

Article

Axial Compression Performance of Post-Fire Concrete Columns Strengthened Using Thin-Walled Steel Tubes

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Abstract: For concrete columns damaged by fire, a strengthen technique using thin-walled steel tubes is proposed. To investigate the axial compression capacity of post-fire concrete columns strengthened using thin-walled steel tubes, considering tube thickness, the strengthening method, and section geometry, 12 specimens were fabricated, of which two were control columns and 10 were exposed to fire in accordance with the ISO834 temperature curve. Subsequently, eight specimens were strengthened. Axial compression tests were conducted to provide a better understanding of the strengthening technique. The experiments indicated that: (1) Different failure modes were observed for different cross section geometry—local buckling for square sections, and shear failure for circular sections. (2) The stiffness, axial strength, and ultimate deformation capacity of the enhanced columns may be rehabilitated and even better than the undamaged ones. (3) Two enhancement methods were compared. The steel tubes act as restraints, are merely subjected to tension, and provide strong restraint to the core concrete. The tube carrying load together with the concrete columns are mainly subject to compression, and likely to buckle with longitudinal strain. Finally, axial compressive equations of post-fire reinforced concrete (RC) with thin-wall steel tubes, including both square sections and circle sections, were proposed on the mechanism of concrete filled steel tubes.

Keywords: thin-walled steel tube; strengthening; post-fire; RC column; axial compression

1. Introduction

When a concrete building is subjected to a fire disaster, the ultimate strength and stiffness of the reinforced concrete (RC) columns may be seriously degraded after exposure to fire. In many cases, the concrete elements cannot be fully replaced. To increase the strength of post-fire RC columns, the technique of wrapping thin-walled steel tubes is explored in this paper. At first, two steel half-tubes are placed outside the concrete column after removal of the damaged surface and the installation of the post-drilled rebars if required, then the half-tubes are welded together. Finally, free flow non-shrink grout is infilled in the gap to form a new combined structure. A similar method is widely used in the enhancement of damaged structures by steel tubes. Due to the restraint of the steel tube, the core concrete is placed under triaxial compressions, and the compressive strength is improved, resulting in an enhancement in the ultimate strength and deformation capacity of the member. The thin-walled steel tube for strengthening can be used as a formwork for new concrete to be infilled. Compare with other enhancement methods, it has the advantages of minimum space occupation, easy installation, less pollution, and good durability under high temperatures.

There are several theoretical and experimental studies that have introduced the structure performance wrapping tube enhanced RC column. Isleem and Wang et al. [1] built a numerical model to estimate the strength of un-reinforced and reinforced rectangular concrete columns confined with fibre-reinforced polymer wraps. The effects of aspect ratio and the size of the cross-section, effective rupture strain of the polymer, corner radius and internal hoop steel reinforcement were discussed. Janwaen and Barros [2] introduced an efficient confinement method on rectangular reinforced concrete (RC) columns under axial compression using the strips of carbon fiber reinforced polymer (CFRP) wet layup sheets together with a certain level of prestress. The influence of the cross-section aspect ratio of columns on the axial stress–strain response, the strain field in the CFRP and strength increase provided by the different adopted strengthening configurations was investigated. It may be difficult using FRP wet layup sheets to enhance post-fire column structural strength, because any surface of a column above 500 °C should be removed before enhancement and core concrete after a fire with the high temperatures of 250 °C to 500 °C can only be used with weaken strength.

In 1991, K. Yoshimura, K. Kikcuri, and M. Kuroki [3] published the test results of reinforced concrete columns of square sections enhanced with steel tubes of square hollow sections. Two L-shape steel elements were welded on the roughed surface of the damaged reinforced concrete column, then the gap was infilled by epoxy resin cement aggregate. The experiment data indicated that the ultimate bending strength of the short column was enhanced, and could be fully developed without brittle failure from the shear actions. This strengthening method has been verified to also be valid for the short columns damaged due to brittle shear failure. M. J. Nigel Priestley, Frieder Seible, Yan Xiao, and Ravindra Verma [4,5] used elliptical steel tubes to enhance circle and square concrete columns, and carried out experimental and theoretical analysis on the shear strength. The study demonstrated that the enhancement by the wrapping steel tube can significantly improve the shear strength and bending stiffness of the strengthened column. The work was related to the shear failure mode of an RC bridge designed and built before 1971. The models were applied to predict the shear strength of the structure enhanced by circular or elliptical jackets. The axial load level, strength of longitudinal reinforcement, and aspect ratio as well as section shape were investigated and analyzed in an experimental test program. As-built and retrofitted columns were compared. It was shown that all columns enhanced by circular or elliptical jackets have a large improvement on the shear strength.

