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Assessing the Agricultural Drainage Water with Water Quality Indices in the El-Salam Canal Mega Project, Egypt

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Abstract: The water quality index (WQI) is considered one of the most promising methods for the classification of water quality (WQ), which also contributes to water resource management. This study adopted the irrigation WQ index (IWQI) and an analogous index based on a fuzzy logic approach, namely, the fuzzy logic water reuse index (FWRI) to assess the water quality in the El-Salam canal project in Egypt where agriculture drainage water (ADW) is expected to be reused for irrigation. Simulated WQ data using a one-dimensional hydrodynamic model indicated that the WQ deteriorated towards the downstream of the canal due to the polluted water discharged from canal feeders (e.g., the El-Serw and Bahr Hadous drains). The comparison of the FWRI and IWQI indices showed that the FWRI was more sensitive to variations in the WQ parameters compared to the IWQI. In contrast, the Z-test indicated that the indices have different statistical properties. Moreover, a chi-square test (χ^2) illustrated that the FWRI and IWQI values can both reasonably explain the current situation. However, the FWRI was more relevant to the official classification than the IWQI. Overall, the FWRI proved its capability and accuracy for the assessment of water quality in the El-Salam canal.

Keywords: agricultural drainage water; El-Salam Canal; fuzzy logic water reuse index; MIKE 11; water quality index

1. Introduction

Water scarcity is a serious global issue, including Egypt and other African countries [1]. Annually, 17 billion cubic metres (BCM) of agricultural drainage water (ADW) is produced in Egypt, and this represents a potential backbone for non-conventional water resources in this country [2]. While approximately 55% of ADW is officially reused for irrigation purposes, most of the drainage canals are likely polluted by discharges of untreated domestic and industrial wastewater [3]. The fluctuation

in ADW quality due to pollution is considered to be the main issue in the appropriate reuse of ADW. The most common challenge involved in decisions regarding ADW reuse is how to determine whether the quality of the drainage water is suitable for reuse [4].

According to the Food and Agriculture Organization (FAO), there are a number of different water quality guidelines related to irrigated agriculture [5]. Each has been useful, though none has been entirely satisfactory because of the wide variability in environmental conditions. The FAO is mainly concerned about the effect of water quality upon soil and crops, therefore, five categories are applied to water quality-related problems in irrigated agriculture: (a) salinity hazards (electrical conductivity (EC) and total dissolved solids (TDS)), (b) infiltration and permeability hazards (EC and sodium absorption ratio), (c) specific ion toxicity (sodium adsorption ratio (SAR), boron, and chloride), (d) trace element toxicity, and (e) miscellaneous impacts on sensitive crops (pH, nitrate, and bicarbonates). In the Egyptian standards (Law 48/1982), chemical water quality parameters (e.g., pH, TDS, dissolved oxygen (DO), biochemical oxygen demand (BOD₅), nitrates (NO₃-N), phosphate, and heavy metal) are selected to classify the suitability of ADW for reuse in irrigation. In addition, the expected water quality level may be different depending on the specific types of irrigation [6]. Consequently, there is a need for the development of a comprehensive approach to spatiotemporally assess the water quality in drainage canals in countries with potential water scarcity.

Several approaches to water quality (WQ) assessment have been applied to provide an accurate or reasonable evaluation [7,8]. Among them, the WQ index (WQI) is considered one of the most promising methods for WQ classification and a set of WQ parameters is employed depending on the purposes [4]. Basically, the WQI includes a mathematical approach to convert the WQ parameters at a certain site and time into a number ranging from 0 to 100, with the number indicating the real WQ status against the standards [9]. The first WQI was proposed by Horton [10], and numerous WQIs have been further developed, including WQ indices developed by the National Sanitation Foundation (NSFWQI) [11], Florida Stream (FSWQI) ((SAFE) 1995) [12], Canada (CWQI) [13], British Columbia (BCWQI) and Oregon (OWQI) [14]. Most of these indices were established on the basis of the WQ index for the National Sanitation Foundation, i.e., NSFWQI [9,15,16]. However, these indices generally provide a rough estimation, and in some cases, result in imprecise outcomes [17], therefore, it is unlikely that the existing indices are capable of handling environmental and experimental uncertainties in an appropriate manner [18].

WQ assessment approaches utilizing artificial intelligence (AI) computational methods have been developed in the last two decades to integrate the distinct parameters involved [19,20]. The advanced AI techniques in WQ/HD include the knowledge-based system (KBSs), genetic algorithm (GA), artificial neural network (ANN), and fuzzy inference system (FIS) [21]. Among them, the fuzzy logic was initially proposed by Zadeh [22] in 1965, and it is useful for modelling complex and imprecise systems. Also, many aquatic systems have a paucity of WQ data due to cost constraints [23]. Moreover, the fuzzy logic method was recently tested with real environmental problems to diminish the uncertainty and imprecision of the criteria which were utilized in decision-making procedures [24,25]. Furthermore, the fuzzy inference system (FIS) has provided an alternative tool to deal with information that is not well identified, or not precise [20,26].

Since these approaches describe the WQ status at a specific time and location according to the monitoring location distribution and schedule, WQ simulation is an ideal approach in the holistic evaluation of temporal and spatial water quantity and quality conditions [4]. To date, several WQ models have been developed for the simulation of water quality in any aquatic system, such as the Environmental Fluid Dynamics Code (EFDC), Delft 3D, SOBEK software, MIKE software and the Water Quality Analysis Simulation Program (WASP) [27]. Among them, MIKE 11 represents the most widely used hydrodynamic (HD) and WQ simulation software. The MIKE 11 has proved its computational stability, high accuracy and reliability, and consequently it can be used for the comprehensive design of all types of channel systems [28,29].

Therefore, this study aimed to assess the WQ in agricultural canals. As an agricultural canal, the El-Salam Canal was selected because it is the largest, currently on-going ADW reuse project in Egypt. The HD and one-dimensional WQ simulation model were employed to obtain a spatiotemporal data set for WQ parameters. The WQ assessment was then performed to investigate the suitability of the WQ index for water reuse via a comparison between the results generated by the irrigation WQ index (IWQI) and the fuzzy logic water reuse index (FWRI). Moreover, statistical assessment of both indices (FWRI and IWQI) was performed to verify these indices in the WQ assessment.

2. Materials and Methods

2.1. Study Area

The El-Salam Canal is one of the largest on-going projects for ADW reuse in Egypt, in which 0.872 BCM/year of the Nile River is mixed with 0.255 BCM, 0.980 BCM and 1.235 BCM of ADW from the Faraskor, El-Serw and Bahr Hadous drains, respectively. The mixed water in the El-Salam Canal is mainly used for the reclamation of 620,000 hectares of land located along the Mediterranean coast of Egypt (220,000 hectares extending west of the Suez Canal and approximately 400,000 hectares extending east of the Suez Canal) [2]. The canal is in the northeast region of the Nile Delta with a total length of approximately 88 km (Figure 1a). The flow rates of these drains are controlled using pumping station units to keep the total dissolved solids (TDS) of the mixed water in the El-Salam Canal at a level of 1200 mg/L to satisfy the Egyptian standards of reuse for irrigation purposes [30].

Figure 1b shows the ADW supply sites from the Faraskor, El-Serw, and Bahr Hadous drains at distances of 1.80 km, 17.85 km, and 54 km from the intake, respectively. Pump Stations No. 1 and No. 2 have been constructed along the main stream of the canal to maintain a suitable head for gravity flow (Figure 1b). These pump stations divide the canal into three reaches (Figure 1c). The first reach extends from 0.0 km to 22 km along the canal, with a bed width of 38 m and a berm width of 2.8 m. The second reach spans from 22 km to 53 km along the canal, with bed and berm widths of 44 m and 6.4 m, respectively. The third reach is 35 km long, spanning from 53 km to 88 km along the canal with a bed width ranging from 46 m to 54 m and a berm width of 6.5 m. The side slope for all cross-sections along the El-Salam Canal is 2:1 (horizontal:vertical).

