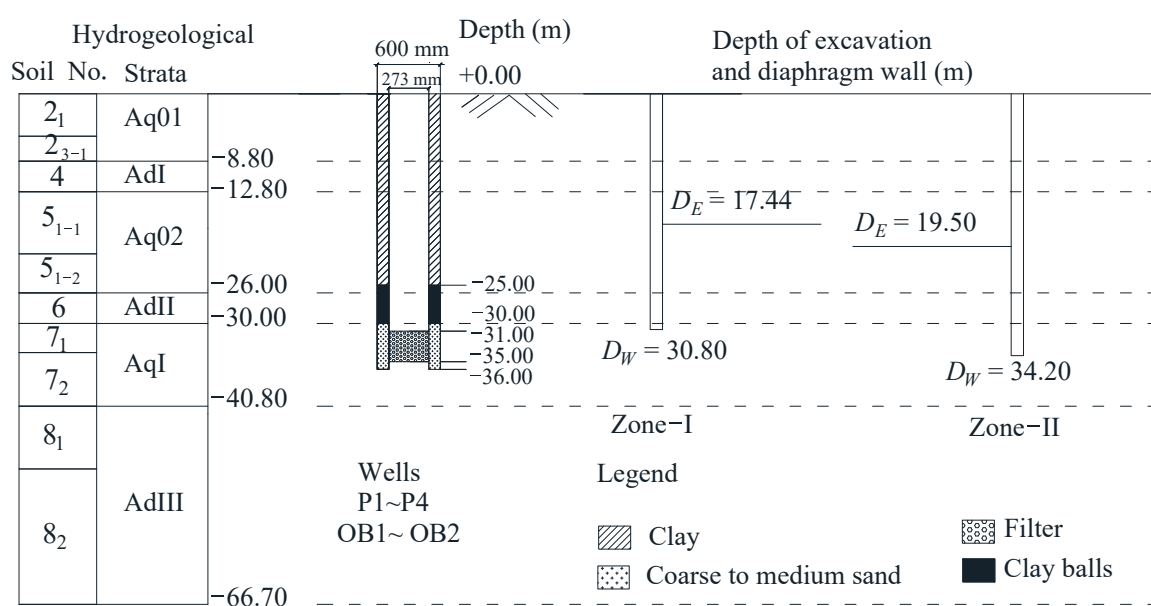


Figure 3. Structure and burial depth of wells and diaphragm wall.

Table 1. Arrangement of pumping test.

Pumping Well	Observation Well	First-Step Dewatering		Second-Step Dewatering	
		Pumping Time (d)	Discharge Rate (m³/d)	Pumping Time (d)	Discharge Rate (m³/d)
P1, P2	OB1, OB2	0–3	169.1	12–15	223.5
		3–11	42.3	15–27	61.3
P3, P4		0–3	96.3	12–15	229.7
		3–11	30.6	15–27	41.9

3. Numerical Analysis

3.1. Numerical Theory

The calculation method of software in the numerical simulation is a kind of finite different method [13,26,45], which is three-dimensional (3D) groundwater seepage model. The basic equation of 3D groundwater seepage model is:

$$\frac{\partial}{\partial x_i} \left(K_{ij} \frac{\partial H}{\partial x_j} \right) - q = S_s \frac{\partial H}{\partial t} \quad (2)$$

where K_{ij} = hydraulic conductivity of different direction, i, j = axes of x, y, z in Cartesian coordinate system, H = hydraulic head of groundwater, q = external source/sink flux, t = time, and S_s = specific storage, $S_s \approx \gamma_w m_v$, γ_w = unit weight of water, m_v = soil coefficient of volume compressibility. Based on Terzaghi's 1D consolidation theory, if the total vertical pressure is constant during the withdrawal or recharge of groundwater from an aquifer, the following equation could be applied:

$$\Delta H = - \frac{\Delta \sigma'}{\gamma_w} \quad (3)$$

where $\Delta \sigma'$ = change of effective stress.

3.2. Model Setup

The plan size of the numerical model is required to be larger than the influence radius of dewatering. The length and width of the numerical model is 1670 m, and the depth is 66.7 m, which is the buried depth of the bottom face of layer AdIII. Figure 4 shows the three-dimensional model domain and the grid mesh. The number of nodes and elements in each plane is 1126 and 1148, respectively. The mesh size inside the foundation pit is 5 m × 5 m, and that is gradually enlarged to about 100 m × 100 m at the boundary of the model. The plan view of local enlarged mesh inside the foundation pit is presented in Figure 5. Figure 6 shows the profile of section II-II. The soil layer related to the construction is subdivided into 21 layers vertically. The total number of nodes and elements of the numerical model are 24,772 and 24,108, respectively.

The placement of the diaphragm wall, pumping and observation wells is shown in Figure 5 according to the actual engineering case. Points G11, G12, G21 and G22 were added for further discussion of the simulation results. The initial groundwater level of Aq01, AqI and AqII layer is −0.5 m, −1.0 m and −2.8 m respectively. The groundwater level of four lateral boundaries is set as fixed hydraulic head boundary, which is equal to the initial groundwater level. The bottom boundary is set as the confining boundary.