In 2000, Hui Wu [6] proposed a method to use a combination of welded steel tubes and stiffeners to enhance square columns. The stiffeners were placed in the area where plastic hinges may occur at the end of the column. The test results verified that the stiffened steel jacketing can prevent brittle shear failure and significantly improve the deformation capacity. The efficiency of the partially stiffened rectilinear steel jacketing was verified, and the ductility of the strengthened elements were significantly improved.

Studies on thin-wall steel tube strengthened post-fire concrete structures, based on the numerical simulation of the temperature field model and mechanical properties, have been carried out [7,8]. Considering the effects from the steel tube, concrete reinforcement ratio and strength of the new added concrete, the strength and ductility of RC columns were investigated and discussed. The results indicated that the RC columns were well strengthened by the thin-walled steel tube, and the strength and stiffness of the strengthened specimens were obviously improved, with increased ductility.

In this work, the axial strength and ultimate deformation capacity of the post-fire RC columns after enhancement are tested and presented. The effect of steel tube thickness, the enhancement method and the cross-section are studied and investigated. The practical equations for the axial strength of post-fire RC columns strengthened by thin-wall steel tubes are proposed. Besides the practical calculation equations, the efficiency of axial compression equations and standards of concrete filled steel tube are compared and verified in detail.

2. Experimental Program

2.1. Specimen Details

A total of twelve RC specimens, designed following “Chinese Concrete Structure Design Code” [9], were fabricated and tested, including eight RC columns of square sections and four RC ones of circular sections.

In order to obtain the effect of the section geometry on the efficiency axial compression performance of post-fire RC columns enhanced by thin-walled steel tubes, the similar cross-section geometries of the specimens were adopted. The side length of the square column was 300 mm (the area was 90,000 mm²), and the diameter of the circular was 340 mm (an area of 90,792 mm²), and all the specimens were 1500 mm long.

The parameters of each specimen are given in Table 1. There are two unfired specimens, SC1 and C1, and two fired specimens without strengthening, SC2 and C2, as bench marks. The specimens SC3, SC4, SC5, SC6, S-C1, S-C2, C3, C4 were fabricated to investigate the effects on the failure mode, stiffness, axial compressive strength and ultimate deformation capacity. The variables of the effects include the thickness of the steel tube (2 mm, 3 mm), the strengthening methods (the steel tube only acting as a restraint, or both restraint and load bearing material), and the section geometry (including square, circular, or section change after strengthening).

Table 1. Design parameters of specimens.

Specimen	Initial Section Size (Side Length or Diameter)/mm	Duration in Fire/Min	Strengthening Tube Thickness/mm	Strengthening Mode	Completed Geometry After Strengthening (Side Length or Diameter)/mm
SC1 ¹	Square (300)	-	-	-	-
SC2 ¹	Square (300)	90	-	-	-
SC3 ¹	Square (300)	90	2	A	Square (300)
SC4 ¹	Square (300)	90	3	A	Square (300)
SC5 ¹	Square (300)	90	2	B	Square (300)
SC6 ¹	Square (300)	90	3	B	Square (300)
S-C1 ²	Square (300)	90	3	A	Circular (340)
S-C2 ²	Square (300)	90	3	B	Circular (340)
C1 ³	Circular (340)	-	-	-	-
C2 ³	Circular (340)	90	-	-	-
C3 ³	Circular (340)	90	3	A	Circular (340)
C4 ³	Circular (340)	90	3	B	Circular (340)

Notes: ¹ Specimen SC means a test specimen of square section; ² Specimen S-C means the test specimen of a circular cross-section developed from an initial square cross-section; ³ Specimen C means a circular specimen of cross-section.