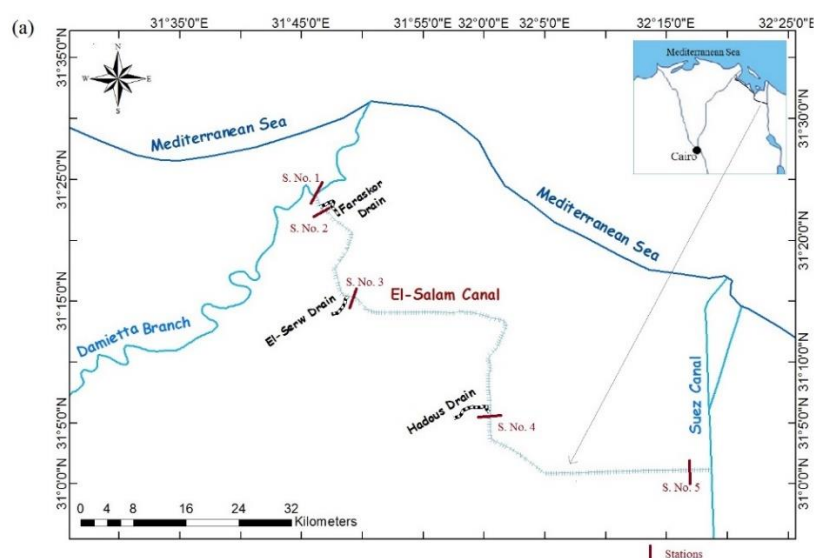


Figure 1. Cont.

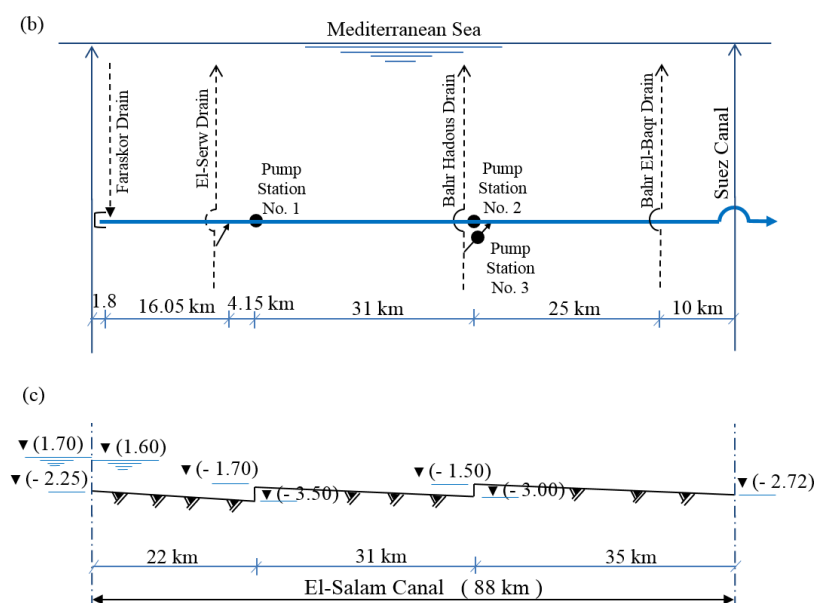


Figure 1. (a) The El-Salam Canal project; (b) schematic diagram; (c) longitudinal cross-section, of the El-Salam Canal connected with the Nile River and agricultural drains.

In this study, the average monthly records of water discharges along the canal were obtained from the Ministry of Water Resources and Irrigation (MWRI) from September 2012 to August 2014 for seven locations: the intake of the canal (0.0 km), the Faraskor drain (1.80 km), the El-Serw drain (17.85 km), Pump Station No. 1 (22 km), Pump Station No. 2 (53 km), the Bahr Hadous drain (54 km) and the Suez canal (88 km). The average monthly WQ data were collected by the Drainage Research Institute (DRI) from September 2013 to August 2014 at five locations along the canal at 0.0 km, 1.80 km, 17.85 km, 54 km and 88 km. Table 1 summarizes the average discharges from September 2012 to August 2014 and WQ from September 2013 to August 2014 for the locations along the El-Salam Canal, as well as Egyptian standards for the reuse of drainage water in irrigation according to Law 48/1982.

Table 1. The average discharges from September 2012 to August 2014 and WQ parameters from September 2013 to August 2014 for the locations along the El-Salam Canal.

Location (km)	September 2012 to August 2013		September 2013 to August 2014			
	Q (m ³ /s)	Q (m ³ /s)	TDS (mg/L)	DO (mg/L)	BOD ₅ (mg/L)	NO ₃ -N (mg/L)
Intake of the canal (0.0)	27.6	12.8	323.3	9.2	12.1	11.8
Faraskor drain (1.80)	7.9	7.7	934.7	3.5	40.9	7.2
El-Serw drain (17.85)	30.5	31.0	1034.7	2.9	28.2	12.6
pump station No.1 (22)	65.1	74.1	-	-	-	-
pump station No.2 (53)	58.4	59.2	-	-	-	-
Bahr Hadous drain (54)	38.4	27.3	1443.7	2.7	36.9	12.5
Suez Canal (88)	53.3	27.9	919.1	3.9	19.7	14.0
Egyptian standards for drainage water reuse in irrigation			<1200	>5	<30	<30

2.2. Water Quality Modelling

The software 'MIKE 11' was used to simulate the water quantity and WQ along the El-Salam Canal. MIKE 11 was initially developed by the Danish Hydraulic Institute (DHI) to simulate water flows, WQ and sediment transport in rivers, estuaries and irrigation systems [31]. The HD module is a

one-dimensional, non-steady, non-uniform flow simulation model that describes the water motion using Saint-Venant equations. The WQ module (ECO-Lab) was coupled to the advection-dispersion (AD) module [32]. The ECO-Lab module deals with the biochemical transformation processes along the canal and the AD module is used to simulate the simultaneous transport processes. The WQ template of the ECO-Lab module is divided into six levels starting from the simplest relationship between biochemical oxygen demand (BOD₅) and chemical oxygen demand (COD), and leading to complex processes, such as nitrification, denitrification, sediment precipitation, resuspension, and oxygenation. A complete description of the model theory is found elsewhere [31].

The 88 km long El-Salam Canal (west of the Suez Canal) was modelled, including a number of structures, namely the head regulator, two siphons and two pump stations as shown in Figure 1. The boundary editor of MIKE 11 was used to define the water levels and inflow hydrographs. The initial upstream water level of the El-Salam Canal was set to 1.6 m to facilitate the inflow to the canal from the Damietta Branch Dam, which was built to raise the water level to 1.7 m above the mean sea level (MSL) at the El-Salam Canal intake. The simulation time step was set to five seconds to ensure the stability of the numerical calculations and to keep the Courant number in the desired model's range. A WQ template was developed to simulate the particular WQ parameters such as total dissolved solids (TDS), dissolved oxygen (DO), BOD₅ and nitrates (NO₃-N). The WQ data for the canal over one year from September 2013 to August 2014 were extensively used for the simulation process. Moreover, the integration solution was conducted using the Euler integration method [31].

The calibration process of the HD model was performed by modifying Manning's roughness coefficient (M) values to minimize the difference between the simulated and observed discharge data. The HD data over one year from September 2012 to August 2013 were used in the El-Salam Canal model calibration process. The model was continuously run to obtain the least difference between the simulated and observed discharges at two locations (Pump Station No. 1 and Pump Station No. 2 at 22.0 km and 53.0 km along the canal, respectively). For validation, the calibrated model was run using a completely different year data set (from September 2013 to August 2014) to assess the ability of the calibrated model to predict the water quantity and WQ under different conditions. The calibration and validation accuracy were tested based on calculation of the root mean square error (RMSE) and normalized objective function (NOF) as follows:

$$\text{RMSE} = \sqrt{\frac{\sum (\text{Simulated value} - \text{Observed value})^2}{\text{Number of Observations}}} \quad (1)$$

$$\text{NOF} = \frac{\text{RMSE}}{O_{\text{mean}}} \quad (2)$$

where O_{mean} is the mean of the observed data. Model simulations are acceptable for NOF values ranging from 0.0 to 1.0, where the ideal value for the coefficient is 0.0 [4].

2.3. Fuzzy Inference System (FIS)

Membership functions, fuzzy set operations and inference rules are the principles that are used by the FIS to develop a WQ reuse index based on fuzzy logic. A membership function can be varied in form, such as trapezoidal, triangular, etc., and defines each point in the input space plotted to a membership value between 0 and 1. The universe of discourse is the domain of the input set, while the output-axis represents the membership value (μ). A fuzzy set A is derived from Equation (3) where the universe of discourse is X and its elements are denoted by x.

$$A = \{(x_1, \mu_A(x)) | x \in X\} \quad 0 \leq \mu_A(x) = 1 \quad (3)$$

The degree of membership of element x is ($\mu_A(x)$) value in fuzzy set A. The relationships among the fuzzy subsets are union (OR), intersection (AND) and additive complement (Negation) (NOT).

