

Article

Leakage Management and Pipe System Efficiency. Its Influence in the Improvement of the Efficiency Indexes

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Abstract: Water is one of the most valuable resources for humans. Worldwide, leakage levels in water distribution systems oscillate between 10% and 55%. This causes the need for constant repairs, economic losses, and risk to the health of users due to possible pathogenic intrusion. There are different methods for estimating the level of leakage in a network, depending on parameters such as service pressure, orifice size, age and pipe material. Sixty-two water distribution networks were analyzed to determine the leakage method used, the calibration method, and the percentage of existing leaks. Different efficiency indicators were proposed and evaluated using this database. Several cases of installation of pumps working as turbines (PATs) in water distribution networks were analyzed in which the use of these recovery systems caused a pressure drop, reducing the level of leaks and recovering energy.

Keywords: leakage; PATs; efficiency indexes; water networks; energy recovery

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1. Introduction

Water is probably the world's most precious natural resource. It is sad then to think that so many people already live with less than their daily needs and that this situation is predicted to get even worse [1].

In a survey carried out by the International Water Services Association (IWSA), water losses vary between 8% and 24% in developed countries, 15–24% in newly industrialized countries, and 25–45% in developing countries [2]. Current statistical surveys indicated that 50% of the treated drinking water in city centers is lost and unaccounted for in big countries like Turkey [3]. According to other surveys [4], water leakages in a water distribution system may vary from 5% to 55% of the total supply and generally increase with pressure and it can therefore have an important effect on the operation of the system. When a network is simulated by a model considering the relationship between pressure and leakage flows, it can provide more realistic results compared to if no leakage is assumed [5].

Detecting and locating leaks in pipes has become an important aspect of water management systems. Since monitoring leakage in large-scale water distribution networks (WDNs) is a challenging task, the need to develop a reliable and robust leak detection and localization technique is essential for loss reduction in potable WDNs [6].

The literature describes different methods for detecting leaks in the water supply [6] classified leak detection methods as internal and external. The external detection methods are acoustics, fibre optics, gas injection, magnetic induction, ground penetration radar, among others. The internal methods are balancing methods, pressure/flow monitoring, signal processing approach, statistical analysis, model-based methods. Currently, the use

of Transient Test-Based Techniques (TTBTs) is a good technique to improve the water management of the systems [7]. The merit of the TTBTs results from the simplicity and the limited cost of the equipment [8]. There is extensive literature which shows the development and progress of the current transitional theory, including transient flow models, models of unstable friction and turbulence and numerical simulation methods.

The contribution described in [9] presented a literature review of some major standpoints in hydraulic transient-based leak detection of pipe systems from 1992 to 2018. Reference [10] proposed a technique for leakage detection based on the well-known properties of transient pressure waves. A method for the analysis of inverse transients through a numerical model is proposed in [11].

This research focuses on model-based leakage detection methods. Reference [5] showed the formulation of a rupture prediction model that relates the rupture of a pipe with the exponent of its age, the optimal moment to replace a pipe, and the economic analysis with water losses for leaks.

Managing losses in water distribution systems efficiently is one of the key issues of water utilities. In particular, tools and methods for managing leakage are actively developed among components of losses [12]. There are many causes of such leakages, including natural process of wear, corrosion of the inner and outer surface of pipes, mechanical damage of pipes caused by excessive loads, assembling errors, seasonal temperature changes, movements of a subsoil, and material defects of pipes. The failure of pipelines is usually attributed to the ageing infrastructure and/or severe environmental conditions [13].

As water mains deteriorate both structurally and functionally, their breakage rates increase, network hydraulic capacity decreases and the water quality in the distribution system may decline [14]. A network with physical failures may be exposed, under certain operating conditions, to pathogen intrusions that affect the quality of water [15]. The leaky pipes increase pumping energy to guarantee the service levels. In addition, the energy wasted in leaks involves an environmental burden related to the many impacts associated with energy production and consumption, including greenhouse gas emissions, acid rain, and resource depletion [6].

The general objectives of pressure management for leakage minimization are threefold [16]: (i) reduce background leakage which refers to acoustically undetectable seeps at pipe joints and small cracks; (ii) decrease the rate of new leaks and breaks which occur on mains and service connections, due to diminished stress on the pipes; and (iii) minimize the flow rate from any leaks and breaks. To reduce leakage in the distribution network, it is necessary to apply models to know their efficiency volumetric. Different methods were found in the literature review discussed below.

Related to pressure management, one of the most used elements are pressure reduction valves (PRVs). A PRV consists of the main valve and a PRV controller which senses the output pressure and adjusts the opening of the main valve to maintain the outlet pressure at a given set-point, thereby reducing leakage and mitigating the stress on the downstream water distribution network. A research topic is how to understand and explain the instability phenomena in pressure control schemes for low flow rates (small valve openings) [17]. The results of a mathematical model representing the static and dynamic properties of a hydraulic controller and a PRV show good agreement with experimental data [18]. Optimization models through genetic algorithms are used to minimize leaks in water distribution sites through the most effective location and adjustment of control valves [19].

A hybrid multi-objective algorithm was established, which has as decisive variables the pipe diameter and the positions and settings of the valves. Analysis of the results shows that the new algorithm is more efficient than a multiobjective genetic algorithm widely adopted in the scientific literature [20]. The analysis of the tests demonstrates the versatility of PRVs as a powerful tool for pressure management, and also when the flow condition changes according to the users' demand pattern. Notwithstanding the crucial

importance of PRVs, few experimental data are available in the literature [21]. The pressure control strategy through PRVs has been thoroughly researched as a management strategy. In contrast, little experimental data is available in the literature, regarding the transient behaviour of PRV in terms of its response to incoming pressure waves, as well as the time required to reach the pressure set point [22].

The use of pumps working as turbines (PATs) was proposed by [23] as an unconventional solution to reduce pressure and recovery energy. PATs are standard water pumps utilized as hydraulic turbines by reversing the flow direction across them. Electric power generation through PATs has a lower cost compared to hydraulic turbines. However, in the literature, the few available cost figures relative to PAT purchase price are discordant and often outdated, and such a lack of information is likely a severe barrier to a more widespread PAT implementation [24].

The application of PATs appears as an alternative and sustainable solution to either control network pressure as well as to produce energy [25]. Different pumps suitable to run in turbine mode for low capacity power generation in micro-hydropower plants as well as in water supply piping systems are discussed. The research work on PAT including criteria for selection of pump running as turbine, cavitation analysis, force analysis, loss distribution, various methods of performance enhancement, cost analysis of hydro-power plant with conventional hydro turbine and PAT, applications of PAT in water supply pipelines, among others [26].

Performance data in pump and turbine mode were published in [27]. An energy evaluation is shown considering a test case of a water distribution network. Due to some problems such as limited resources and environmental pollution that they cause in recent years, researchers and engineers have focused on various types of renewable energies. Benefiting renewable resources improves air quality and prevents further greenhouse gas emissions [27]. Being the implementation of PATs a sustainable alternative for the reduction of leaks and energy production.

The objective of this research is to show the different methodologies to determine the leakage flow in distribution networks as well as the implications of leaks in volumetric performance, energy losses, and the costs associated with the loss of water and energy in forty-five case studies. The comparison between different case studies enables to show the range of the different key performance indicators (KPIs) applied to them.

2. Leakages Evaluation and KPIs

Different methods were used to estimate the leakages in the water systems. The most used are Torricelli theorem [16], MNF [28], Fixed and Variable Area Discharges (FAVAD) [6], N1 Power Law and leakages model [29], and Background and Bursts Estimates (BABE) [30].

Figure 1 shows the process and analysis carried out in this research. Forty-five different distribution networks were selected and analyzed for leaks. The leak calculation method implemented in the model was identified and relevant information was subsequently extracted from each case to calculate the efficiency indicators. The water distribution systems were different in terms of topology and pressure. However, to consider the general indicators and get the normalization of the different indicators, the research work with the value of average pressure to establish the correlation between indicator values. However, the different cases studies were analyzed in the different published studies considering the pressure value in the different leakage methods.