The geometry dimensions and reinforcement of each test specimen are shown in Figure 1. After removing the damaged surface, the test specimens can be enhanced using the thin-walled steel tubes. Keeping the similar cross section geometry as undamaged concrete columns, the grout is infilled in the gap between the tube and column. Steel tube manufacturing is illustrated in Figure 2a,b. The cross-section geometry was changed from the original 300 × 300 mm square, and strengthened into a circular cross-section with 340 mm diameter after fire; the modified geometry is shown in Figure 2c. The strengthening method A was applied to test specimens SC3, SC4, S-C1, C3, where the thin-walled steel tube only acts as a restraint by cutting a circular strip of 15 mm wide at each end of the steel tube at a distance of 150 mm from the end, as shown in Figure 3a. Strengthening method B was applied in test specimens SC5, SC6, S-C2, C4, where the thin-walled steel tubes bear longitudinal loads in addition to acting as restraints wrapping the whole column.

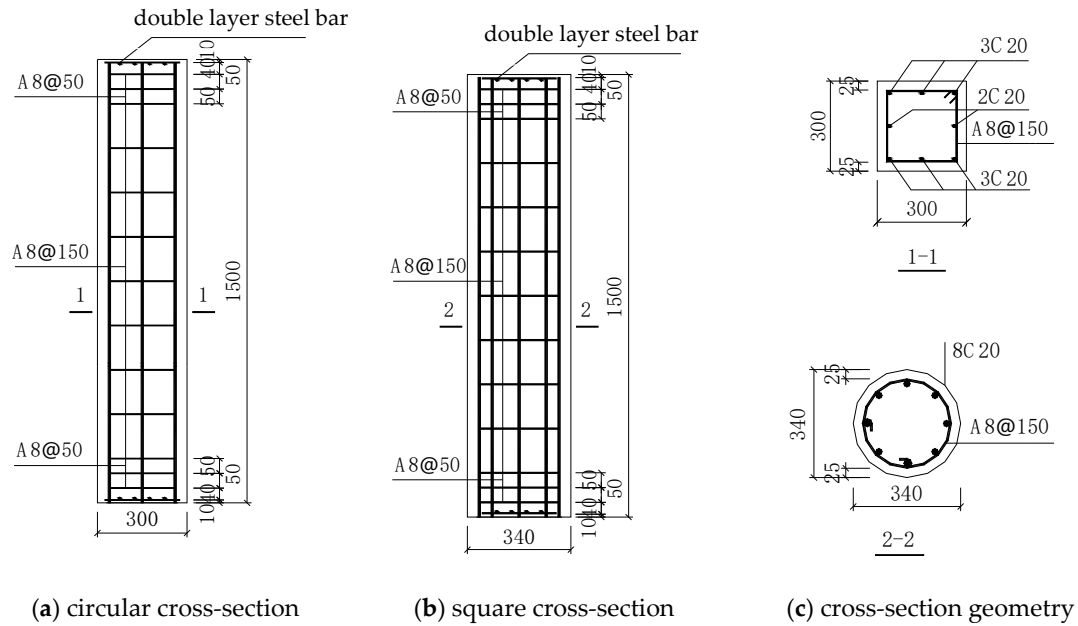


Figure 1. Specimens details (dimensions in mm).

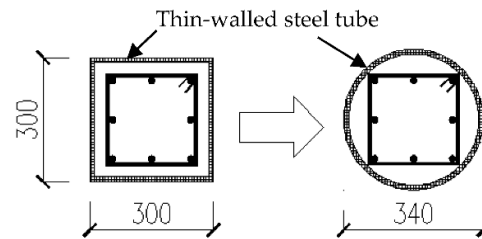


Figure 2. Section change after strengthening.

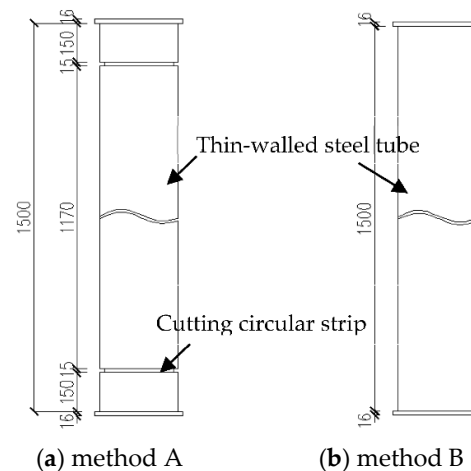


Figure 3. Strengthening methods (dimensions in mm).