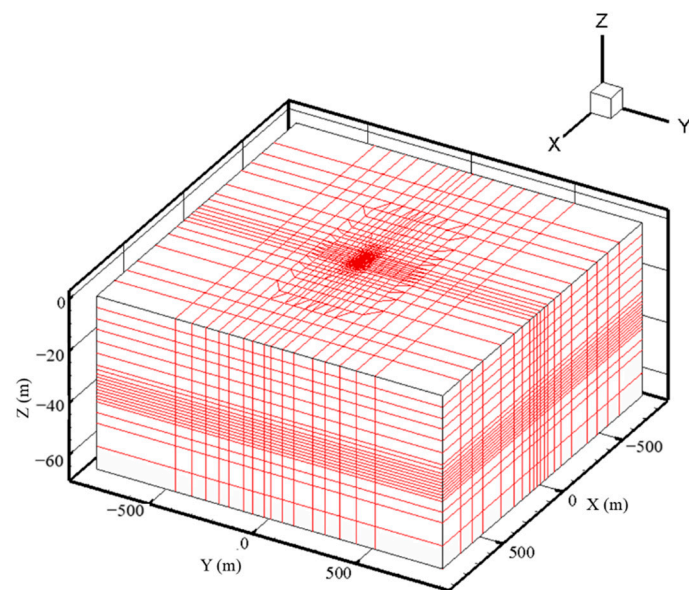


Figure 4. Three-dimensional model domain and the grid mesh.

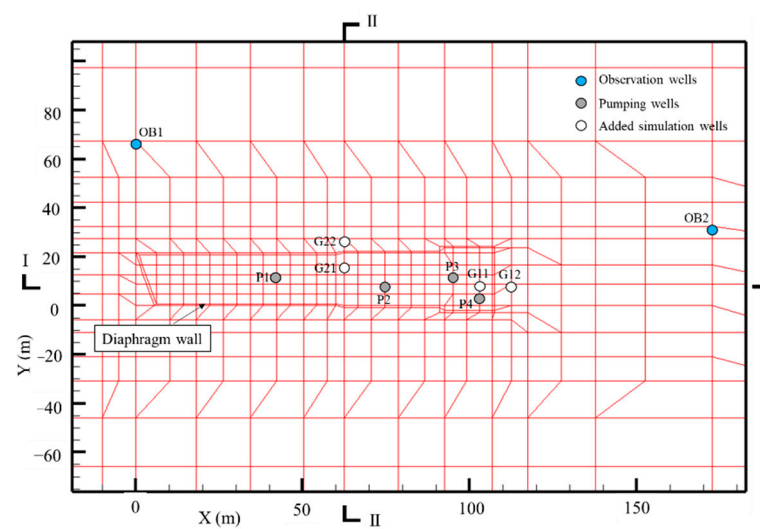


Figure 5. Plan view of local enlarged mesh inside foundation pit.

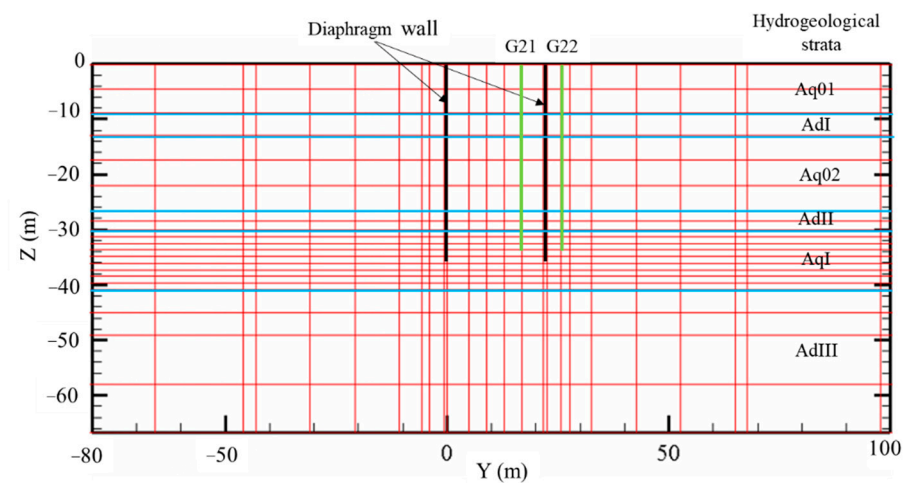


Figure 6. Profile of section II-II.

3.3. Soil Parameters

Initial soil parameters in Figure 2 are applied in the numerical model to simulate the pumping test. Then the soil parameters are inversely fitted to the results of the simulation to the observed data. When the results fit well enough and the maximum deviation between the measured data and simulated data is less than 5%, the parameters are thought to be appropriate for use in the simulation process. The final soil parameters determined by the aforementioned steps are tabulated in Table 2.

Table 2. Parameters used in numerical simulation.

No.	Hydrogeological Strata	Thickness (m)	γ (kN/m ³)	e	K_h (m/d)	K_v (m/d)	S_s (m ⁻¹)
1	Aq01	8.8	19	0.80	4.00×10^{-3}	2.00×10^{-3}	3.50×10^{-3}
2	AdI	4	17.4	1.25	2.32×10^{-3}	9.05×10^{-5}	8.51×10^{-5}
3	Aq02	13.2	18.9	0.84	5.80×10^{-2}	6.70×10^{-3}	1.15×10^{-3}
4	AdII	4	19.4	0.94	8.80×10^{-5}	1.20×10^{-5}	1.67×10^{-4}
5	AqI	10.8	18.9	0.77	4.91	2.45	3.14×10^{-3}
6	AdII	25.9	18.3	0.92	2.32×10^{-3}	9.90×10^{-4}	8.01×10^{-3}
Diaphragm wall					1.00×10^{-10}	1.00×10^{-10}	1.00×10^{-9}

3.4. Model Verification

The comparison between measured data and simulated data is shown in Figure 7. As shown in Figure 7, the simulated groundwater drawdown decreases synchronously with the measured data. For the first step of dewatering, groundwater drawdown of observation wells OB1 and OB2 was 1.93 m and 1.74 m respectively, and that for simulated data was about 1.99 m and 1.72 m, of which the deviation is 3.1% and 1.2%. For the second step of dewatering, the final drawdown of wells OB1 and OB2 was 3.23 m and 3.20 m, and the simulated data was 3.30 m and 3.22 m. The deviation of the final drawdown was 2.2% and 0.63% respectively. These results demonstrate that the simulated results fit the measured data reasonably.