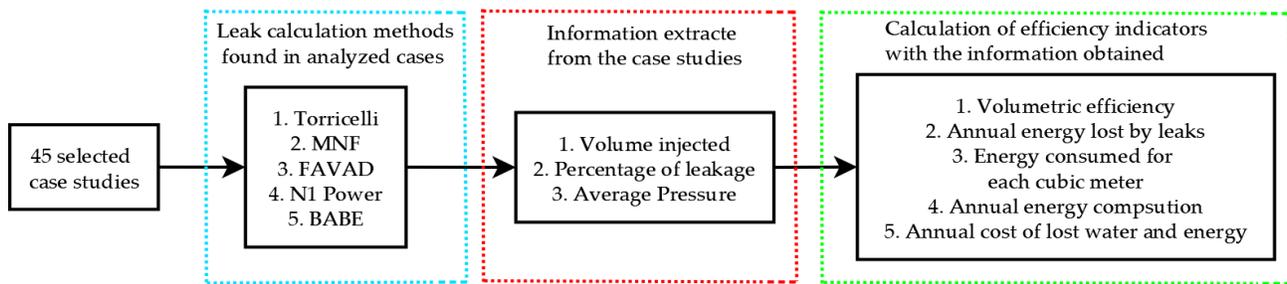


Figure 1. Analysis and calculations made to case studies.

2.1. Torricelli Teorem

The hydraulics of orifices is well understood, and a fair amount of research has been conducted on different orifice shapes and conditions. In Torricelli's equation, the velocity through an orifice is expressed as [31]:

$$v = \sqrt{2gh} \quad (1)$$

where v is the theoretical velocity (ms^{-1}); g is the acceleration due to gravity (ms^{-2}) and h is the pressure head (m w. c.).

The starting point is that water leakages in WDN are directly related to pressure as well as to the age and material of WDN elements (joints, valves, pipes, etc.). Many numerical models were developed starting from experimental observations and assuming the validity of the Torricelli law in the following form, which is valid for a single leak orifice [30].

One of the major factors that influence leakage rate is the pressure in the distribution system. A conventional physical model that relates the pressure and the leakage rate is the well-known Torricelli orifice equation, described as [32]:

$$Q_{leak} = C_d A \sqrt{2gh} \quad (2)$$

where Q_{leak} represents the leakage flow rate, C_d is the leakage discharge coefficient, h denotes the pressure head, and A the area of the leak opening in m^2 . However, from several studies on real WDNs, it has been shown that the Torricelli law does not provide a satisfactory model for the relationship between leakages with pressure within a WDN [30].

2.2. Minimum Night Flow (MNF) Analysis

Minimum Night Flow (MNF) analysis is the most common method for leakage assessment at the scale of the DMA. The MNF is the lowest inflow in the DMA over 24 h along the day, which occurs depending on consumption patterns but reportedly, between 02:00 and 04:00 AM when most of the customers are probably inactive and the flow at this time is predominantly leakage [33]:

$$L_{DMA} = Q_{MNF} - Q_{LNC} \quad (3)$$

where L_{DMA} is the leakage rate in the DMA (m^3h^{-1}) at the time hour of MNF, Q_{MNF} is the minimum flow rate (MNF) and Q_{LNC} is the legitimate nighttime consumption in the DMA at the MNF time.

The leakage in the MNF time cannot be generalized for all the hours of the day because of the pressure leakage relationship, where higher pressure at night leads to higher night leakage and lower pressures during the day lower the day leakage. For this reason, the MNF leakage should be modelled according to the leakage-pressure relationship [28]. Lambert [34] in his research shows three basic steps that must be performed to determine the MNF.

Once the leakage flow has been obtained in the hour of the night minimum, its value can be extrapolated to the rest of the hours using a multiplying factor known as the hour-day factor (HDF). The HDF represents the change throughout the day of the leakage flow and it is proportional to the pressure variation in the network for the pressure of the hour of minimum night flow. This pressure variation is calculated in the Average Zone Pressure (AZP), which is assumed to be the most representative point of pressure in the network, therefore [35]:

$$Q_{leak(t)} = Q_L(t_{MNF}) \cdot \left[\frac{P_{AZP(t)}}{P_{AZP}(t_{MNF})} \right]^\alpha \quad (4)$$

where $Q_{leak(t)}$ is the leakage flow at a different time from that considered for the night minimum, (t_{MNF}) is the time at which the night minimum flow is measured, $P_{AZP(t)}$ is the pressure in the AZP (selected as point average of the pressures in the network), $P_{AZP}(t_{MNF})$ is the pressure in the AZP at the hour (t_{MNF}) and finally, α is the exponent of the emitter. This equation is based on the FAVAD theory. Stenberg [36] has found that night leakage flow rates should then be multiplied by 20 h. This assumption does not consider that pressure is not constant over a period.

2.3. FAVAD Concept

A variety of different concepts have been proposed to explain the diversity of pressure: leakage rate relationships including the widely used fixed and variable area discharge (FAVAD) model. This model offers a means to demonstrate the sensitivity to the pressure of different leaks and to quantify the change of the area of elastically deforming leaks subject to hydraulic pressure loading [16].

The FAVAD concept, as realistic modelling of leakage and intrusion, flows through leak openings in pipes in water supply systems, allows for variations in the area of leaks resulting from changes in pressure [34]. The FAVAD concept as the principle of conservation of energy provides a logical hydraulic basis to explain, analyze and predict diverse relationships between Average Zone Pressure (AZP) and leak flow rates [37].

Emitters are devices associated with junctions that model the flow through a nozzle or orifice that discharges to the atmosphere [38]. The technique for a more consistent description of demands is to allocate part of the total metered inflow to the leakage component, and account for this as a pressure-dependent demand [39].

When the leakage flow at each pipe is known, this value can be assigned to each node according to Equation (5) [40]:

$$ql_i = C_i \cdot P_i^\beta \quad (5)$$

where ql_i is the leakage flow at node $l \text{ s}^{-1}$, β is the emitter exponent which takes into account the pipe material and the shape of the orifice, and P_i is the pressure at node i . [41] defined the leakage exponent as α , which is sometimes called $N1$. The coefficient C_i was determined by equation:

$$C_i = \alpha \cdot 0.5 \cdot \sum_{j=1}^{K_{ji}} L_{ji} \quad (6)$$

α is a coefficient ($l \text{ s}^{-1} \text{ m}^{-1-\beta}$), j is an index related to pipe, K_{ji} is the number of pipes connected to node i and L_{ji} is the length of pipe j connected to node i (m). The leakage volume was obtained by applying the equation:

$$Vl = \frac{1}{1000} \cdot \sum_{h=1}^{24} Ql_h \cdot \Delta h \quad (7)$$

where the term $1/1000$ is used to convert units from $l \text{ s}^{-1}$ to $\text{m}^3 \text{ s}^{-1}$, Ql_h is the total leakage flow at time h and Δh (s) is the time in which Ql_h ($l \text{ s}^{-1}$) is applied.

α and β variables are two leakage model parameters, representing the influence of some factors on the relationship leakage/pressure. Parameter β can represent the pipe deterioration over time, thus it depends on both pipe characteristics (pipe age, diameter, and material) and various external factors (mainly the average pressure, but also other environmental conditions, traffic loading, external stress, and corrosion, etc.). In contrast, α is a function of pipe characteristics only (material and elasticity) [16,30].

In general, β is more closely related to the number of leaks (or leakage area) per unit of pipe length while α is more strongly related to the type of leakage (therefore to the hydraulics of leakage) as governed by the pipe material. For this reason, changes in β need to be determined for the specific system, i.e., by model calibration, while most experimental studies have focused mainly on leakage parameter α [4,30]. Table 1 shows the range of α and β coefficients for different pipe materials.