2.2. Material Properties

The test specimens were poured in two batches sequentially. The first batch included C2, C3 and all square columns, and the second batch included C1 and C4. Three groups of standard test blocks of two batches were made, each side length was 150 mm, in order to check the cube compression strength at 28 days, the day of the fire test and of the axial compression performance

test. The post-fire specimens were strengthened by C50 free flow non-shrink grout. A standard cubic mortar block of the side length 70.7 mm was made for testing compressive strength at 28 days (just before the axial compression test). All test blocks and test specimens were cured under the same conditions, and the compression test was carried out in a microcomputer-controlled electro-hydraulic servo universal testing machine according to the Standard Test Method for Mechanical Properties on Ordinary Concrete (GB/T50081-2016) [10]. The test results are shown in Table 2.

Table 2. Parameters of the Concrete Mix Proportion.

Group	Concrete Cube Compressive Strength (MPa)			Grout Cubic Compressive Strength (MPa)
	28d	Fire Test	Axial Strength Test	
No. 1	34.6	43.1	44.3	50.4
No. 2	39.5	42.0	47.8	50.4

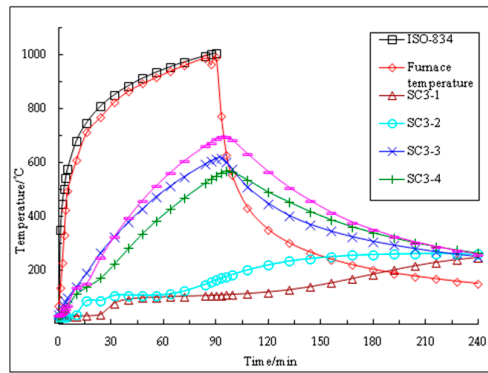
According to the “Metallic materials—Tensile testing at ambient temperature” (GB/T 228-2002) [11], the material properties of the steel were tested. All the longitudinal reinforcements were made of HRB400 steel by 20 mm diameter. The stirrups were made of HPB300 steel by 8 mm diameter. The steel plate for enhancement was made of Q235B steel. The tensile test specimen of steel plate was fabricated and formed in the factory. The test results on the mechanical properties of the steel reinforcement and steel plates are shown in Table 3. The elastic modulus of steel is 203GPa, and Poisson’s ratio is 0.3.

Table 3. Mechanical properties of reinforcements and steel plates.

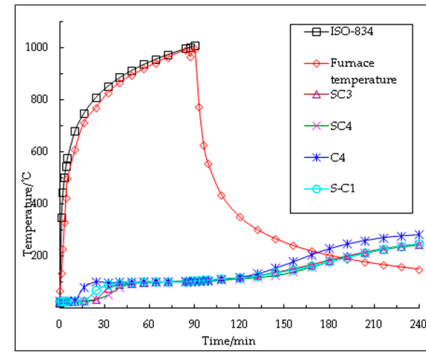
Sample	Grade	Diameter/Thickness (mm)	Yield Strength (MPa)	Ultimate Strength (MPa)
Steel reinforcement	HPB300	8	358.4	456.2
	HRB400	20	446.7	609.0
Steel Plate	Q235B	2	250.0	343.3
	Q235B	3	256.7	320.0