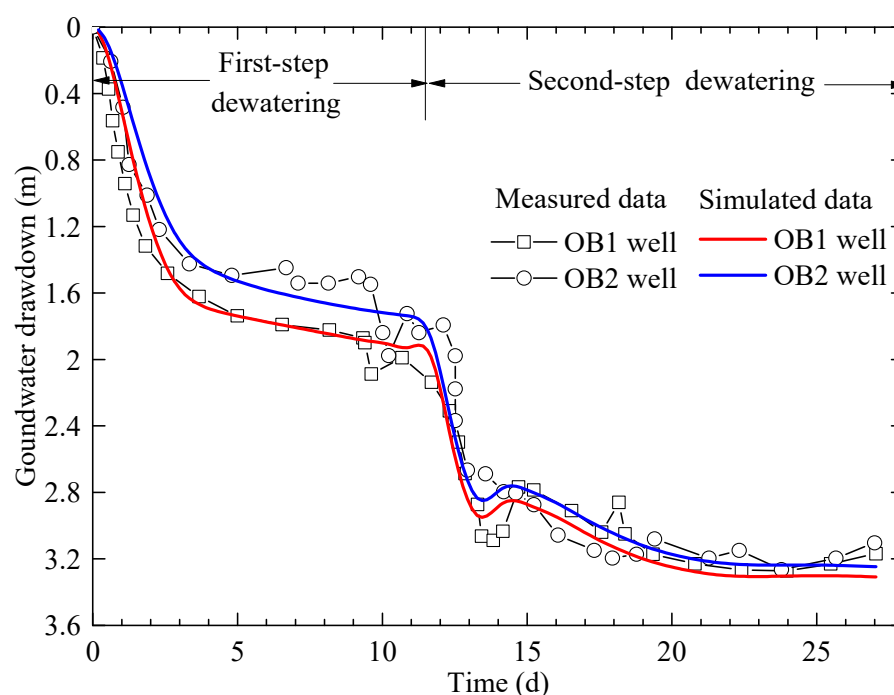


Figure 7. Comparison of measured and simulated groundwater level of observation wells in pumping test.

3.5. Simulation Results

To analyze the effect between the buried depth of the diaphragm wall and the anisotropic permeability of the dewatering aquifer, the penetrating depth of the diaphragm wall into the dewatering aquifer (D) and the ratio of horizontal and vertical hydraulic conductivity in dewatering confined aquifer (R_c) are varied. The thickness of the dewatering confined aquifer (T_a) is set as 10.8 m in this case. D ranges from 0 to 10.8 m with an increment of 1.2 m, and R_c changes from 1 to 10 with an increment of 1.

Figure 8a shows the groundwater drawdown and ground settlement with different values of D when R_c equals 2. Groundwater drawdown outside the pit is obviously smaller than that inside the pit. The groundwater drawdown and ground settlement after dewatering of point G12, which is 5 m away outside the diaphragm wall is shown in Figure 8b. With the increasing of D , groundwater drawdown at point G12 decreases from 5.03 m ($D = 0$ m) to 3.53 m ($D = 9.6$ m). Ground settlement at point G12 decreases from 19.5 mm ($D = 0$ m) to 13.70 mm ($D = 9.6$ m). Both groundwater drawdown and ground settlement decrease gently initially and then change quickly.

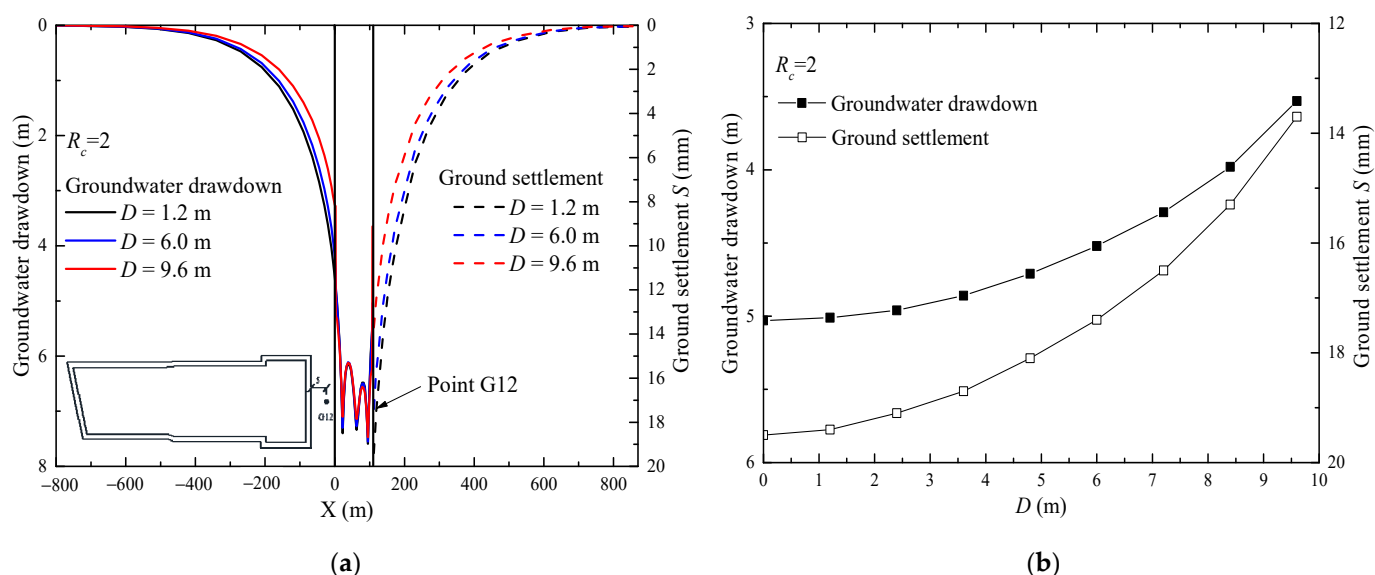


Figure 8. Groundwater drawdown and ground settlement with different value of D : (a) Section I-I; (b) Point G12.