Table 1. Coefficients for different materials.

Material	α	β
Cement	$10^{-7} \leq \alpha \leq 10^{-5}$	$0.75 \leq \beta \leq 1.10$
Steel	$10^{-6} \leq \alpha \leq 10^{-4}$	$1 \leq \beta \leq 1.3$
PVC	$10^{-5} \leq \alpha \leq 10^{-4}$	$1.1 \leq \beta \leq 1.5$

The value of the leakage parameter, α , can be described using the fixed and variable area discharge (FAVAD) approach. The range of exponents observed reflects substantial differences in the impact of pressure on the rate of leakage. For example, when the pressure halved in a pipe it caused a decrease of the flow rate of 29%, 50% and 82% respectively, when the exponents were 0.5, 1.0 and 2.5 [41]. Reference [42] showed the coefficient is a function of the length of the pipe and the type of material in the network. Table 2 shows the leakage emitter exponent used in different study cases. It is observed that the most used emitter exponent in the analyzed case studies is 0.5, however, in some cases, they use different coefficients to observe the sensitivity of the parameter to the model.

Table 2. Case study emitter exponent.

Reference	ID	Emitter Exponent	Material
[29,43–48]	7, 9, 12, 13, 27, 33	0.5	PVC, HDPE, steel, asbestos cement and cast iron
[49]	-	0.61	PVC, asbestos cement, galvanized steel
[50]	8	0.5–1	PVC, Polyethylene (PE), iron, steel.
[51]	1	0.91–1.13–1.41	PVC, metal, ambient.
[52,53]	2, 10	1.1	PVC, iron.
[54]	5	0.5–1.18	-
[55]	3	1.1–1.18	Ductile iron, Steel, High-Density Polyethylene (HDPE)
[56–58]	11, 14, 15, 18	1.18	PVC, asbestos cement, galvanized steel
[37,41,59–61]	-	0.5–2.5	PVC, iron, galvanised iron, asbestos cement.
[16,31]	16	0.5–2.79	PVC, asbestos cement.
[59,62]	-	0.5–2.95	-

The research described in [63] confirmed that the leak exponents can be significantly higher than the theoretical value of 0.5. While leakage exponents for round holes were near 0.5, the values for corrosion holes varied between 0.67 and 2.30, for longitudinal cracks between 0.79 and 1.85, and circumferential cracks between 0.41 and 0.52. A variety of different concepts have been proposed to explain the diversity of pressure: leakage rate relationships including the widely used fixed and variable area discharge (FAVAD) model [64].

2.4. FAVAD and the N1 Power Law

The N1 Power Law an approximate version of the FAVAD concept—has been increasingly used internationally since 1994 for practical assessment of pressure-dependent leakage in water distribution systems. The estimation of system leakage and average zone pressure head (AZP) before and after pressure management can be used to estimate the leakage coefficient C and leakage exponent N_1 [65]. When modelling the pressure-leakage rate relationship in individual water distribution systems, a more general form expression was proposed instead of the orifice equation [37]:

$$Q_{leak} = Ch_{AZP}^{N_1} \tag{8}$$

where Q_{leak} is the leakage flow rate and h_{AZP} the AZP. Equation (8) is also called the N_1 power-law or the N_1 equation. The N1 Power Law approximation simplifies FAVAD by assuming that the leak flow rate varies with average pressure N_1 , where N_1 lies between 0.5 (less sensitive) and 1.5 (more sensitive). FAVAD and N_1 are both widely used internationally in distribution systems and laboratory tests [34].

Different investigations [63,66,67] experiment with the materials of the pipes to calibrate the parameters of C and N_1 , according to the orifice in the pipe, it is transverse, longitudinal or circular (Table 3).

Table 3. Summary of leakage exponents N_1 .

Failure Type	uPVC	Asbestos Cement	Mild Steel
Round hole	0.52	-	0.52
Longitudinal crack	1.38–1.85	0.79–1.04	-
Circumferential crack	0.41–0.53	-	-
Corrosion cluster	-	-	0.67–2.30

The median used is 1.15 depending on the mixture of leaks and the dominant type of leaks (fixed area leaks: $N_1 = 0.5$; longitudinal split which opens in one dimension: $N_1 = 1.5$; linear-radial opening: $N_1 = 2.0$ – 2.5). Typically found in the literature an exponent value of 0.5 [16]. Ref. [15] presented a methodology for evaluating water losses where a sensitivity analysis of the coefficients is carried out to determine the influence of various leakage exponents on the results of the study.

2.5. Background Leakage and Emitter Coefficient (BABE)

It is assumed that leakage continuously increases with pressure and can be expressed as the sum of the background leakage and the burst leakage. So, the pressure-leakage relationship for a pipe k can be stated as follows:

$$q_k^{leak}(P_k) = \begin{cases} \beta_k l_k (P_k)^{\alpha_k} + C_k (P_k)^{\delta_k} & P_k > 0 \\ 0 & P_k \leq 0 \end{cases} \tag{9}$$

where q_k^{leak} is the total leakage along pipe k (m); l_k is the length of pipe k (m); α_k and β are parameters of the background leakage model; C_k and δ_k are parameters of the bursts leakage model; and P_k is the average pressure(m) in pipe k computed as the mean of the pressure values of its end nodes. The leakage model assumes a uniform distribution along the pipe [4], which is the most common experimental relationship between leakage and pressure [5]. Several studies model leaks along the pipe [16,60,68]. For each pipe, the total leakage is assigned to its end nodes, half to each node. So, the nodal leakage flow q_i^{leak} for a node i can be computed as follows:

$$q_i^{leak} = \frac{1}{2} \sum_k q_k^{leak} \tag{10}$$

2.6. Summary of Leak Calculation Methodologies

As mentioned above, there are different methodologies to calculate water leaks in distribution networks. These methods are used in designs to know the leakage percentage of a network. Table 4 shows the calculation equations, parameter calibration methods, advantages and disadvantages of the leakage methodologies described in this research.

Table 4. Methodologies for calculating leaks.

Technique	Reference	Equation	Calib.	Advantages	Disadvantages
Torricelli	[16,69–73]	$Q_{leak} = C_d A \sqrt{2gh}$		<ul style="list-style-type: none"> • Pressure dependent • The scale is system-wide 	<ul style="list-style-type: none"> • Only applicable for rigid pipe orifices • Does not consider the flexibility of the pipes • It does not provide a satisfactory model of the relationship between leakage and pressure.
MNF	[28,29,35,44,47,70,74–76]	$L_{DMA} = Q_{MNF} - Q_{LNC}$ $Q = Q(t_{MNF}) \cdot \left[\frac{P_{AZP(t)}}{P_{AZP(t_{MNF})}} \right]^\alpha$		<ul style="list-style-type: none"> • No need lots of network data • No need for a mathematical model 	<ul style="list-style-type: none"> • Need to data loggers and mechanical flow meter • Needs measurements • Intensive fieldwork, zoning
FAVAD (Fixed and Variable Area Discharges)	[6,16,29–31,34,36,37,42,45,49,50,53,55,59,62,64,65,69,71,73,77–84]	<p>The equation can be written in different ways:</p> $Q_{i,l}(t) = K_i [P_i(t)]^\alpha$ $L = c P^\gamma$ $q l_i = c_i \cdot P_i^\beta$ $Q_{leak} = \beta P^\alpha$	K_i, C, β [59,66,78]	<ul style="list-style-type: none"> • Consider that the area of the orifice varies with pressure • Pressure dependent • Analyse field data and volumes of bursts and the rates of small background leaks • The scale is to District Metered Area (DMA) 	<ul style="list-style-type: none"> • Many assumptions • Further calibrations are useful
The N1 Power Law	[29,31,34,37,43,63,65–67,76,84]	$Q = C_d h_{AZP}^{N_1}$	N_1 [59,66]	<ul style="list-style-type: none"> • Pressure dependent • The scale is system-wide 	<ul style="list-style-type: none"> • It is an empirical equation and therefore not founded on fundamental principles of fluid mechanics. • The values of C and N1 are not constant for a given system but depending on the pressures at which they are being estimated. • The equation is dimensionally awkward since the units of C include the variable N1.
Background leakages model	[5,16,37,43,60,68,76,83–91]	$Q = \beta_k l_k (P_k)^{\alpha_k} + C_k (P_k)^{0.5}$	β and α [4], β [85,89]	<ul style="list-style-type: none"> • Consider leaks along the pipe • Pressure dependent • The scale is system-wide 	<ul style="list-style-type: none"> • Further calibrations are useful