These basic operators express the core of fuzzy logic. Two fuzzy sets A and B are defined on the universe X, for a given element x belonging to X. Equations (4)–(6) express the operations of the fuzzy set.

$$\text{OR : } \mu_{A \cup B}(x) = \mu_A(x) \cup \mu_B(x) = \max(\mu_A(x), \mu_B(x)) \quad (4)$$

$$\text{AND : } \mu_{A \cap B}(x) = \mu_A(x) \cap \mu_B(x) = \min(\mu_A(x), \mu_B(x)) \quad (5)$$

$$\text{NOT : } \mu_{\bar{A}}(x) = 1 - \mu_A(x) \quad (6)$$

The relationships among the subsets of the inputs and outputs are defined as the inference rule. To generate a new output subset, the if-then rule is implemented with each rule consisting of two parts. The first part for the ‘if’ is called the antecedent, while the ‘then’ part is termed the consequent, with the rule form:

IF A is a THEN C is c.

IF B is b THEN C is c.

where a, b, and c are the linguistic values for the subsets defined for fuzzy sets in the universes of discourse A, B, and C, respectively.

2.4. Water Quality Indices

2.4.1. Water Reuse Index Based on Fuzzy Logic

For the evaluation of the ADW quality for reuse in irrigation, a fuzzy model (i.e., FWRI) was built for four major WQ parameters (TDS, DO, BOD₅ and NO₃-N) according to the Egyptian standards (see Table 1). The prediction of the fuzzy model depends on the number of fuzzy sets used in the mapping process, since it facilitates giving more continuity to the universe of discourse [33].

A triangular membership function was utilized through the FIS for the fuzzy sets of parameters in terms of TDS, DO, BOD₅ and NO₃-N, as shown in Figure 2. The fuzzy sets in this index were defined by the linguistic variables ‘very low’ (VL), ‘low’ (L), ‘medium’ (M), ‘high’ (H), and ‘very high’ (VH). According to the developed fuzzy sets and linguistic terms for the fuzzy-based index (Table 2), the fuzzy sets were utilized according to Equation (7) as follows:

$$f(x; a, b, c) = \left\{ \begin{array}{ll} 0 & x < a \text{ or } c < x \\ \frac{(a-x)}{(a-b)} & a \leq x \leq b \\ \frac{(c-x)}{(c-b)} & b \leq x \leq c \end{array} \right\} \quad (7)$$

The FIS normalized the specified WQ parameters to a value between 0 and 100—values near 100 show that the water is more suitable for reuse for agricultural purposes, as shown in Table 2. The rules in the FIS were set based on the Mamdani systems to achieve the maximum possible number of WQ conditions creating the inference rules (75 rules). The generated inference rules are given as follows, e.g.: if (DO is VL) and (BOD₅ is H) then (FWRI is VB), if (TDS is VH) and (DO is VL) then (FWRI is VB) and if (DO is H) and (NO₃ is M) then (FWRI is VG).

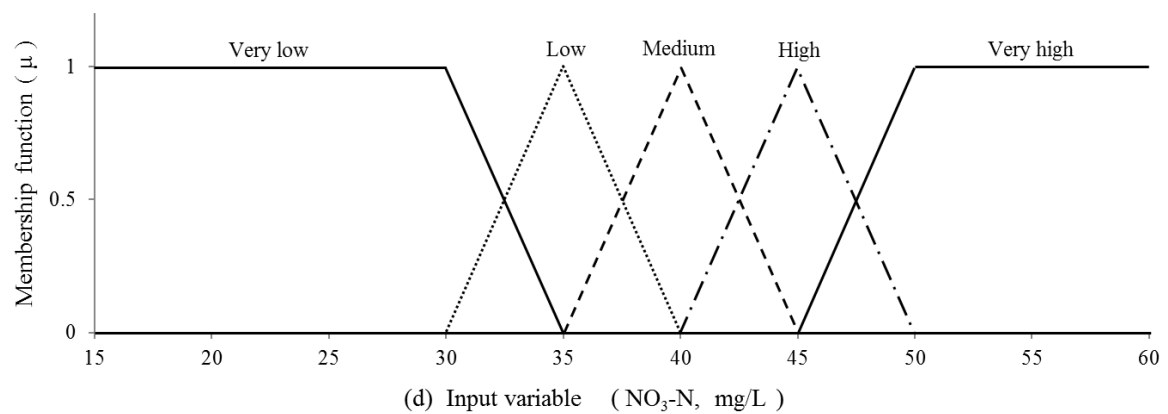
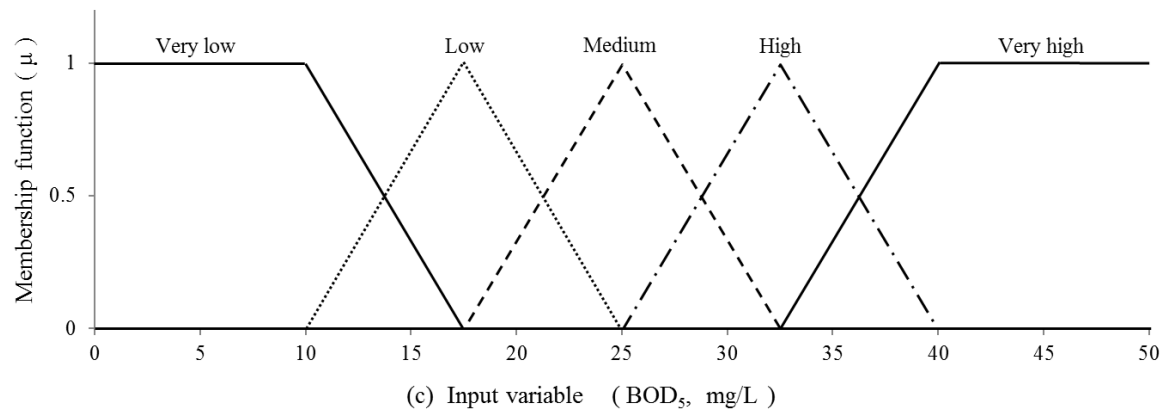
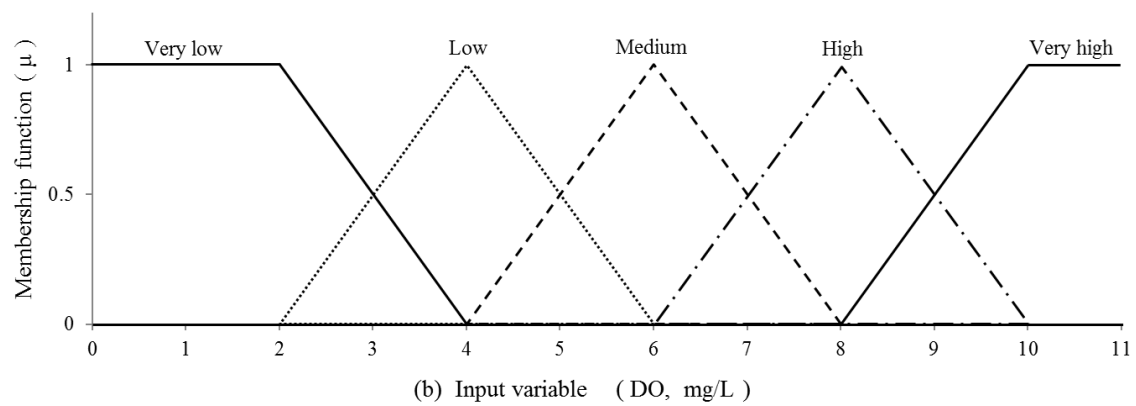
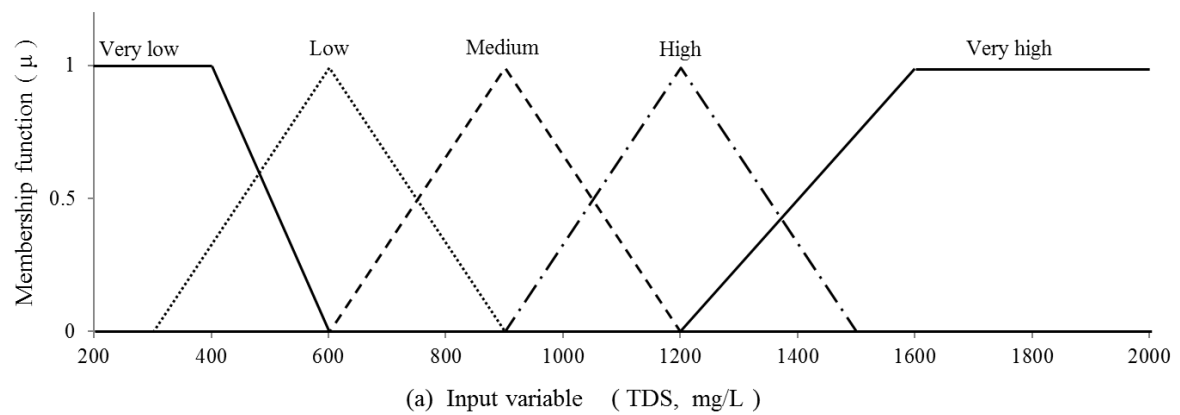


Figure 2. The fuzzy set membership functions of the input variables: (a) TDS; (b) DO; (c) BOD₅ and (d) NO₃-N.