Figure 9a presents the groundwater drawdown and ground settlement with different value of R_c when D is 4.8 m. Groundwater drawdown at point G12 is 4.71 m when R_c is 2 and 4.25 m when R_c is 10. Due to the decrease of vertical hydraulic conductivity, water supply from the boundary is more difficult which results in the decrease of groundwater drawdown and reduction of ground settlement outside the pit. The ground settlement at point G12 is 18.6 mm when R_c is 1 and 15.7 mm when R_c is 10. Moreover, comparing the change of groundwater drawdown and ground settlement outside the pit under different value of D and R_c , the influence of D on the groundwater drawdown outside the pit is much larger than that of R_c .

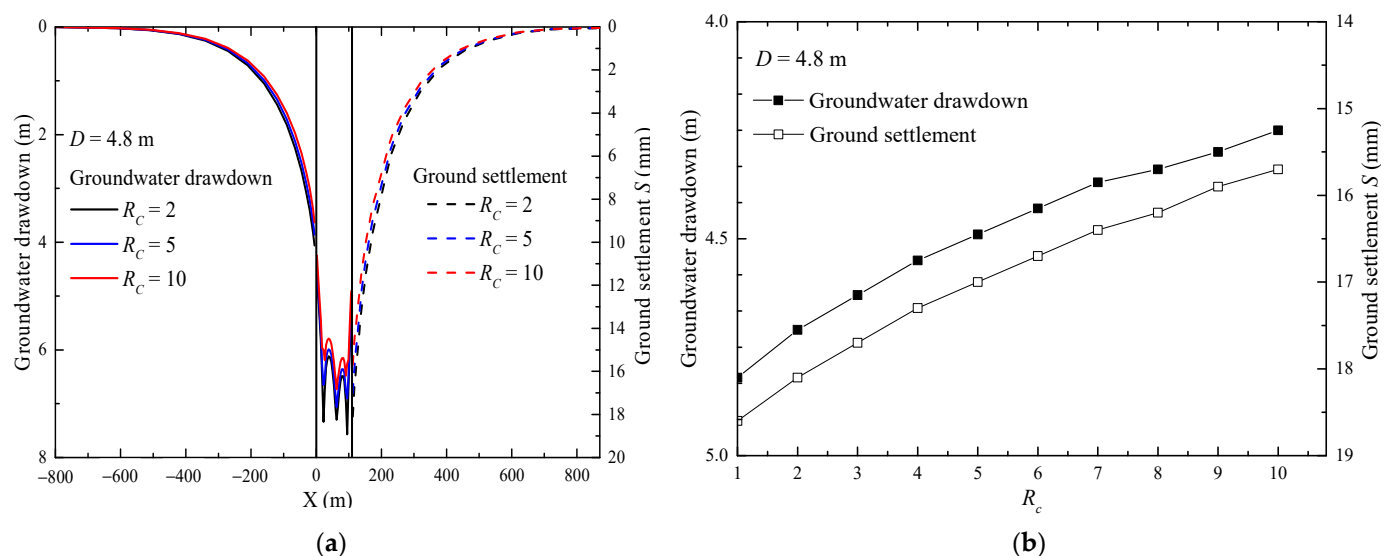


Figure 9. Groundwater drawdown and ground settlement with different value of D : (a) Section I-I; (b) Point G12.

4. Discussion

4.1. Groundwater Drawdown at Two Sides of Diaphragm Wall

G11 with G12, and G21 with G22 are two groups of points at two sides of the diaphragm wall, of which the distance from diaphragm wall is 5 m. Figure 10 shows the groundwater drawdown at two sides of the diaphragm wall when R_c is 10 with different R_w , which is defined as the ratio of D to T_a .

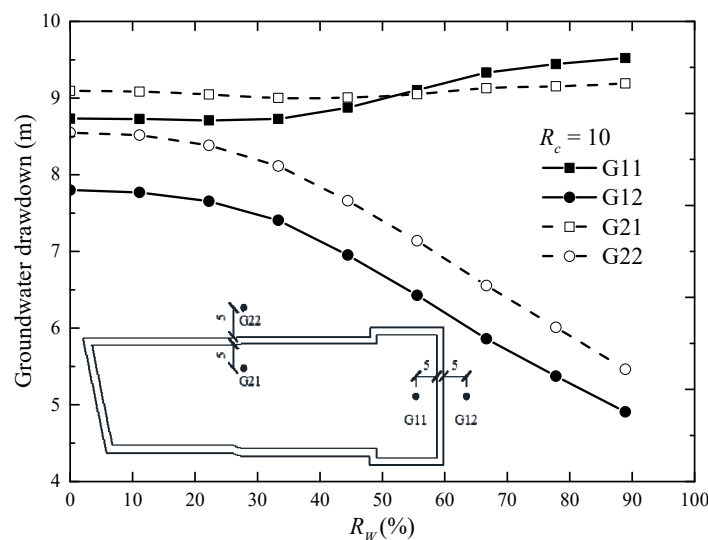


Figure 10. Groundwater drawdown at two sides of diaphragm wall.