2.7. Leakages Modelling and Calibration

In a distribution network under MNF conditions, using leakage and pressure data, the leakage exponent of the system was determined by the following expression. From the two data sets, system leakage exponent N_1 was calculated using the equation below [76]:

$$N_1 = \frac{\log\left(\frac{Q_1}{Q_2}\right)}{\log\left(\frac{AZNP_1}{AZNP_2}\right)} \quad (11)$$

where Q_1 and Q_2 are system leakages and $AZNP_1$ and $AZNP_2$ are average zone night pressures before and after pressure reduction, respectively. Van Zyl, Lambert, and Collins [77] suggested a modified version of the orifice equation where fixed orifice area and flexible orifice area are considered as shown in Equation (12):

$$Q_{leak} = \text{sgn}(h)C_d A \sqrt{2g(A_0|h|^{0.5} + m|h|^{1.5})} \quad (12)$$

where Q_{leak} is leakage flow, $\text{sgn}(h)$ is the sign function, C_d is the discharge coefficient, h is the pressure head, g is the acceleration of gravity and A_0 is the initial area of the orifice.

Calibration methods can leverage steady-state hydraulic models and optimization tool technology, such as Genetic Algorithms (GAs) to improve the detection of a leak [61,92]. Reference [62] proposed a methodology for the estimation of C and γ , based on the solution of an inverse modelling problem where demand and leakage pattern are assumed as unknowns, is solved using GAs. Reference [4] proposed a genetic algorithm using Epanet to calibrate the alpha and beta coefficients in a network and shows the variation of the coefficients in the case of new and old pipes. Reference [69] showed a table of calibrated coefficients, which were determined through experimental tests on PVC and Steel materials from 1" to 3" with holes from 1.5 to 10 mm. References [12,93] proposed a method of estimating area leakages in virtual areas of a water network to prioritize leak surveys for the areas.

To calibrate both emitter C and exponent N_1 coefficients, the leakage flow rate was calculated as the difference between the upstream and downstream measured flow rates, this methodology is described in [66]. This research used a CFD model with experimental results to different geometric configurations of the orifice, to assess the dependence of the orifice geometry and orientation on the calibration of leakage law parameters.

Reference [92] showed a GA. This is used to solve optimization problems of searching for calibration parameters values while minimizing the differences between observations and model predictions. A Matlab optimization code was developed for model calibration and was linked to the Epanet toolkit. The optimization process uses a non-dominated sorting genetic algorithm II (NSGA II) [94]. A multi-objective procedure based on non-dominated sorting GAs has been developed to calibrate a water supply system characterized by elevated pressure and a high amount of background leakages [89].

Reference [85] used a method of optimization in the network through a differential evolution algorithm (DE). It is a simple power and population-based stochastic optimization algorithm that outperforms many meta-heuristic algorithms on numerical single-objective optimization problems.

Reference [95] proposed a method of optimization using is the BOBYQA algorithm, this model is an iterative algorithm for finding a minimum of a function. Table 5 shows a summarize of the used algorithms.

Table 5. Summary of optimization algorithms.

Reference	Algorithm	Parameters	Objective Function	Error
[4,39,52,54,61,62,81,83,89,92,96–98]	Genetic algorithm.	Operation conditions, flows, demands, pressures, total leakage.	Minimize network pressure and producing a new generation of solutions α and β .	0.5 to 23%, with an average value of 11%.
[6,29,91]	Algorithm for detecting and estimating background leakage.	Operation conditions flow, pressure and fluid temperature.	Detect critical causes and their location for possible pressure control.	-
[88]	Pseudogenetic algorithm.	Basic network	Operational costs, capital costs (pipe and pump replacement, tank expansion, and PRVs), and constraints.	-
[76]	Global Gradient Algorithm.	Flow, pressure.	Reduce excess pressure.	-
[1]	Neural networks.	Flow, pressure.	Detection of water leaks.	-
[99]	Differential evolution with temporal analysis.	Water distribution network.	Estimation and location to leakage.	Root mean squared error: 0.05
[85]	Differential evolution.	Flow, pressure, network data.	Estimation of leakage.	-
[100]	Algorithm with convergence analysis.	Operation conditions, flows, demands, pressures.	Estimation to leakage	-
[54]	Sequential Quadratic programming.	Water distribution network.	Leakage minimization	-

2.8. Leakages Key Performance Indicators (LKPIs)

Performance indicators are a powerful management tool, they can provide measures of how many resources are being used relative to those available, they can be used to assess the extent to which management objectives are being met, and even to assess the overall impact of strategies management [101]. Some proposed key performance indicators leakages are pressure-dependent demand, volumetric efficiency, performance indicators for water supply services:

Pressure-dependent demand concept; Two types of pressure-dependent demands are considered: consumptions and leakage (background and bursts). For pressure-dependent consumption, it is assumed that the available demand (q_i^{avl}) is computed with the pressure-demand relationship [86,91]:

$$q_i^{avl}(P_i) = q_i^{req} \cdot \begin{cases} 1 & P_i \geq P_i^{ref} \\ \left(\frac{P_i - P_i^{min}}{P_i^{ref} - P_i^{min}}\right)^\alpha & P_i^{min} < P_i < P_i^{ref} \\ 0 & P_i \leq P_i^{ref} \end{cases} \quad (13)$$

where P_i^{ref} is the reference pressure necessary to satisfy the required demand q_i^{req} , P_i^{min} is the pressure below which no water can be supplied, α (typically 0.5) is the exponent of the pressure-demand relationship, P_i is the current pressure at node i .

- 1) Volumetric efficiency (η_v); One of the most important ratios among the system’s efficiency indicators is volumetric performance. It is defined as the relationship between the registered volume V_{Reg} and the total volume V_{Tot} contributed in the same reference period [102]:

$$\eta_v = \frac{V_{Reg}}{V_{Tot}} \quad (14)$$

- 2) Performance Indicators for Water Supply Services; International water association (IWA) provides performance indicators of water supply systems to compare the management of water losses, these are (i) Water losses and real losses as a % of system input volume; (ii) Water losses per house connection and km of mains per day (density of connections < 20 per km of mains), and (iii) Infrastructure Leakage Index (ILI) [103].
- 3) Infrastructure Leakage Index (ILI); The ILI is a measure of how well a distribution network is managed (maintained, repaired, rehabilitated, etc.) for the control of real losses, at the current operating pressure. It is the ratio of the Current Annual volume of Real Losses (CARL) to Unavoidable Annual Real Losses (UARL) [104].

$$ILI = \frac{CARL}{UARL} \quad (15)$$

- 4) Unavoidable Annual Real Losses (UARL); UARL is a useful concept as it can be used to predict the lowest technically annual real losses for any combination of mains length (18 L/km mains/day/meter of pressure), number of connections (0.8 L/service connection/day/meter of pressure), customer meter location and average operating pressure (25 L/km/day/m of pressure) assuming that the system is in good condition with high standards for the management of real losses [105].
- 5) Absolute annual consumed energy (IAAE); this index is sum of the total active consumed energy in the network subtracted by the sum of the total energy recovered in the network, the units are kWh/year [106].
- 6) Absolute consumed energy per unit volume (IAEFW); Ratio between IAAE and the total volume of water introduced in the network, the units are kWh/m³ [107].