Table 2. Fuzzy sets and linguistic terms for the fuzzy-based index.

Classification		TDS (mg/L)	DO (mg/L)	BOD ₅ (mg/L)	NO ₃ -N (mg/L)	Classification	Fuzzy-Based Index
Very low	a = b	0	0	0	0	Very Bad (VB)	0
	c	200	2	10	10		12.5
	d	700	4	17.5	17.5		37.5
Low	a	400	2	10	10	Bad (B)	12.5
	b	700	4	17.5	17.5		37.5
	c	900	6	25	25		60
Medium	a	700	4	17.5	17.5	Average (A)	37.5
	b	900	6	25	25		60
	c	1100	8	32.5	32.5		80
High	a	900	6	25	25	Good (G)	60
	b	1100	8	32.5	32.5		80
	c	1400	10	40	40		100
Very high	a	1100	8	32.5	32.5	Very Good (VG)	80
	b	1400	10	40	40		100
	c = d	1600	12	50	50		100
Range		0–1600	0–12	0–50	0–50	Range	0–100

The process of defuzzification for the outputs was implemented by the centre of gravity (centroid) method, which is considered the most prevalent and applicable technique. It is based on the derivation of the following algebraic expression [34], where z^* is the defuzzified value.

$$z^* = \frac{\int \mu(z)zdz}{\int \mu(z)dz} \quad (8)$$

All computations were implemented using the “fuzzy logic toolbox” in MATLAB2015.

2.4.2. Irrigation Water Quality Index

The irrigation WQ index (i.e., IWQI) was developed by Meireles et al. [35], (see Equations (9) and (10)). This index was selected to be used for the assessment of ADW for reuse in irrigation. The IWQI was calculated according to the WQ parameters for the classification in terms of: TDS, DO, BOD₅ and NO₃-N. Table 3 shows the proposed water reuse suitability classes, where the index values ranged from 0 to 100, providing a qualitative description of the index output; a higher value indicates better WQ:

$$IWQI = \sum_{i=1}^n q_i \times w_i \quad (9)$$

$$q_i = q_{\max} - \left[\frac{(x_{ij} - x_{\inf}) \times q_{\text{imap}}}{x_{\text{amp}}} \right] \quad (10)$$

where q_i is the sub-index of the parameter calculated on the basis of the tolerance boundaries as shown in Table 3. In Equation (10), q_{\max} is the maximum value for q_i to each class; x_{ij} is the observed value for each parameter; x_{\inf} is the associated value for the lower limit of the class where the parameter belongs; q_{imap} is the class amplitude; x_{amp} is the class amplitude where the parameter belongs. In order to estimate the x_{amp} of the last class for each parameter, the higher limit is counted to be the maximum value for the water parameter. A specific weighting factor (w_i) for the parameters was proposed based on the basic calculation procedures by Abbasi [36], as shown in Table 4.

Table 3. Parameter limiting values utilized in the IWQI.

qi	TDS (mg/L)	DO (mg/L)	BOD ₅ (mg/L)	NO ₃ -N (mg/L)
90–100	TDS ≤ 500	9 ≤ DO	BOD ₅ ≤ 13.75	NO ₃ -N ≤ 13.75
70–90	500 ≤ TDS ≤ 800	7 ≤ DO ≤ 9	13.75 ≤ BOD ₅ ≤ 21.2	13.75 ≤ NO ₃ -N ≤ 21.2
50–70	800 ≤ TDS ≤ 1000	5 ≤ DO ≤ 7	21.2 ≤ BOD ₅ ≤ 28.8	21.2 ≤ NO ₃ -N ≤ 28.8
25–50	1000 ≤ TDS < 1200	3 ≤ DO ≤ 5	28.8 ≤ BOD ₅ < 30	28.8 ≤ NO ₃ -N < 30
0–25	1200 ≤ TDS	DO < 3	30 ≤ BOD ₅	30 ≤ NO ₃ -N

Table 4. The developed weight factor for each water quality parameter in the IWQI.

Parameter	Rate (1–5)	Temporary Weight (0–1)	Final Weights
TDS (mg/L)	2	1.0	0.353
BOD ₅ (mg/L)	3	0.7	0.235
NO ₃ -N (mg/L)	3	0.7	0.235
DO (mg/L)	4	0.5	0.177
Total ($\sum w_i$)		2.8	1.00

3. Results and Discussion

3.1. Water Quality Simulation

The one-dimensional HD and WQ model were applied to simulate water quantity and WQ along the El-Salam Canal. In the calibration process (September 2012–August 2013), the M value which achieved the lowest error in discharge was 40 m^{1/3}/s. The RMSE and NOF values between the observed and simulated discharge were 1.66 m³/s and 0.02 for Pump Station No. 1 and 1.10 m³/s and 0.02 for Pump Station No. 2, respectively. Moreover, for the validation period (September 2013–August 2014), the RMSE values were 2.94 m³/s and 1.39 m³/s, with NOF values of 0.04 and 0.02 for the discharges of Pump Stations No. 1 and No. 2, respectively. The model calibration and validation for the discharges of the pump stations (with the low NOF values) evidenced the ability of the model to correctly simulate the processes in the canal and therefore, it seemed reasonable to employ the simulated discharge to investigate the impact of various management scenarios. The TDS, DO, BOD₅ and NO₃-N results from the calibration model (September 2013–August 2014) were calculated at two different locations: (1) after mixing with the Faraskor drain at 1.9 km; and (2) before the downstream of the canal at 86 km from the intake (Table 5). Overall, the model provided reasonable agreement with observed WQ data along the El-Salam Canal, though TDS at 1.9 km and DO at 86 km showed relatively high NOF values.

Table 5. The RMSE (mg/L) and NOF for water quality parameters of the El-Salam Canal.

WQ Parameter	At 1.9 km		At 86 km	
	RMSE	NOF	RMSE	NOF
Temperature	0.02	0.001	0.09	0.004
TDS	79.8	0.16	31.7	0.03
DO	0.56	0.07	1.77	0.39
BOD ₅	3.00	0.16	3.90	0.14
NO ₃ -N	0.88	0.09	0.76	0.06

The simulated WQ parameters for the period from September 2013 to August 2014 along the El-Salam Canal at four selected stations (S.) from the intake and after mixing with the agricultural drains, were compared with the Egyptian standards for water reuse in irrigation as shown in Figure 3a–d. Stations S. No. 1, S. No. 2, S. No. 3, and S. No. 4 were located at 0.0 km, 1.85 km, 18.5 km and 55.0 km, respectively. The TDS values along the El-Salam Canal varied from 292 mg/L to 1300 mg/L, from September 2013 to August 2014 (Figure 3a). The TDS concentration violated the Egyptian

standards (TDS < 1200 mg/L) for direct reuse of water for irrigation after the connection with the Bahr Hadous drain (S. No. 4). The highest TDS values from 1085 mg/L to 1300 mg/L were registered at S. No. 4 in the summer season, particularly from May to August, due possibly to highly polluted water discharged from the Bahr Hadous drain, where the TDS values of ADW were at a maximum of 2420 mg/L. Seepage of salt water from the surrounding region is likely another source that increase TDS in the El-Salam Canal. Seasonal variation in water quantity as well as the fluctuation in mixing ratios of Nile River water with ADW may also have an adverse impact on the level of TDS in the water of the canal. These notions are indeed in agreement with those reported by El Gammal [37]. Moreover, Hafez et al. [38] detected high TDS fluctuation and variation in the canal after mixing with the Bahr Hadous drain (34–85%) and the El-Serw drain (31–57%). El-Sheekh et al. [39] found that electrical conductivity (EC), TDS, salinity, and chloride were maximized at levels of 5616 $\mu\text{S}/\text{cm}$, 10,636 mg/L, 10.9%, and 5.1 g/L, respectively, at the Bahr Hadous drain, and decreased to minimum values of 1933 $\mu\text{S}/\text{cm}$ for EC, 2104 mg/L for TDS, 2.6% for salinity, and 1.2 g/L for chlorides at the intake point (i.e., the Nile River). The authors claimed that variations in TDS might have been due to the effect of the ADW being rich in salts at the Bahr Hadous drain.