With the increasing of R_w , the seepage path lengthens and the blocking effect is more. Therefore, the supply volume from outside to inside the pit decreases, which results in decreased drawdown outside the pit obviously. Due to the drawdown inside the pit being required to stay the same, the drawdown inside the pit (G11 and G21) varies slightly. For example, the drawdown of G11 ranges from 9.52 m ($R_w = 88.9\%$) to 8.73 m ($R_w = 0$), and the variation is 0.79 m. Drawdown of G12 ranges from 4.91 m ($R_w = 88.9\%$) to 7.80 m ($R_w = 0$), and the variation is 2.89 m. Moreover, the effect of dewatering is dissymmetry in the rectangle foundation pit. The ground drawdown outside the pit along the short side (e.g., G12) is larger than that along the long side (e.g., G22) [46,47].

Approximate hydraulic gradient at two sides of the diaphragm wall (Δi) is defined as the quotient of the division of the difference in groundwater drawdown by seepage distance between the group of observation wells outside and inside the pit. Due to the seepage distance between G11 and G12 the well is a curve bypassing the bottom of the diaphragm wall, which is difficult to simulate accurately. Therefore, approximate seepage distance is calculated as the polyline length, which starts from the middle of the well filter outside the pit to the bottom of the diaphragm wall, and then to the middle of the well filter inside the pit. In the discussion process, Δi between observation well G11 and G12, and G21 and G22 is calculated.

4.2. Penetrating Depth of Diaphragm Wall

Figure 11 presents the relationship between Δi and R_w when R_c changes from 1 to 10. All the curves can be fitted by the Boltzmann curve, which is widely used in the simulation of different fields [48,49] and can be divided into three parts: initial gradual part (Part-I), middle sharp part (Part-II) and final gentle part (Part-III). With the increasing of R_w , Δi increases gradually, due to the blocking effect of the diaphragm wall on the groundwater seepage being stronger. When D is larger than the filter length of pumping well, drawdown inside the pit changes little while drawdown outside decreases sharply, and this results in the sharp increase of Δi in Part-II. When R_w keeps increasing, since the difference of drawdown at two sides of diaphragm wall keeps constant while the seepage distance is much longer, Δi increases gently at the Part-III.

However, the fitted curve is different when comparing R_c ranges from 4 to 10 with R_c ranges from 1 to 3. Part-III is not so specific when R_c ranges from 1 to 3, since the permeability anisotropy is not obvious enough. Vertical hydraulic conductivity is close to the horizontal conductivity and the water supply from outside is relatively easy, which shortens the seepage distance relatively. When R_c ranges from 4 to 10, Part-III can be distinguished obviously, due to the difficulty of vertical seepage.

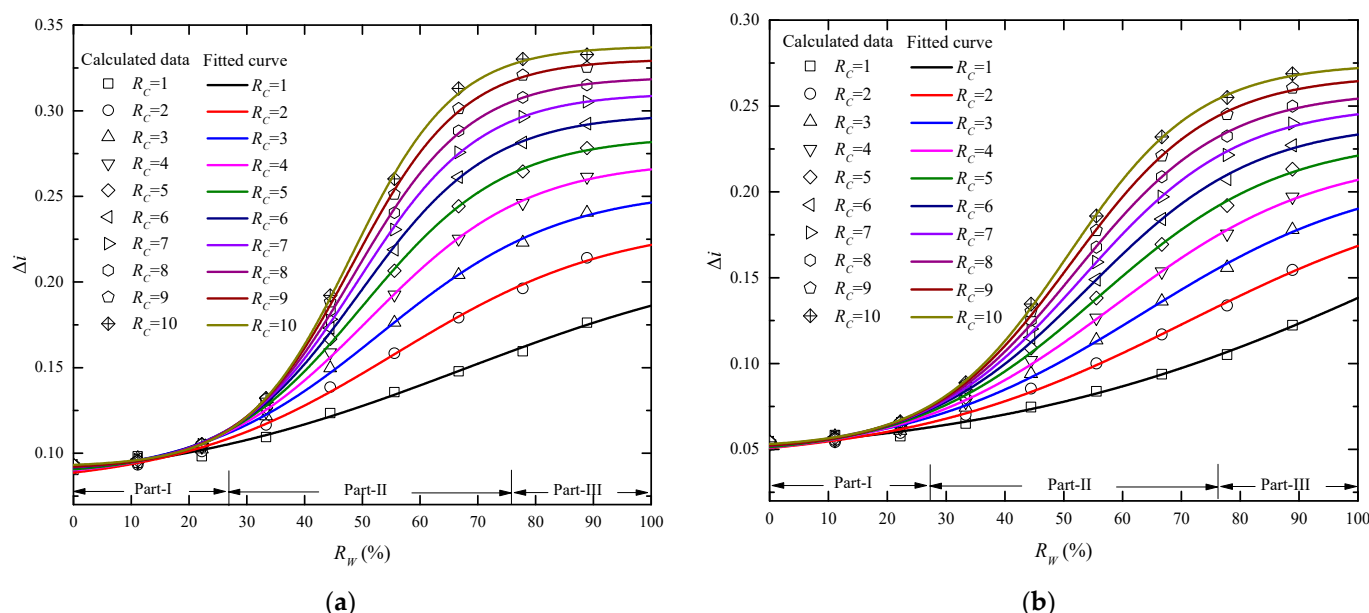


Figure 11. Relationship between Δi and R_w when R_c changes from 1 to 10: (a) section I-I; (b) section II-II.