3. Results

3.1. Case Studies

To observe the different methodologies applied to real cases of water distribution networks in different parts of the world, this research analyzed 45 case studies, as shown in Table 6.

Table 6. Information of the studied cases.

ID	Case Study	Year	Ref	ID	Case Study	Year	Ref
1	Skiathos, Greece	2020	[51]	24	Zarqa, Jordan	2020	[70]
2	Leicester, UK	2012	[52]	25	Zarqa, Sana'a, Yemen	2020	[70]
3	Benevento, Italy	2017	[55]	26	Mwanza, Tanzania	2020	[70]
4	Pretoria, South Africa	2017	[29]	27	Mutarea, Zimbabwe	2006	[47]
5	Polokwane, South Africa	2019	[54]	28	Skopje, Macedonia	2011	[108]
6	Villarreal, Spain	2014	[109]	29	Pittsburgh, Pensilvania	2005	[110]
7	Guayaquil, Ecuador	2015	[46]	30	Azogues, Ecuador	2019	[13]
8	Antalya, Turkey	2017	[50]	31	Mankessim, Ghana	2014	[74]
9	Konyaalti, Turkey	2012	[44]	32	Rzesów, Poland	2019	[1]
10	Valencia, Spain	2015	[53]	33	Gorino Ferrarese, Italy	2021	[48]
11	Palermo, Italy	1999	[56]	34	Salzburg, Austria	2011	[111]
12	Nagpur, India	2016	[45]	35	Belgium	2014	[112]
13	Nagpur, India	2016	[45]	36	Dryanovo, Bulgaria	2014	[112]
14	London, UK	1989	[57]	37	Pula, Croatia	2014	[112]
15	London, UK	1989	[57]	38	Lemesos, Cyprus	2013	[112]
16	Nourhan Samir, Egypt	2017	[16]	39	Odense, Denmark	2013	[112]
17	C-Town	2015	[113]	40	England	2013	[112]
18	Verona, Italy	2019	[58]	41	Bordeaux, France	2012	[112]
19	Udine, Italy	2014	[114]	42	Munich, Germany	2014	[112]
20	Patras, Greece	2016	[115]	43	Italy	2010	[112]
21	Case I—San Gregorio, México	2014	[70]	44	Lisbon, Portugal	2014	[112]
22	Case II—San Gregorio, México	2014	[70]	45	Scottish, UK	2014	[112]

23 Drama, Greece 2016 [70]

Relevant information from the distribution networks was extracted from each case (Table 7) such as location, annual injected volume, mean operating pressure, leakage percentage, leak calculation method, calibration method.

Table 7. Information from case studies.

ID Case	Leakage (%)	Average Pressure (m)	Annual Volume Consumed (m ³)	Energy Consumed per m ³ Injected (kWh/m ³)	Annual Consumption (kWh)	Annual Energy Lost by Leaks (kWh)
1	57.56	54	33,016	0.15	115,093	66,252
2	51.00	93	629,552	0.25	325,600	166,056
3	12.03	50	807,216	0.14	1,250,363	150,387
4	25.00	63	12,772,080	0.17	2,923,529	730,882
5	27.16	70	27,899,748	0.19	7,306,457	1,984,580
6	3.05	60	2,332,019	0.16	393,260	11,975
7	23.00	55	320,397	0.15	62,363	14,343
8	34.94	45	1,436,640	0.12	268,653	93,873
9	34.38	47	513,336	0.13	101,834	35,010
10	31.37	40	1,277,500	0.11	202,904	63,656
11	45.60	41	3,190,939	0.11	659,074	300,538
12	15.00	40	5,361,120	0.11	687,485	103,123
13	15.00	40	7,505,568	0.11	962,479	144,372
14	14.90	59	11,003,226	0.16	2,068,799	308,251
15	17.20	34	4,047,330	0.09	452,881	77,895
16	54.00	30	169,243	0.08	30,077	16,242
17	26.05	40	5,370,085	0.11	791,534	206,194
18	23.92	65	1,577,530	0.18	367,280	87,860
19	28.31	31	8,842,478	0.09	1,052,511	297,966
20	55.00	30	9,855,000	0.08	1,790,325	984,679
21	28.45	40	251,605	0.11	38,328	10,903
22	34.41	40	308,260	0.11	51,230	17,630
23	19.80	30	9,358,600	0.08	953,945	188,879
24	63.27	50	24,588,597	0.14	9,122,084	5,771,888
25	38.24	50	13,766,774	0.14	3,037,055	1,161,332
26	46.06	50	16,231,357	0.14	4,100,160	1,888,637
27	57.00	77	18,049,250	0.21	8,750,213	4,987,622
28	52.50	40	426,919	0.11	97,967	51,432
29	40.00	114	862,194	0.31	446,401	178,560
30	46.86	75	157,946	0.20	60,746	28,465
31	12.00	56	430,151	0.15	74,592	8951
32	30.00	40	2,571,680	0.11	400,623	120,187
33	13.33	30	81,994	0.08	7734	1031
34	5.57	46	12,210,000	0.13	1,620,776	90,252
35	20.70	38	130,180,000	0.10	16,998,768	3,518,629
36	74.96	42	1,780,000	0.11	813,740	610,019
37	22.55	40	6,630,000	0.11	933,040	210,370
38	23.22	40	10,120,000	0.11	1,436,620	333,540
39	47.00	30	530,009	0.08	81,751	38,423
40	17.54	44	330,660,000	0.12	48,079,900	8,433,766
41	15.87	37	40,010,000	0.10	4,795,237	76,1229
42	13.33	60	91,000,000	0.16	17,167,500	2,289,000
43	24.71	44	33,830,000	0.12	5,387,107	1,330,890
44	12.99	51	26,524,047	0.14	4,236,514	550,334
45	58.69	45	147,752,776	0.12	43,854,719	25,736,535

The FAVAD method is the most used in research analyzed, there is an extensive literature of its application in real case studies where the calibration methods applied in the networks are shown. With the information extracted from each case study, key performance indicators (KPIs) such as volumetric efficiency of the system, the volume of water lost annually and its associated cost, the energy lost in kWh for each m^3 injected and its cost was calculated.

3.2. Influence of the Leakages in the KPI of the Water Systems

Water leaks in distribution networks condition the quality and quantity of water for users. The existence of leaks increases the loss of water and the operating costs of the networks management because more energy is required in the flow to guarantee the minimum operating pressure.

For each of the case studies shown in Table 7, the following information was extracted: annual volume injected into the network, percentage of leaks, and mean pressure. The analysed data enabled the calculation of the following parameters: volumetric efficiency of the network, energy consumed for each cubic meter of water injected into the network, annual energy consumption, energy lost due to leaks, and the associated costs due to loss of water and energy.

As mentioned above, volumetric efficiency is the ratio of recorded volume to total volume over a period. For the 45 case studies, the distribution corresponding to the volumetric efficiency is shown in Figure 2a resulting in a mean of $\eta = 0.73$ and quartiles of $\eta = 0.53$ and $\eta = 0.83$. Figure 2b shows the leakage percentage in the distribution networks with an average leakage value of 27% and quartiles of 17% and 46%.