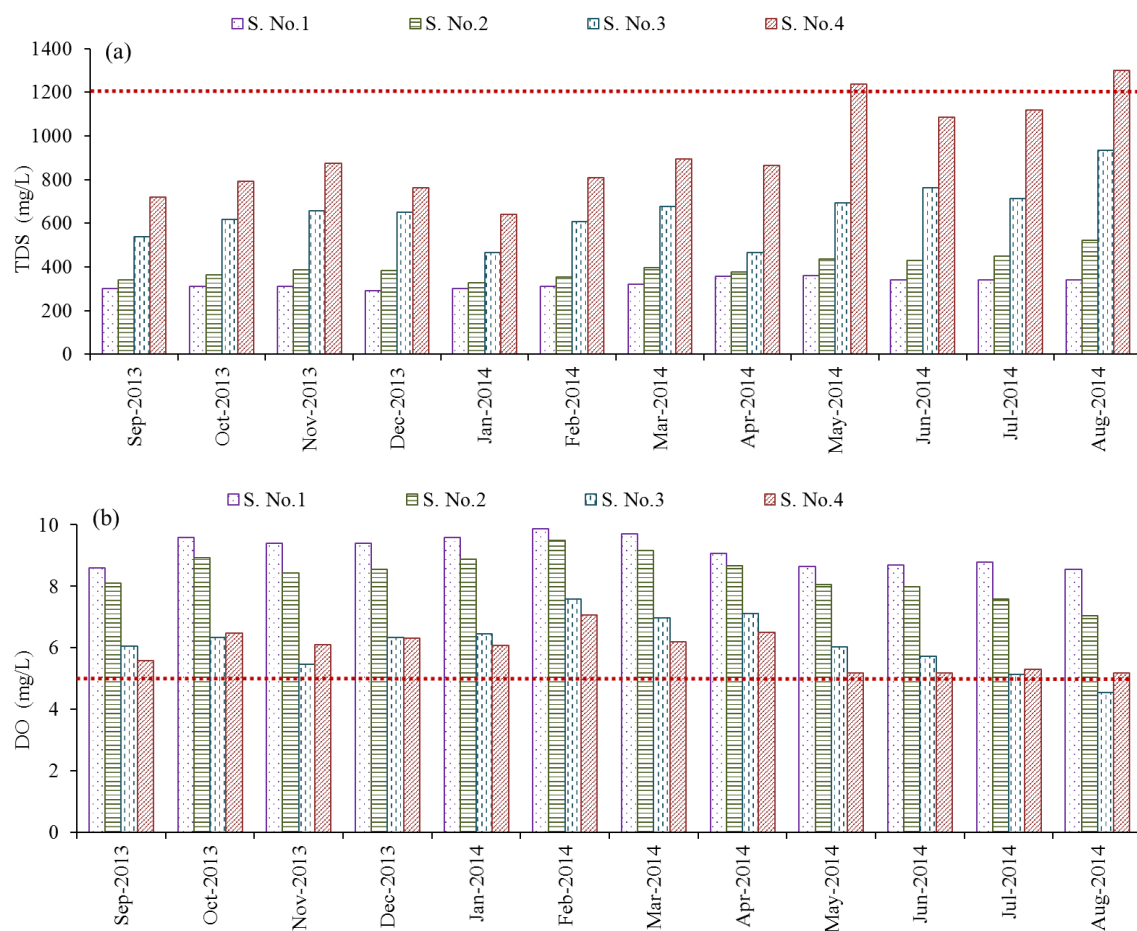


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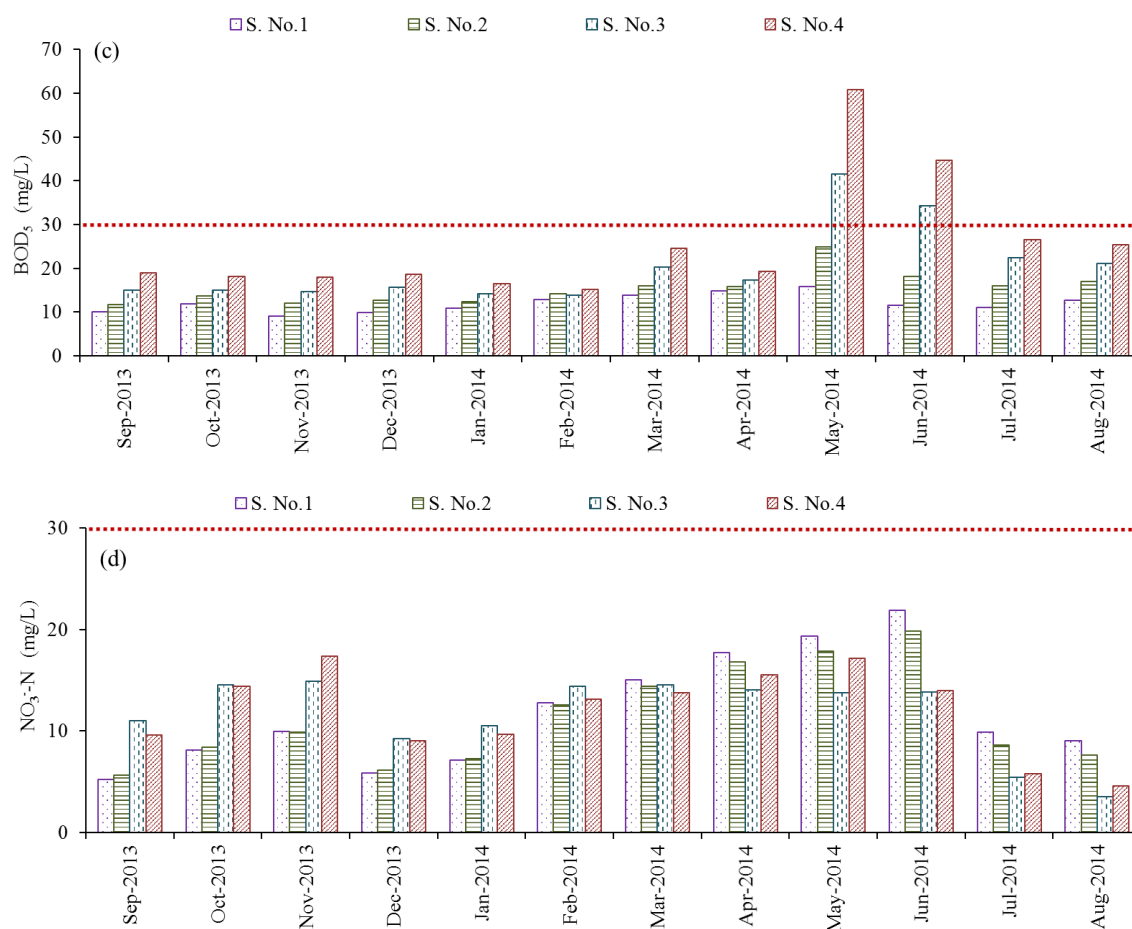


Figure 3. Simulated water quality parameters at four stations (S. No.1 to S. No.4) for the period from September 2013 to August 2014 in terms of: (a) TDS, (b) DO, (c) BOD₅, (d) NO₃-N.