Figure 12 presents the relationship between Δi and R_w when R_c is 10 on section I-I and II-II, which is fitted by the Boltzmann curve. The x-coordinate of the maximum and minimum value of the second derivative of the curve is defined as the effective and control value of R_w , which are the demarcation points of the three parts. With the increase of R_w , Δi also increases due to the blocking effect of the diaphragm wall. Since D_w is above the

bottom of the pumping wells filter, the increase of Δi is gradual. When D_w is over the effective value, Δi increases quickly, and the x-coordinate of the largest acceleration point (the contra-flexure point) is defined as the suggested value. If R_w is over the control value, drawdown at G11 increases and that at G12 decreases, while the seepage distance also increases and this results in the little increment of Δi .

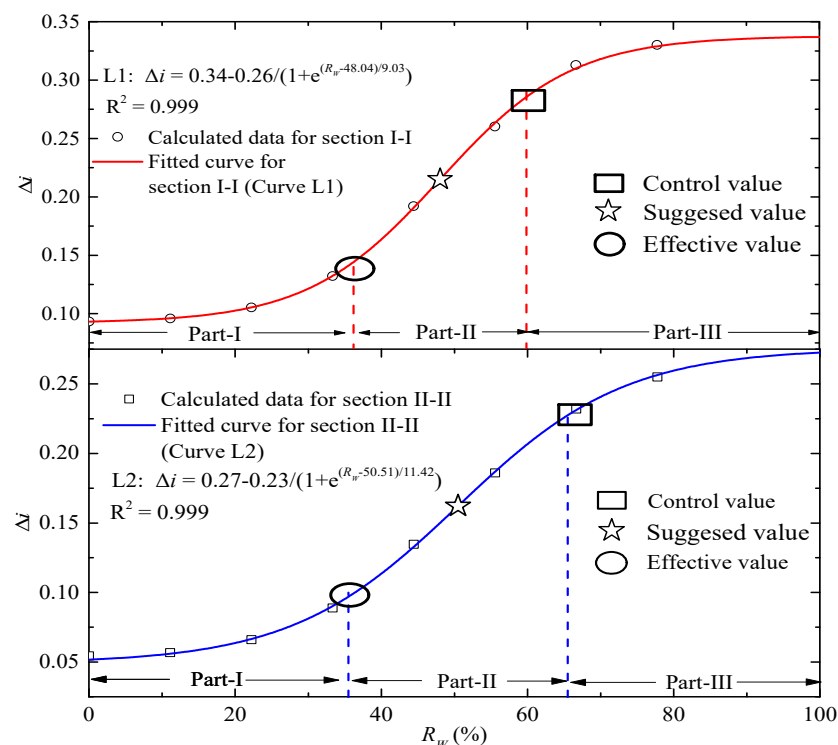


Figure 12. Relationship between Δi and R_w when R_c is 10.

As is shown in Figure 12, the effective, suggested and control value of R_w is 36.2%, 48.0% and 59.9% for section I-I, and 35.9%, 50.6% and 65.4% for section II-II, respectively. By a comprehensive consideration of Δi on two sections, the effective, suggested and control value of R_w is 37%, 51% and 66% when R_c is 10.

4.3. Ratio of Horizontal and Vertical Hydraulic Conductivity

The relationship between Δi and R_c when R_w changes from 0 to 88.89% is shown in Figure 13. Δi increases with the increase of R_c at all curves, because the permeability in vertical direction reduces and water supply from outside to inside is less. Therefore, the difference value of drawdown on the two sides of the diaphragm wall increases. When R_w is less than 33.33%, the increment of Δi is slight and the acceleration is slow. Since the buried depth of the diaphragm wall is above the filter of the pumping wells, the blocking effect is not obvious and drawdown at the two sides of the diaphragm wall varies little. When R_w is larger than 33.33%, the blocking effect of the diaphragm wall is obvious and the variation of Δi increases obviously.