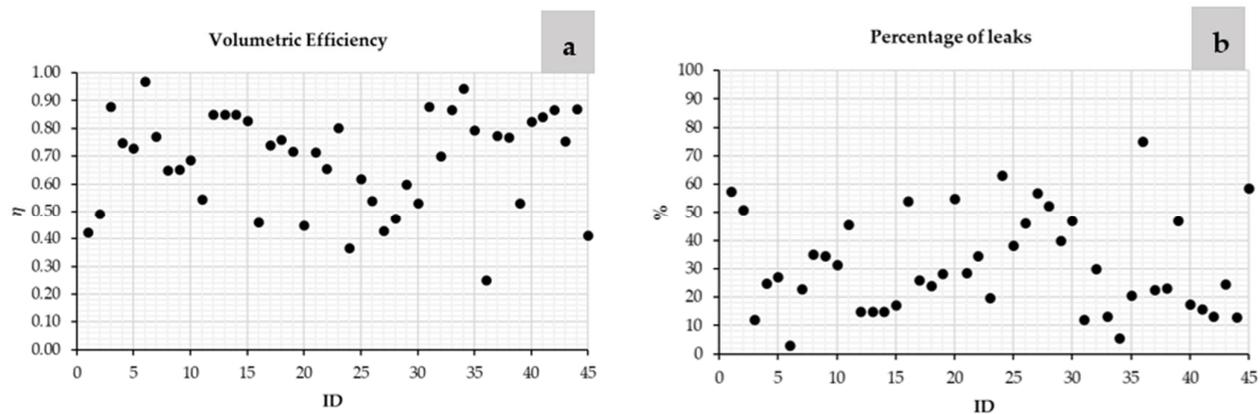


Figure 2. Volumetric efficiency (a) and percentage of leaks (b).

Each of the distribution networks has different physical characteristics, such as diameters, lengths, valves, pressures, and flow rates. The configuration of each network will depend on the design conditions, this will make each of them operate differently, some of the networks transport the flow by gravity and others under pressure using pumps.

The average pressure of each distribution network was obtained as shown in Figure 3a. Its average value was 44 m w.c. and the quartiles were 40 and 45 m w.c. Two atypical values are observed that correspond to networks where their topography had high slopes. Figure 3b shows the annual volume injected into the network. The average value was 7.26 Hm^3 and the quartiles were 1.21 and 24.24 Hm^3 . There were 5 cases above 50 Hm^3 . These reached annual values of up to 400 Hm^3 .

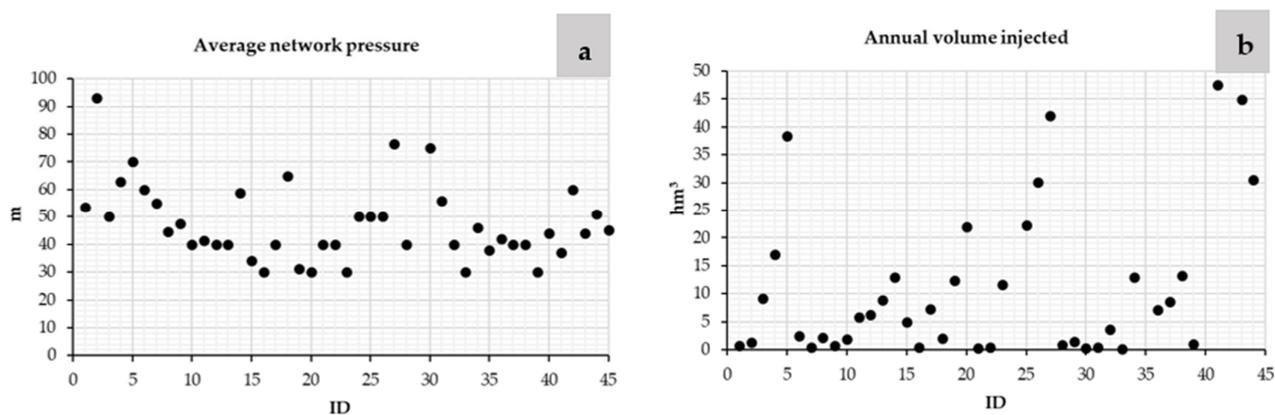


Figure 3. Average pressure (a) and annual flow injected into the network (b).

The average pressure of each case study was related to the percentage of leaks and the volumetric efficiency of the system. The ratio between the average pressure and the leakage percentage is shown in Figure 4a. The values are between 50 and 500, being the average equal to 243. Figure 4b shows the relationship between the average pressure of the network and the volumetric performance. This ratio shows values between 40 and 200, being the average value equal to 78 m.w.c.

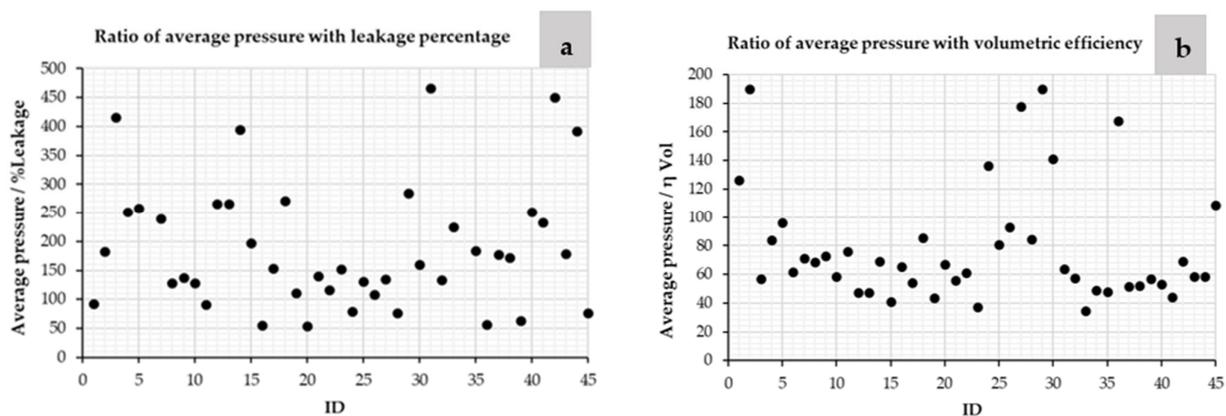


Figure 4. Ratio between average pressure with leakage percentage (a) and volumetric efficiency (b).

In energy terms, the value of average pressure and flow injected into the network-enabled to estimate the average energy consumption of the network per unit volume ($\text{kWh} \cdot \text{m}^{-3}$). Subsequently, when the percentage of leaks associated with each network was known, the annual unit energy lost was determined [116].

Figure 5a shows the average value of the energy lost for the case studies is 178,600 kWh with quartiles 60,600 kWh and 817,100 kWh. Above 1,000,000 kWh there were 10 cases that reached values of up to 10 GWh.

Once the percentage of leakage from the network was known, the water lost in the network was calculated. The average value of the annual volume leaked was 1,325,000 Hm^3 , being the quartiles between 470,400 Hm^3 and 7,793,000 Hm^3 . Figure 5b shows the energy lost for each m^3 of water injected into the distribution networks. The average value was 0.12 kWh/m^3 , being the quartiles between 0.11 and 0.15 kWh/m^3 .

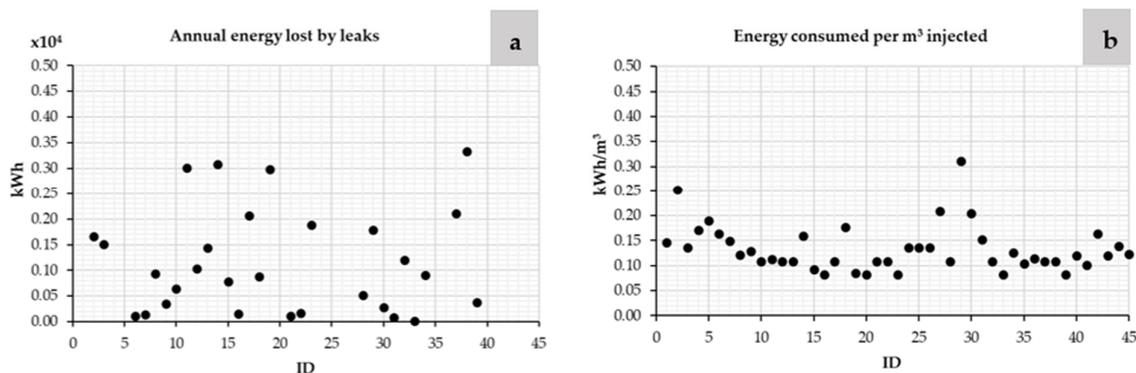


Figure 5. Annual energy lost by leaks (a) and energy consumed per m³ injected (b).