2.3. Fire Test

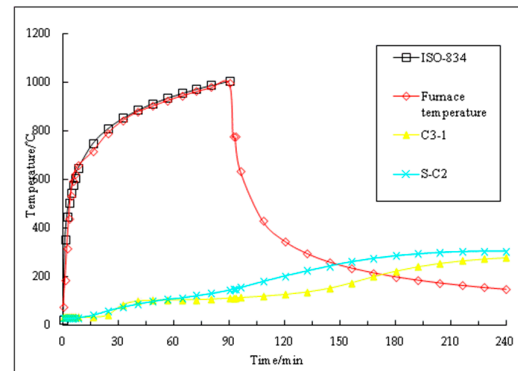
The fire test was carried out in the structural fire-resistance laboratory of the South China University of Technology. A large-scale horizontal component fire-resistant test furnace was used. The size of the furnace was $4 \times 3 \times 1.65$ m. The gas used was fuel, and 12 combustion nozzles were evenly arranged on both sides of the north and south. After ignition, the measurement was carried out by using the sheathed thermocouple. The gas temperatures at 10 different locations in the furnace were recorded and averaged as furnace temperature. During the fire test, the temperature control and data collection were all carried out by computer. The heating curve was adopted following the building fire standard heating curve specified in China’s “Fire-resistance tests elements of building construction (GB/T9978-2008)” [12]—the ISO834 standard heating curve. The temperature distribution inside the column during the heating and cooling process was measured using WRNK-101D3 armored thermocouples, see Figures 4 and 5a illustrating the test specimens after the fire test. In Figure 5b, after the fire test, the concrete surface color of all the test specimens changed from ash to brown, and irregular micro-cracks appeared on the surface. The corners of some square columns were partially spalling in Figure 5c, and a significant ring crack at the upper end was observed, as shown in Figure 5d.



(a) Specimens SC3

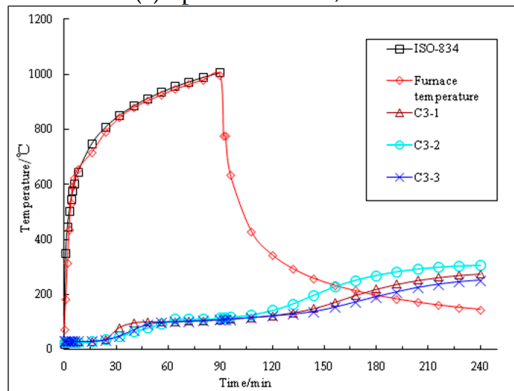


(b) Specimens SC4, C4 and S-C1

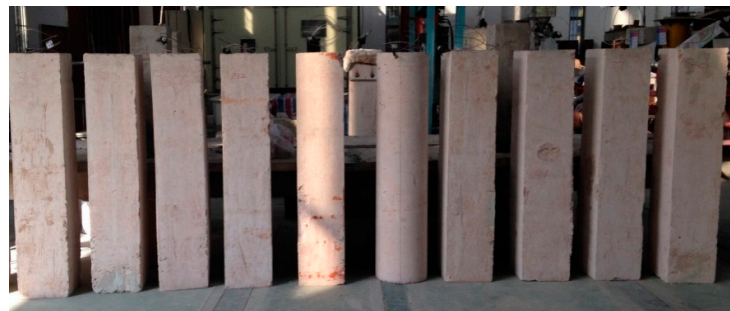


(c) Specimens SC5, SC6

(d) Specimens S-C2



(e) Specimens C3

Figure 4. Measured temperature–time curve of thermocouples.

(a) Specimens after fire test

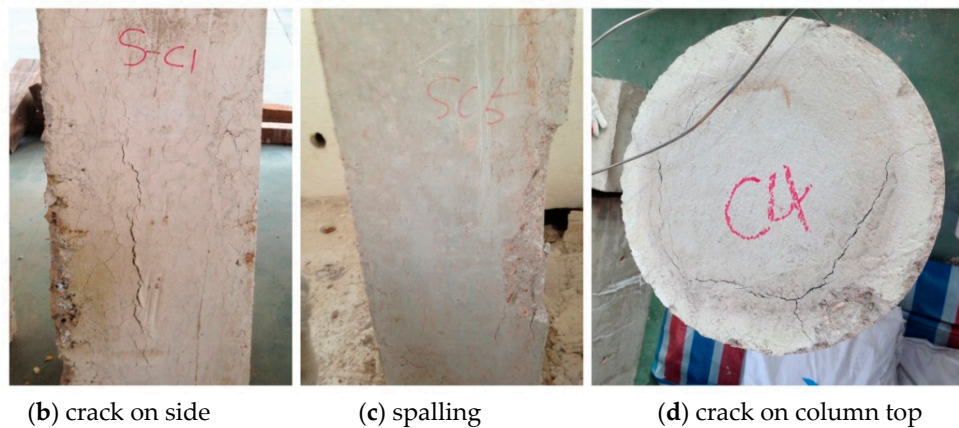


Figure 5. Specimens after fire test.