The simulated DO values varied from 4.5 mg/L to 9.9 mg/L during the period from September 2013 to August 2014 (Figure 3b). The DO concentration along the El-Salam Canal complied with the Egyptian standards (DO > 5 mg/L) for direct reuse in irrigation except at the El-Serw drain (S. No. 3) in August. Nevertheless, a significant decrease in the DO levels was observed at the El-Serw (S. No. 3) and Bahr Hadous drains (S. No. 4), as shown in Figure 3b. The DO concentration amounted to 4.5 mg/L in the El-Salam Canal at the connection with the El-Serw drain and 5.1 mg/L at the connection with the Bahr Hadous drain in August. This result is consistent with the results of Hafez et al. [38], who indicated that the DO levels of Nile River water decreased sharply after the mixing point with the El-Serw and Bahr Hadous drains. One of the plausible reasons for low DO is due to the discharge of domestic wastewater into these drainage canals causing a depletion of oxygen. Indeed, El Gammal [37] reported that the El-Serw and Bahr Hadous drains receives a significant amount of untreated wastewater that causes severe pollution of the ADW, which subsequently adversely affects the WQ of the El-Salam Canal. Othman et al. [40] also found that the DO concentration decreased at the connection sites of the drains with the canal. Furthermore, El-Sheekh et al. [39] found that the maximum annual mean concentration of DO was 6.0 mg/L at the Nile River site, whereas DO was 2.2 mg/L at the Bahr Hadous drain site connection with the canal. However, the relatively higher DO at the Bahr Hadous in this study (5.1 mg/L in 2014) compared to reported value by El-Sheekh et al. [39] (2.2 mg/L in 2010) is most likely associated with construction of wastewater treatment plants along the drainage systems, which mitigate the effect from pollution source.

As shown in Figure 3c, the simulated data for BOD₅ gradually increased from the intake point to the connection sites with the drainage canals. For example, the simulated BOD₅ along the El-Salam

Canal exceeded the limit of 30 mg/L for direct reuse of the water in irrigation in May and June 2014; the BOD₅ values further increased downstream at the El-Serw and Bahr Hadous drains. The BOD₅ values were 42 mg/L in May and 34 mg/L in June after mixing with El-Serw drain water (S. No. 3). Moreover, the highest value of BOD₅ was 61 mg/L in May and 45 mg/L in June after mixing with Bahr Hadous drain water (S. No. 4). BOD₅ of the El-Serw drain (21–51 mg/L) and Bahr Hadous drain (30–136 mg/L) were generally higher than that for the main channel of the Nile River (6–34 mg/L). Hafez et al. [38] noticed that the BOD₅ levels in the canal were high due to the supply of ADW to the canal with the BOD₅ values being recorded as 75 mg/L in June 2004 and 33 mg/L in November 2004 after mixing with El-Serw drain water. The highest value of BOD₅ was 112 mg/L in June 2004 (Bahr Hadous drain), and the lowest value was 43 mg/L in December 2004 after mixing with El-Serw drain water. Therefore, the BOD₅ values of the canal can be affected by the quantity and quality of discharges, as well as seasonal and spatial effects.

In contrast to BOD₅, the results for NO₃-N indicated relatively lower values, which varied from 3.6 to 22 mg/L and complied with the WQ standards for reuse as shown in Figure 3d. The highest values were observed from April to June 2014 and were most likely due to elevation of the temperature up to 33 °C, which positively affected the nitrification process. These results demonstrate that the canal environment deteriorated after mixing with water from the El-Serw and Bahr Hadous drains. Othman et al. [40] reported that the NO₃-N values in the El-Salam Canal ranged from 0.01–5.47 mg/L and from 0.07–1.49 mg/L, respectively.

All statistical analyses were performed by Minitab software with a significance level of $p < 0.05$. The Durbin–Watson test was used to test the WQ data for serial autocorrelation. For the majority of parameters, autocorrelation was insignificant (i.e., $D > D_u$). The results indicated that the WQ parameters were independent over time, but depended on the agriculture seasons. As a consequence of the minimum degree of serial autocorrelation, its effects were neglected in the following statistical analyses.

The WQ data was divided into two groups (winter and summer seasons) according to the irrigation periods, for assessing the seasonal differences of variability for the four stations. Table 6 displays the results of the homogeneity of variance test, which clearly demonstrated that the ratio of standard deviations or variances is not statistically significant ($p > 0.05$), except for BOD₅ at S. Nos. 2–4 ($p < 0.05$). These results were confirmed with the simulated outputs in Figure 3, which illustrated the simulated BOD₅ along the El-Salam Canal exceeded the limit of 30 mg/L for direct reuse of the water in irrigation, with a significant difference especially in May and June 2014.

Table 6. Seasonal variances of the WQ parameters for the four stations.

Method	WQ Parameter	S. No.1	S. No.2	S. No.3	S. No.4
TDS					
Bonett's Test	P-Value	0.716	0.417	0.456	0.656
Levene's Test	P-Value	0.624	0.391	0.752	0.240
DO					
Bonett's Test	P-Value	0.884	0.229	0.369	0.337
Levene's Test	P-Value	0.240	0.312	0.323	0.287
BOD ₅					
Bonett's Test	P-Value	0.579	0.019	0.009	0.020
Levene's Test	P-Value	0.598	0.103	0.075	0.125
NO ₃ -N					
Bonett's Test	P-Value	0.194	0.243	0.373	0.425
Levene's Test	P-Value	0.140	0.195	0.507	0.461

However, the TDS parameter was the only one that had a significant difference of variances from one station to another ($p < 0.05$), as shown in Figure 4. The result indicated that the TDS values was the most effective parameter for the water quality along the canal, confirming the officials' concern that

the El-Salam Canal be constructed where agricultural drain water can be diluted with fresh water to maintain a TDS less than 1200 mg/L. Moreover, an ANOVA test was applied to illustrate the differences among the four stations for each parameter. Table 7 shows the results of the ANOVA test that clearly illustrated the significant differences ($p < 0.05$) for all parameters in the four stations except for nitrate levels ($p > 0.05$). The results indicated that most parameters varied in their spatial variation along the canal after the connection with the drains.

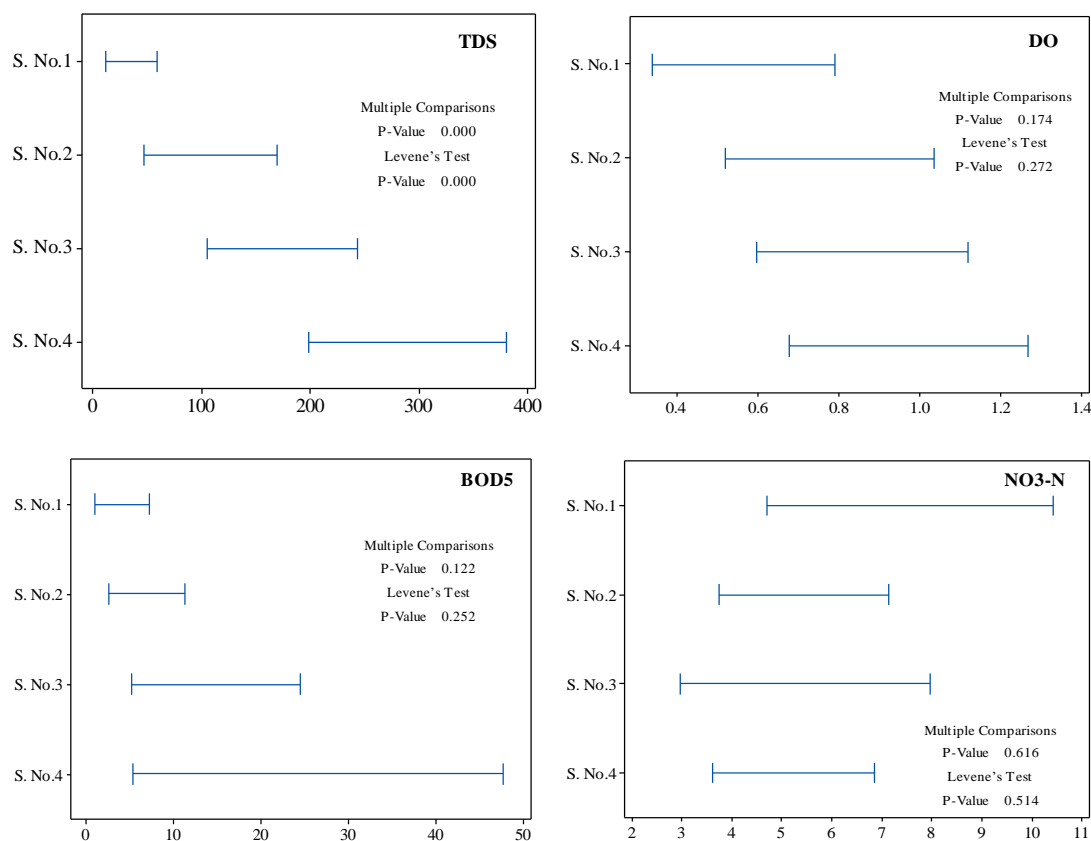


Figure 4. The variability of water quality parameters in terms of: TDS, DO, BOD₅, and NO₃-N at the four stations (S. No.1 to S. No.4) for the period from September 2013 to August 2014.