To estimate the lost economic value of energy and water, the following rates were established: for water 1.9 €/m³ and energy 0.11 €/kWh. Once the value of the volume of water lost in leaks had been calculated, its average annual cost corresponding to a value of €2,097,000 with quartiles €885,600 and €11,180,000 as shown in Figure 6a. Above €10,000,000 13 cases reached values of up to €100,000,000. Similarly, the average annual energy cost lost is €121,400 with quartiles €51,270 and €647,400 as shown in Figure 6b.

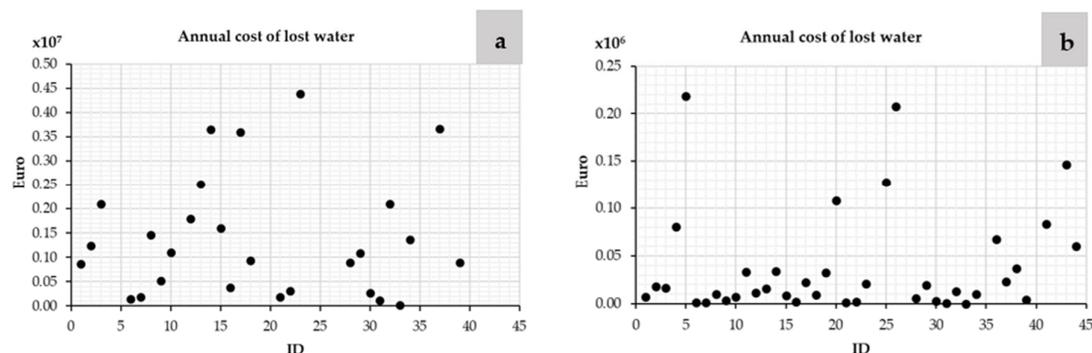


Figure 6. The annual cost of lost water (a) and the annual cost of lost water (b).

The leak calculation methods such as BABE, MNF, and water balance were applied and compared in different case studies, which were defined by [117,118]. The analysis was established to determine the real water loss in million m³/year as shown in Table 8.

Table 8. Leak method comparison.

Method	Zarqa, Jordan	Sana’a, Yemen	Mwanza, Tanzania	Gavankola, Iran
Water Balance	40	7.1	12.2	-
MNF	16.2	-	12.2	34.9
BABE	4.2	0.4	5.8	39.4

Using the MNF and BABE methods, the average of real water loss in the studied region was calculated at 28% and 37%, respectively. According to the results of the BABE method, the highest leakage values were related to the reported bursts and the background leaks from connections [118]. The water balance method is the difference between the volume of water injected into the network concerning the volume of water consumed. The result of the difference corresponds to leaks, where it is observed that the results obtained are greater than those obtained with the methods calculation of BABE and MNF leaks.

Applying each method requires verification for the factors and assumptions in each method and their sensitivities and uncertainties. Conducting MNF analysis in one or several small areas in the network (DMAs) and extrapolating it to the entire network might be justifiable in some cases, but it is not very rational because every DMA differs in terms of the mains length, service connections, pressure, and burst frequencies. The component analysis of the leakage method (BABE) remains the only way to break down the leakage into subcomponents, enabling the water utilities to understand the nature and behaviour of the leakage in their systems. However, the component analysis of the leakage analyses only a small portion of the total leakages [117].

3.3. Pump Working as Turbine Using Leakages Models

Micro and mini-hydropower generators can be installed in water distribution systems to ensure both pressure control for leakage reduction and energy production [119]. Jain [26] in his research, shows a wide state of the art on pumps working as turbines, from their selection, methods, and cases of application. In water distribution networks (WDNs), the PATs can be used instead of pressure-reducing valves (PRVs) for both pressure reduction and energy production [27].

Reference [106] showed a table with the advantages and disadvantages of installing PATs in the distribution system compared to other methods that reduce pressure and leakage. Ref. [120] implemented a model through Simulink in Matlab to reduce the pressure in the distribution networks through the installation of PATs. This model was implemented in a real case study by [121].

Reference [27] mentioned the importance of the implementation of PATs for pressure management, benefiting the reduction of leaks in the distribution network. In the applied case, this research shows that the proposed optimization model allows a leakage reduction of 26.6% and an energy recovery of 182.15 kWh/day.

Reference [122] showed a methodology for the optimal location of PAT within a water distribution network to produce energy and reduce leakage. The total amount of saved water has been accounted as 275,870 m³/year while the net present value (NPV) resulted as € 75,936. Reference [50] estimated a reduction in pressure of approximately 1 bar and a flow reduction of the order of 50 m³/h were achieved just after the operation of the PAT system in Antalya, Turkey. Reference [123] showed the possibility of optimally managing a water distribution system by reducing leaks and recovering energy. The study considered the installation of between 10 to 15 PATs in the network of the Sorrento Peninsula (Italy) that distributes water in a network of 90,000 users.

Reference [124] estimated that the installation of PATs represents a 45% reduction in water savings equivalent to 7000 m³/year. An optimization method is proposed that uses an objective function for the reduction of leaks and energy production by implementing PATs in a distribution system [125]. This study proposed a new preliminary model to optimize the location of PATs in a water distribution network. The optimization aims to determine the best number and location of turbines to reduce pressure, thus water leakage, and produce energy. The implementation of 6 PATs in the case study would mean a water-saving of 929 m³/day and an average power of 14.53kW. Reference [126] proposed a method for the optimal location of PAT through a mixed integer non-linear program (MINLP). Using the model in the case study, it achieves an average water reduction of 7.88 L/s and an energy recovery of 1958 kWh/day.

The Leakage Control and Energy Recovery Using Variable Speed Pumps as Turbines in two water distribution networks are proposed by [127]. It demonstrated a methodology through flow diagrams for the selection of PATs and different combinations of PATs applications operating constantly and variably, obtaining as results leakage reductions and energy recovery.

Table 9 shows the influence of the installation of PATs in various case studies, where relevant information such as flow, pressure, leakage percentage, annual energy and water recovered per installation were extracted. It is observed that for all the case studies the

implementation of PATs supposes a reduction of leaks in the distribution network. Because not all the case studies show the percentage of total network leakage, 27% leakage is adopted as the average value since this was the value obtained in the leakage study cases previously analyzed.

Table 9. Information extracted from case studies.

Reference	ID	Flow (L/s)	H (m)	Annual Energy Recovered by Installing PATs (kWh/year)	Annual Volume Recovered by Use of PATs (m ³)	Leakage Reduction by Use of PATs (%)	η_v before Installing PATs	η_v after Installing PATs
						21		
[102]	46	29	59	43,800	-	20	-	-
[102]	47	74	54	87,600	-	32	-	-
[102]	48	19	67	39,420	-	18	-	-
[102]	49	33	55	43,800	-	21	-	-
[102]	50	19	63	35,040	-	29	-	-
[102]	51	14	71	26,280	-	65	-	-
[102]	52	31	65	52,560	-	21	-	-
[128]	53	29	21	28,470	22,813	63	0.73	0.90
[128]	54	183	33	169,360	1,634,590	52	0.73	0.87
[128]	55	72	36	130,305	98,185	63	0.73	0.90
[27]	56	302	61	55,626	2,475,059	26	0.73	0.80
[27]	57	212	39	71,876	667,554	10	0.73	0.76
[27]	58	314	50	66,485	1,829,359	19	0.73	0.78
[120]	59	187	70	54,985	-	-	-	-
[122]	60	110	45	125,213	339,085	10	0.73	0.76
[125]	61	110	45	113,880	328,865	9	0.73	0.75
[126]	62	350	45	714,670	248,504	3	0.73	0.74

Pressure Management (PM), which is usually the best way to reduce the Non-Revenue Water (NRW) level in a water distribution network, is achieved mainly by forming (dividing the network into several) District Metered Areas (DMAs). In the city of Kozani (Greece), [102] analyzed the possibility of implementing PATs in the water distribution system. The implementation of PATs in the case study would suppose a reduction of leaks between 20% and 40% approximately. The reduction of leakages decreased 325,500 kWh of lost energy. The investigation shows the possible combinations of PATs with different installed power values from 5 to 12 kW.