2.4. Strengthening Using Thin-Wall Tubes

The surface of the RC column was seriously damaged after exposure to fire. The first step of surface preparation was to remove a 30 mm thick damaged surface. Then the surface of the specimen was cleaned and saturated, with no treatments on the reinforcing bars. Finally, the specimen was wrapped by the steel tube, the gap was filled by C50 free flow non-shrink grout, to ensure that all parts achieve a good combination. Due to the narrow gap between the tube and concrete, to prevent a broken column, the grout was not infilled too much at signal pouring. The external wall of the steel tube was tapped with a hammer to check whether the gap was infilled or not. The grout was filled slightly above the upper end of the steel tube. The surface was levelled after curing, then the epoxy resin glue was evenly applied. The column was aligned with the geometric center of the upper end plate section. After the end plate and concrete surface were tightly combined, the test specimens were inverted, and the steel tube and the end plate were welded together. In the strengthening mode A, a circular strip of 15 mm wide was cut at 150 mm distance from each end of the tube. The strengthening procedure of the specimen is shown in Figure 6.



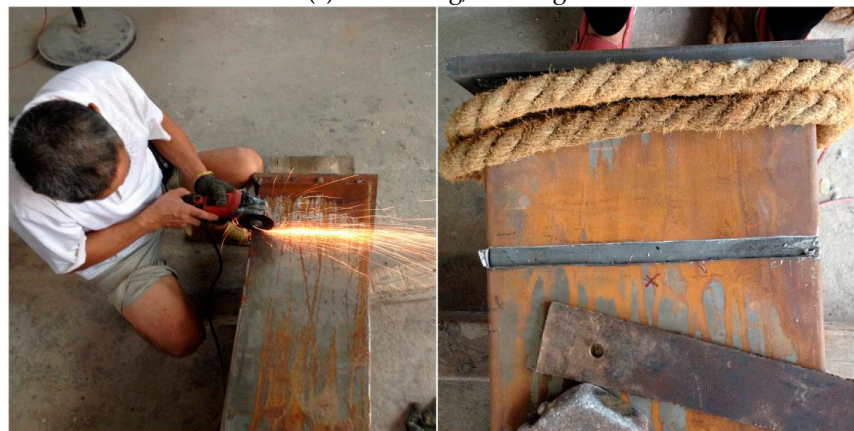
(a) removing the damaged surface, cleaning and saturating the specimens



(b) wrapping steel tube, grouting



(c) Smoothing, welding



(d) Cutting circular strip

Figure 6. The strengthening procedure.

2.5. Axial Compression Test

The test was carried out by a 10,000 kN electro-hydraulic servo compression shear test machine. The loading test was divided into two stages: Preloading and formal loading. To obtain the load-displacement curve, including the de-loading process accurately, the displacement control was adopted in the late stage of the loading test.

The preloading was subsequently divided into three levels. In each level, 5% of the estimated ultimate load was added. In formal loading, the load was carried out by increasing the load until 80% of the estimated ultimate load. The loading increase at each stage was 10% of the estimated ultimate load. When loading reached 80% of the estimated ultimate load, the test was carried by

increasing the displacement, the load increase per stage was changed to 5% of the estimated ultimate load, and the loading rate was 0.5 mm/min. Each load stage was maintained for at least 3 min after the previous loading, until the loading drops reached 60% of the measured limit load, then loading was stopped. The loading device is shown in Figure 7, and the lower steel plate is placed above the ball joint and can be rotated during the loading process. The hydraulic cylinder was located under the testing machine. The upper steel plate remains stationary during the loading process, and the lower steel plate moves upward.



Figure 7. Loading process.

The axial and transversal deformation were measured by using displacement gauge. The strain of the steel tube and the steel bar were measured by strain gauges. The force, displacement and strain signals during all loading were recorded by a DH3816 data acquisition instrument.

The arrangement of the displacement meters is shown in Figure 8.