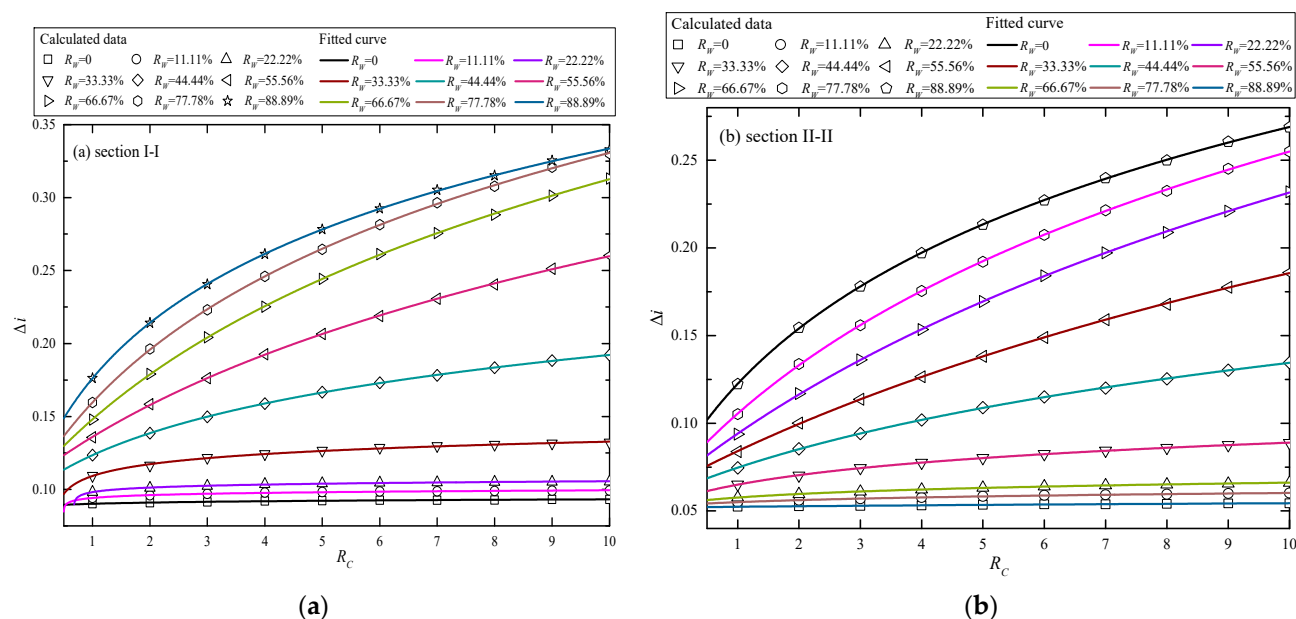


Figure 13. Relationship between Δi and R_c when R_w changes from 0 to 88.89%: (a) section I-I; (b) section II-II.

Figure 14 presents the relationship between Δi and R_c when R_w is 77.78%, of which all the relationship can be fitted by logarithmic curve. Groundwater supply from outside to inside requires flow from the bottom of the diaphragm wall, which means vertical permeability is important for the seepage process. Because of the large value of R_c and low vertical hydraulic conductivity, groundwater supply becomes difficult and Δi increases. The acceleration of Δi decreases, and due to the decrease of vertical hydraulic conductivity is limited.

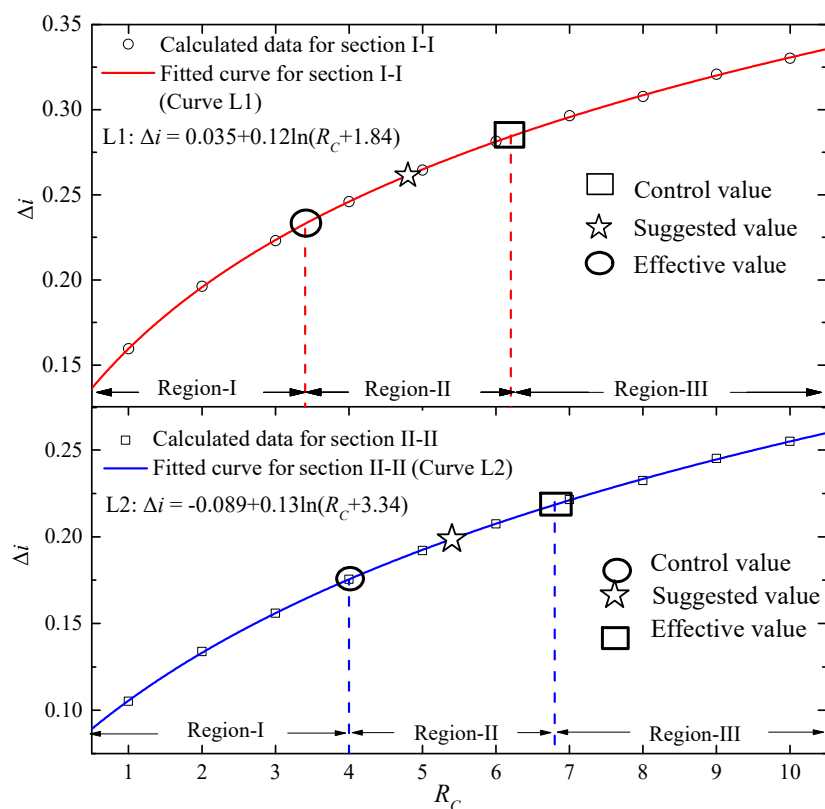


Figure 14. Relationship between Δi and R_c when R_w is 77.78%.

Three regions are divided on the curve: poor effect region (Region-I), general effect region (Region-II) and good effect region (Region-III). The x-coordinate of demarcation points of three parts are defined as the effective value and control value, which represents the variation degree of Δi as 60% and 85%.

When R_c is smaller than the effective value, Δi is small. The groundwater drawdown outside the pit is large and the environmental effect due to foundation dewatering is adverse. When R_c is over the control value, water supply is difficult and the environment effect is good. If R_c of the dewatering aquifer in the engineering field is small, some measurements can be adopted to increase R_c to control the environment effect. By comprehensive consideration of economy and construction technology, the suggested value should be recommended. The suggested value is defined as the x-coordinate of midpoint of effective and control value. The effective, suggested and control value of R_c is 3.4, 4.8 and 5.2 for section I-I, and those values are 4.0, 5.4 and 6.8 for section II-II when R_w is 77.78%.

4.4. Relationship Between R_w and R_c

To determine the common relationship between control, suggested and effective R_w and R_c , all cases are conducted and the relationship is shown in Figure 15. The control and suggested value of R_w decreases with the increase of R_c , while the effective value of R_w increases with the increase of R_c . All the relationships between R_w and R_c can be fitted by the logarithmic function, of which the characteristic is that the curve changes quickly firstly and the acceleration decreases. By the fitted equation, it is convenient to calculate the control, suggested and effective value of R_w according to different values of R_c .