Figure 7a shows the influence of the installation of PATs in a WDN. Figure 7a shows the reduction of the leakage flow after the installation of PATs varies from 18 to 65%, while Figure 7b shows the pressure reduction with values from 29 to 55%.

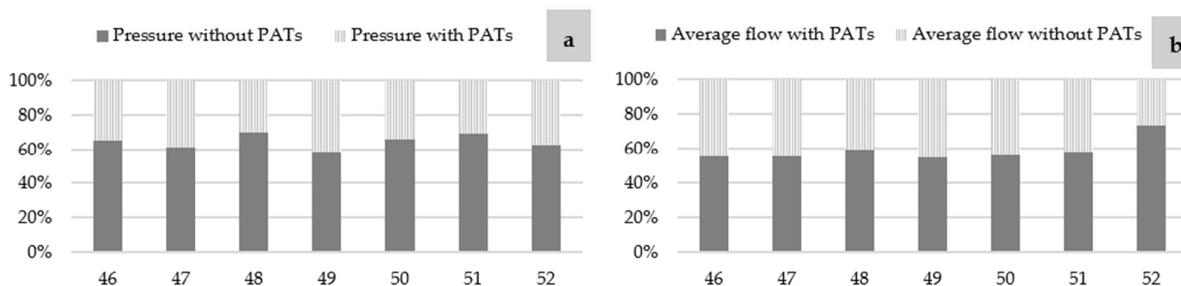


Figure 7. Influence of the installation of PATs. Comparison of network pressure by the installation of PATs (a); Comparison of flow injected into the network by the installation of PATs (b).

Once the leakage value was known, the volumetric efficiency was calculated, which was 0.73 before the consideration of the PATs systems. For all the case studies, the reduction of leaks significantly increases the volumetric performance after installing PATs in the distribution network.

Figure 8 shows the efficiency indicators calculated in water distribution networks with the installation of PATs for the selected case studies. Figure 8a shows the annual energy consumption, the average value of the cases is 93,000 kWh/year. Figure 8b shows the energy consumed for each m³ injected into the network with an average value of 0.10 kWh/m³.

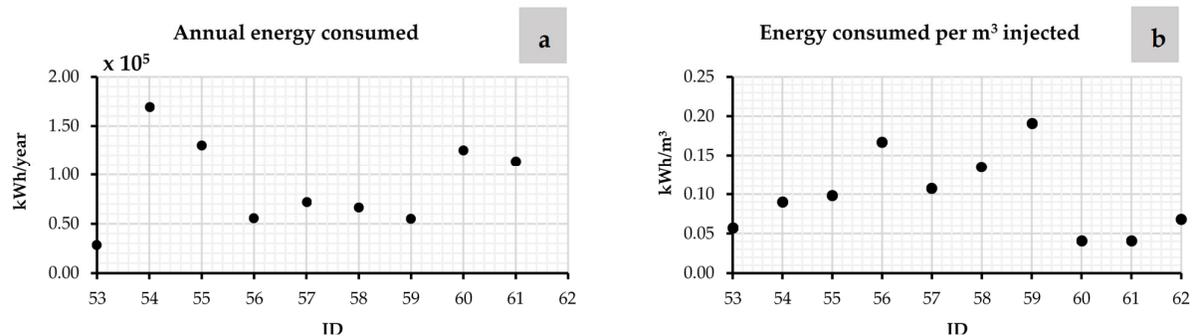


Figure 8. Efficiency indicators in networks with PATs installed. Annual Energy consumed (a); Energy consumed per m³ injected (b).

3.4. Energy Index Calculation Case Study

For each of the study cases, the energy indices were calculated. The energy indices determined in Table 10 are: absolute energy consumed annually (IAAE) in kWh/year, recovered energy (IER) in kWh/year, percentage of recoverable energy (ERP) in %, absolute energy consumed per unit volume (IAEFW) in kWh/m³. Besides, a new index was proposed to consider the influence of the PATs system in the reduction of leakage. This index, called index reduction leakage flow as a function of generating power (IRLGP), shows the ratio between reduction of the leakage volume for each installed power. The units of this index are m³/kW.

Table 10. Calculable energy indices with available data.

Reference	ID	IAAE (kWh/Year)	IER (kWh/Year)	ERP (%)	IEFW (kWh/m ³)	IRLGP (m ³ /kW)
[128]	53	52,335	28,470	54	0.06	8
[128]	54	518,965	169,360	3	0.09	-
[128]	55	222,745	130,305	58	0.10	4
[27]	56	1,583,106	55,626	4	0.17	2
[27]	57	710,516	71,876	10	0.11	16
[27]	58	1,349,189	66,485	5	0.14	13
[120]	59	1,124,897	54,985	5	0.19	-
[122]	60	141,794	125,213	29	0.12	5
[125]	61	141,794	113,880	27	0.12	5
[126]	62	751,937	714,670	53	0.12	4

Annual energy consumption in the analyzed distribution networks ranges from 141,794 to 1,583,106 kWh. By installing PATs in the distribution networks, annual energy recovery values are obtained from 28,470 to 714,670 kWh. Using the IAAE and IER values, the ERP index was determined, resulting in recovery values from 3 to 58%.

The relationship of the annual volume of water injected into the network concerning IAAE allowed the calculation of the IEFW index. This shows values between 0.06 and

0.19 kWh/m³. The difference between the total leaked volume in the network compared to the volume recovered by using PATs is used to calculate the reduction of the volume leaked by the installation of PATs. Considering the relationship between the reduced volume and the annual energy recovery, the IRLGP index was calculated, which shows values between 2 and 16 m³/kW.

4. Conclusions

Several leakage methods have been implemented in distribution networks, and for each formulation it was possible to extract its main characteristics, advantages, and disadvantages. The FAVAD method is the most widely used in the case of studies for estimating leaks in water distribution networks. It was evidenced that through experimentation and estimates different values of leakage exponents have been determined for each type of formulation depending on the type of leak, age and pipe material.

Different methods for calibrating leak models are proposed by various authors, with genetic algorithms being the most widely implemented in the models. The analysis carried out on sixty-two case studies allowed the extraction of relevant information from the network to calculate and compare various efficiency indices such as volumetric performance, percentage of leaks, energy lost due to leaks, annual energy consumption for each cubic meter injected into the network and costs for loss of water and energy.

The analyzed case studies, which had installed PATs systems showed the implementation of micro-hydropower systems causes a considerable reduction in the pressure of the network, reducing the level of leaks between 4% and 63%, while the recovered energy was estimated between 28,470 and 714,670 kWh/year.

To improve the sustainable development goals, the use of efficiency indicators are an important tool for the analysis and management of water distribution networks. In this line, the researchers analyzed the variation of the different indicators in 62 cases studies in energy, economic and environmental terms deeply. The proposal of sustainable measurements should start, knowing the state of the water system. The installation of PATs in the distribution networks is a solution to reduce the volume of leakage and an opportunity to recover clean energy. The analysis of the PATs systems should be considered when leakages are considered. The influence of the leakages can cause differences in the selection, operation and regulation of the recovery machines and it should be considered by water managers in energy optimization procedures as well as the energy analyses. Besides, the installation of PATs systems in real case studies should consider the significance of the leakages in the operation of these recovery machines. This research shows the need to apply the analysis of indicators for the improvement of the energy analysis. Future works should develop around this topic. This new research should develop optimization procedures in which the leakages will be considered as a decision variable. The analysis of the leakages will include the influence of these water losses in the selection and regulation of the PATs systems, and therefore, its impact in the recovered energy.

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