3.2. Water Quality Indices Performance

The WQ indices (FWRI and IWQI) were applied along the El-Salam Canal at the selected four stations over the period from September 2013 to August 2014. Figure 5a–d shows a comparison of the FWRI with the IWQI at the four stations during the period from September 2013 to August 2014. As shown for S. No. 1 (Figure 5a), the FWRI and IWQI values had similar trends and the WQ was likely acceptable for irrigation purposes, although the output classes were different. The FWRI values varied from 83.4 to 82.8, indicating that the WQ is in the 'G' category. In contrast, the WQ was classified into the 'VG' category for the IWQI values, which varied from 96.55 to 90.17.

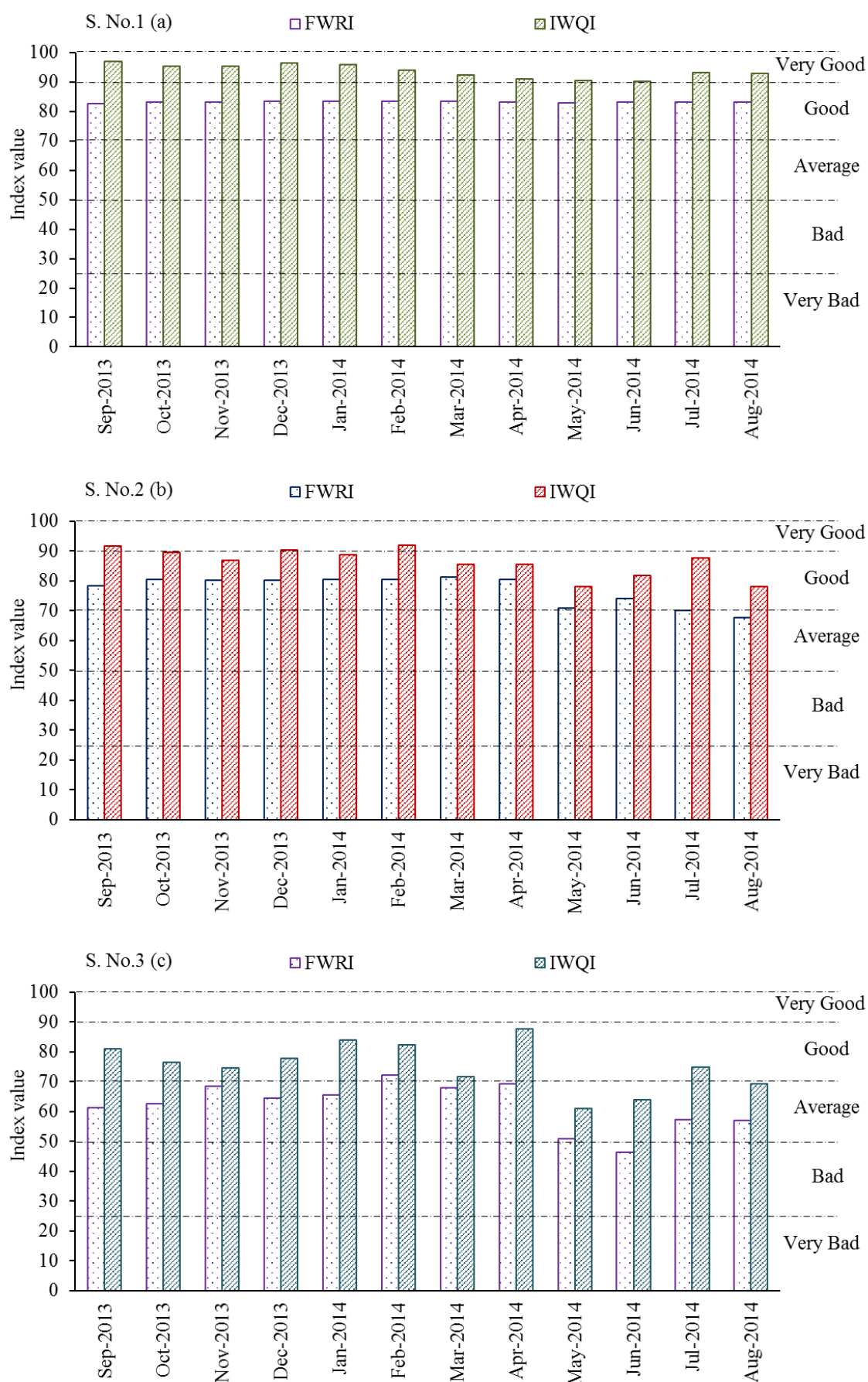


Figure 5. Cont.

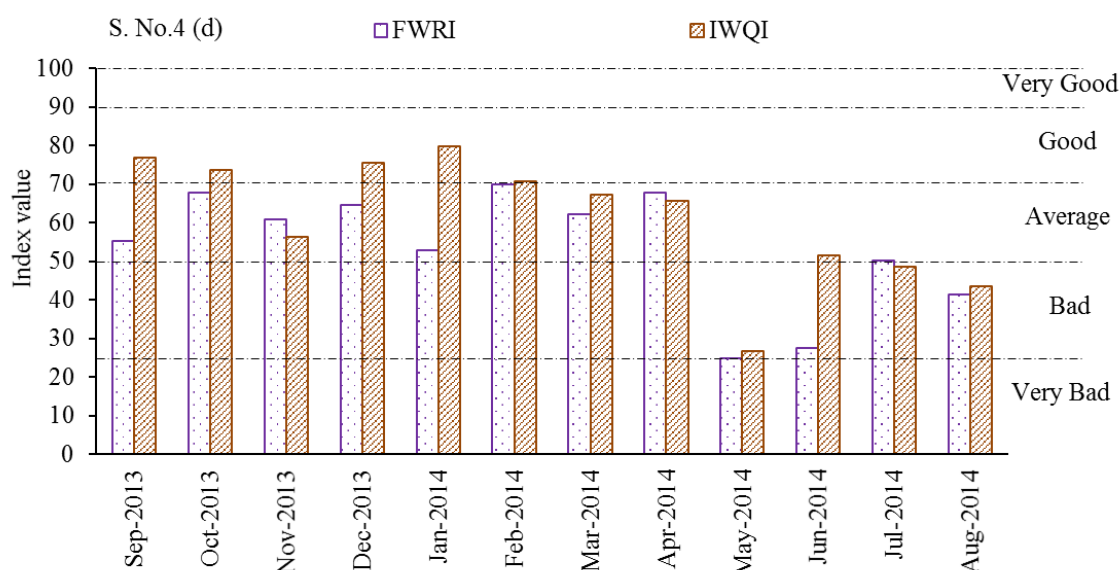


Figure 5. The FWRI versus IWQI at the selected stations over the period from September 2013 to August 2014, (a) S. No. 1, (b) S. No. 2, (c) S. No. 3, (d) S. No. 4.

Table 7. ANOVA results for the four stations.

Source	DF	SS	MS	F-Value	P-Level
TDS					
Factor	3	3,357,471	1,119,157	52.10	<0.05
Error	44	945,155	21,481		
Total	47	4,302,627			
DO					
Factor	3	151.47	50.4899	110.27	<0.05
Error	44	20.15	0.4579		
Total	47	171.62			
BOD ₅					
Factor	3	2469	822.84	9.73	<0.05
Error	44	3720	84.54		
Total	47	6188			
NO ₃ -N					
Factor	3	15.77	5.257	0.22	0.885
Error	44	1071.18	24.345		
Total	47	1086.95			

Moreover, after the connection with the Faraskor drain (S. No. 2), the WQ reduced slightly and ranged between the 'G' to 'A' categories for the FWRI and the 'VG' to 'G' categories for the IWQI (Figure 5b). The FWRI values varied from 81.2 to 67.7 and the IWQI values varied from 91.87 to 78.0. The WQ complied with standards for its safe reuse in agriculture. The results also showed that the annual discharge of drainage water from the Faraskor (8 m³/s) had little adverse effect on the WQ of the El-Salam Canal. However, the FWRI and IWQI values after the connection with the El-Serw drain (S. No. 3) dropped significantly, probably due to the discharge of 31.1 m³/s from the El-Serw drain as shown in Figure 5c. The FWRI values ranged from 'A' to 'B' and varied from 72.1 to 46.3. The IWQI values decreased slightly; however, they ranged between 'G' and 'A' and varied from 87.7 to 61.1.