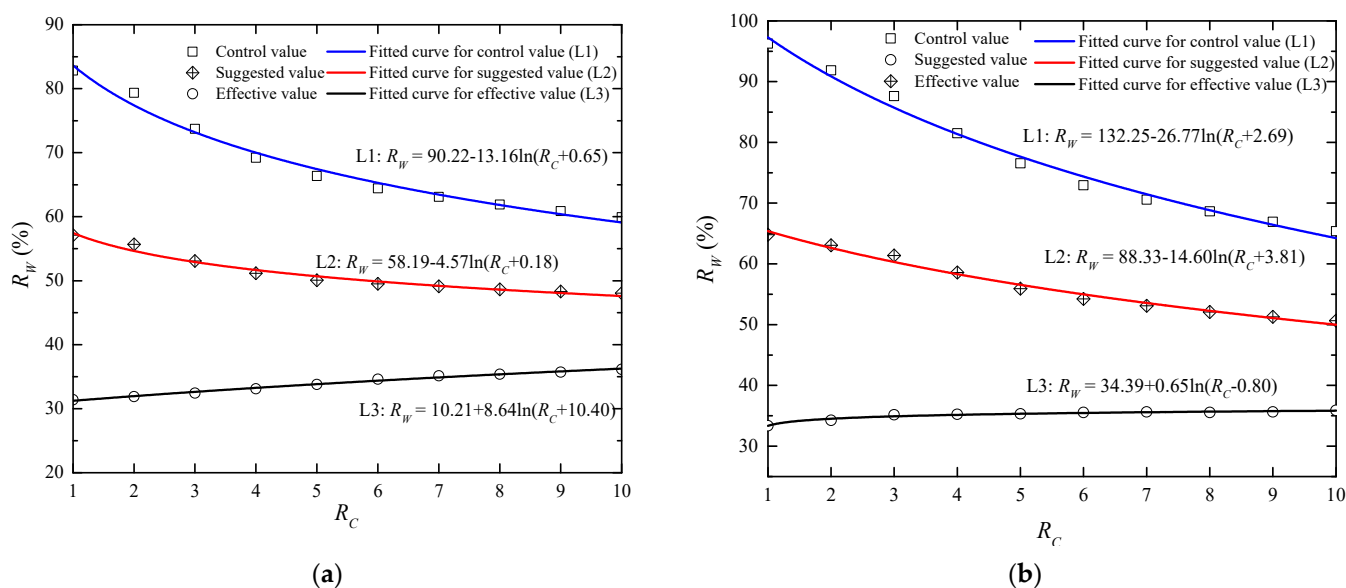


Figure 15. Relationship between control, suggested and effective R_w and R_c : (a) section I-I; (b) section II-II.

For example, if the engineering is located at the place where the anisotropic permeability is not obvious, such as R_c is 1.5, the control, suggested and effective value of R_w for section I-I is 80.1%, 55.8% and 31.6% respectively. Those values are 93.9%, 63.9% and 34.2% for section II-II. Therefore, R_w is recommended to be approximately 64% when R_c is 1.5. As per the dissymmetry effect of the foundation pit mentioned above, control and suggested value of R_w on section II-II is larger than that on section I-I, since the drawdown outside the diaphragm wall on section II-II is larger and drawdown inside the diaphragm wall is close.

5. Conclusions

This paper investigates the reasonable buried depth of diaphragm walls considering the anisotropic permeability of the dewatering aquifer for the control of groundwater drawdown outside the pit during foundation dewatering. The relationship among approximate hydraulic gradient at two sides of the diaphragm wall (Δi), the ratio of the penetrating depth of diaphragm wall and the thickness of the dewatering aquifer (R_w), the ratio of horizontal and vertical hydraulic conductivity in dewatering confined aquifer (R_c) are analyzed by numerical simulation based on an engineering case in Shanghai. The following conclusions can be obtained from this study:

(1) Δi increases with the increasing of R_w , due to the increased blocking effect of the diaphragm wall. The relationship between Δi and R_w can be fitted by the Boltzmann curve, which can be divided into three parts: initial gradual part, middle sharp part and final gentle part. The suggested value of R_w is defined as the x-coordinate of the contraflexure point of the curve. By comprehensive considering of Δi on two sections along the long and short sides of the foundation pit, the suggested value of R_w is 51% when R_c is 10.

(2) Δi increases with the increasing of R_c , because the smaller vertical permeability makes the supply of groundwater from outside to inside the pit more difficult. The relationship between Δi and R_c can be fitted by the logarithmic function, which also can be divided into three parts: poor effect part, general effect part and good effect part. X-coordinate of demarcation points of three parts are defined as the effective and control value of R_c . The suggested value of R_c is defined as the midpoint of effective and control value.

(3) If R_c is small, measurements should be adopted to increase R_c to the suggested value. By a comprehensive considering of Δi on two sections along the long and short sides of the foundation pit, suggested value of R_c is 5.4 when R_w is 77.78%.

(4) The relationship between the control, suggested and effective value of R_w with R_c is fitted by logarithmic function, and these values of R_w can be calculated by the values of R_c . By comprehensive consideration of the two sections along the long and short sides of the foundation pit, the suggested value of R_w ranges from 48% to 65% when R_c ranges from 1 to 10.

(5) This study proposes an optimized method to search suggested R_w and R_c , which can be used in foundation pit dewatering engineering in layered soil to obtain well dewatering effects and reduce environmental effects. Certainly, the specific value of suggested R_w and R_c may change in different engineering projects considering the different geometrical characteristics and hydrogeological strata in different place.

Author Contributions: Conceptualization, X.-w.W. and Y.-s.X.; methodology, Y.-s.X.; software, X.-w.W.; writing—original draft preparation, X.-w.W.; writing—review and editing, X.-w.W. and Y.-s.X. All authors have read and agreed to the published version of the manuscript.

Funding: This research was funded by National Natural Science Foundation of China (NSFC) (Grant No. 41877213). This financial support is gratefully acknowledged.

Institutional Review Board Statement: Not applicable.

Informed Consent Statement: Not applicable.

Data Availability Statement: This study does not report any data.

Conflicts of Interest: The authors declare no conflict of interest.

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