The results in Figure 5d show the FWRI versus the IWQI values after mixing with the Bahr Hadous drain (39 m³/s) (S. No. 4); the WQ deteriorated and resulted in a 'VB' classification. The FWRI values varied from 70.0 to 24.8 and were classified as ranging between 'A' and 'VB'. In contrast, the

IWQI values ranged from 'G' to 'B' and varied from 79.7 to 26.7. Collectively, as illustrated in Figure 5, the outputs of the FWRI were sensitive to variations in the WQ parameters compared with the IWQI.

Furthermore, the Z-test of the means was applied to detect whether the differences between the mean FWRI and IWQI outputs for the four stations were significant. Table 8 displays the statistical properties of the FWRI and IWQI values for the four stations, where the calculated Z (Z_{cal}) values were associated with the means and between Z-critical values (± 1.96 for a 5% significance level). The Z_{cal} values showed that the mean values between the FWRI and IWQI were significantly different in the three stations with the exception of S. No. 4 ($Z_{cal} > 1.96$). As a consequence, the aforementioned analyses consistently indicated that the FWRI and IWQI outputs have different statistical properties and the FWRI values do not preserve the basic statistical properties of the IWQI values.

Table 8. Statistical properties of FWRI and IWQI values at the four stations.

Stations	S. No.1		S. No.2		S. No.3		S. No.4	
Index	FWRI	IWQI	FWRI	IWQI	FWRI	IWQI	FWRI	IWQI
Count	12	12	12	12	12	12	12	12
Mean	83.21	93.62	77.07	86.36	61.92	75.38	53.82	61.39
Median	83.25	93.65	80.1	87.36	63.45	75.69	58.2	66.58
Minimum	82.8	89.17	67.7	78	46.3	61.06	24.8	26.65
Maximum	83.4	96.99	81.2	91.87	72.1	87.65	70	79.7
Range	0.6	7.82	13.5	13.87	25.8	26.59	45.2	53.05
Standard deviation	0.19	2.51	4.96	4.78	7.79	7.97	15.34	16.17
Coeff. of variation	0.23	2.69	6.44	5.54	12.58	10.58	28.51	26.33
Variance	0.037	6.32	24.64	22.88	60.69	63.59	235.43	261.36
Skewness	−0.78	−0.38	−1.01	−0.71	−0.78	−0.37	−0.98	−0.9
kurtosis	0.03	−0.91	−0.78	−0.45	−0.08	−0.44	−0.14	0.2
Correlation Coeff.	−0.037		0.711		0.787		0.765	
$ Z_{cal} < 1.96$	−14.3		−4.67		−4.18		−1.18	

In order to validate which of the two indices provide the best fit for the real situation of the WQ along the canal, a chi-square test (X^2) was applied comparing the FWRI and IWQI with the official DRI WQ data, which is illustrated in Table 9. The X^2 test investigates which index is best fitted to the observed WQ data. Table 10 shows the results of the X^2 test between the FWRI and IWQI outputs with the official classification for the four stations, where all the X^2 values are lower than the critical chi-square value (19.675) with $DF = 11$. Based on the results, the FWRI and IWQI values can both reasonably explain the current situation. However, the X^2 values for the FWRI were always larger than the IWQI one, which indicated that the FWRI was more relevant to the official classification than the IWQI.

This could be attributed to the method of index calculation, e.g., how each parameter is compared with the standard value in the calculations [41]. Moreover, the inference rules used for the FWRI calculation not only deal with numerical data, but also apply the expert's knowledge and experience [18]. Based on the findings in this study, the FWRI proved its capability and accuracy in the assessment of the ADW for reuse in irrigation compared with those obtained from a simulation model of the canal, and thus it can be applied as a comprehensive approach for the assessment of WQ reuse for irrigation purposes. These results confirm those of Ocampo-Duque et al. [24], Lermontov et al. [41] and Gharibi et al. [18], who studied the development of water quality indices based on fuzzy logic. They reported that this index seems to produce accurate and reliable outcomes, and can therefore be used as an alternative tool for effective water quality assessment.

Table 9. Comparison of the official current situation and the indices' values at the four stations over the period from September 2013 to August 2014.

	Stations	Sep-2013	Oct-2013	Nov-2013	Dec-2013	Jan-2014	Feb-2014	Mar-2014	Apr-2014	May-2014	Jun-2014	Jul-2014	Aug-2014
S. No.1	Current situation	VG	VG	VG	VG	VG	VG	G	G	G	G	G	G
	FWRI	G	G	G	G	G	G	G	G	G	G	G	G
	IWQI	VG	VG	VG	VG	VG	VG	VG	VG	VG	VG	VG	VG
S. No.2	Current situation	G	G	G	G	G	G	G	G	G	G	G	G
	FWRI	G	G	G	G	G	G	G	G	G	G	G	A
	IWQI	VG	G	G	VG	G	VG	G	G	G	G	G	G
S. No.3	Current situation	A	A	A	A	A	A	B	A	B	B	A	A
	FWRI	A	A	A	A	A	G	A	A	A	B	A	A
	IWQI	G	G	G	G	G	G	G	G	A	A	G	A
S. No.4	Current situation	A	A	A	A	A	A	B	B	VB	B	B	B
	FWRI	A	A	A	A	A	G	A	A	VB	B	A	B
	IWQI	G	G	A	G	G	G	A	A	B	A	B	B

Table 10. The χ^2 -test comparing the FWRI and IWQI values with the official classification at the four stations over the period from September 2013 to August 2014.

Stations	S. No.1		S. No.2		S. No.3		S. No.4	
Index	FWRI	IWQI	FWRI	IWQI	FWRI	IWQI	FWRI	IWQI
χ^2	3.764	2.278	1.855	1.470	8.597	6.238	17.98	10.305

4. Conclusions

This study used the IWQI and a WQ index based on a fuzzy logic approach (FWRI) to assess the ADW quality according to the results of a hydrodynamic and one-dimensional WQ simulation model. The indices were applied to classify the ADW quality along the largest project in Egypt (El-Salam Canal) from September 2013 to August 2014. The HD module of the El-Salam Canal using two years of data was calibrated from September 2012 to August 2013 and verified from September 2013 to August 2014. This was followed by calibration of the WQ module using data from September 2013 to August 2014. The results provide evidence of the reliability of the model in simulating the water quantity and WQ along the canal with the lowest RMSE values among the observed and simulated data. The results illustrated that the WQ deteriorated towards the downstream of the canal due to the polluted water discharged from the El-Serw and Bahr Hadous drains. An ANOVA test was applied to indicate the differences for each WQ parameter at the selected stations along the canal. The test results provided evidence that the WQ differed in their temporal variation along the canal after connection with the drains ($p < 0.05$). The comparisons between the WQ indices outputs (FWRI and IWQI) were performed at the selected four stations over the simulation period. The results demonstrated that the outputs were sensitive to variations in the WQ parameters. Additionally, the results of the Z-test illustrated that the FWRI values do not preserve the basic statistical properties of the IWQI values. For the validation of the indices, a chi-square test (χ^2) was applied comparing the FWRI and IWQI with the official DRI WQ data. The results indicated that the FWRI and IWQI values can both reasonably explain the current situation. However, the χ^2 values for FWRI were always larger than the IWQI values, which demonstrated that the FWRI was more relevant to the official classification than the IWQI. Accordingly, the FWRI proved its capability and accuracy in the assessment of ADW quality and pollution compared with those obtained from the simulation model of the canal, potentially enabling it to be applied as a comprehensive approach for the assessment of WQ for reuse in irrigation.

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Abbreviations

A	Average
AD	Advection-dispersion module
ADW	Agricultural drainage water
B	Bad
BCM	Billion cubic metres
BOD ₅	Biochemical oxygen demand
DHI	Danish Hydraulic Institute
DO	Dissolved oxygen

DRI	Drainage Research Institute
EC	Electrical conductivity
ECO-Lab	Water quality module
FAO	Food and Agriculture Organization
FIS	Fuzzy inference system
FWRI	Fuzzy logic water reuse index
G	Good
HD	Hydrodynamic module
IWQI	Irrigation water quality index
M	Manning's roughness coefficient
MWRI	Ministry of Water Resources and Irrigation
NO ₃ -N	Nitrates
NOF	Normalized objective function
RMSE	Root mean square error
S.	Stations
SAR	Sodium adsorption ratio
TDS	Total dissolved solids
VB	Very Bad
VG	Very Good
WQ	Water quality
WQI	Water quality index

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