A electrical strain gauge was used to measure the strain of the steel bar. The longitudinal rebars were measured by 3×5 mm electrical strain gauges, and 2×3 mm resistance strain gauges for stirrups. Except for SC1 and C1, on which strain gauges were pre-installed, most specimens to be subject to fire had strain gauges installed after fire. The strain gauges were uniformly arranged on the stirrups of the middle section to monitor the lateral expansion during the axial compression. Four strain gauges were arranged on the unenhanced test specimens, and two strain gauges were arranged on the enhanced test specimens. The arrangement positions of the strain gauges for steel RC columns are shown in Figure 9a–d.

Steel tube strain was measured by 3×5 mm electrical strain gauges. Four pairs of longitudinal and transverse strain gauges were pasted in the middle section, and four transverse strain gauges were attached to the steel tubes near the upper and lower annular strips on the test specimens enhanced by method A. For the test specimens enhanced by method B, divided into the upper, middle and lower three measuring sections, four pairs of longitudinal and transverse strain gauges were respectively attached to the steel tube. The arrangement of the steel tube strain gauge is shown in Figure 10.

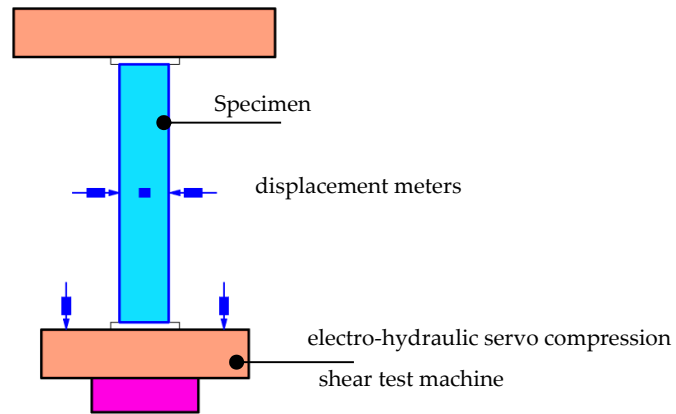


Figure 8. Arrangements of displacement meters for specimens.

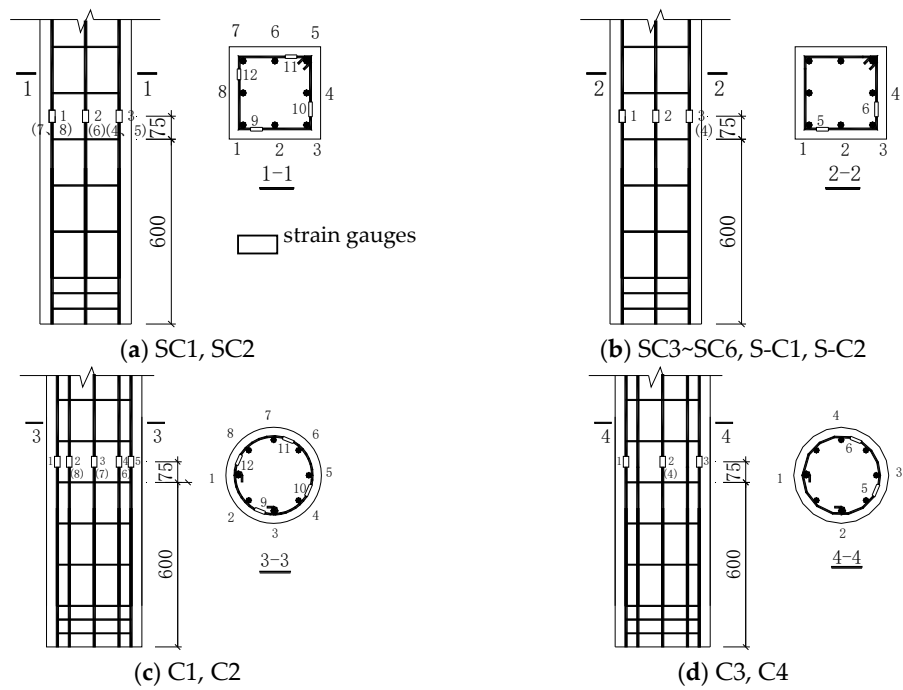


Figure 9. Arrangements of strain gauges for steel bars.

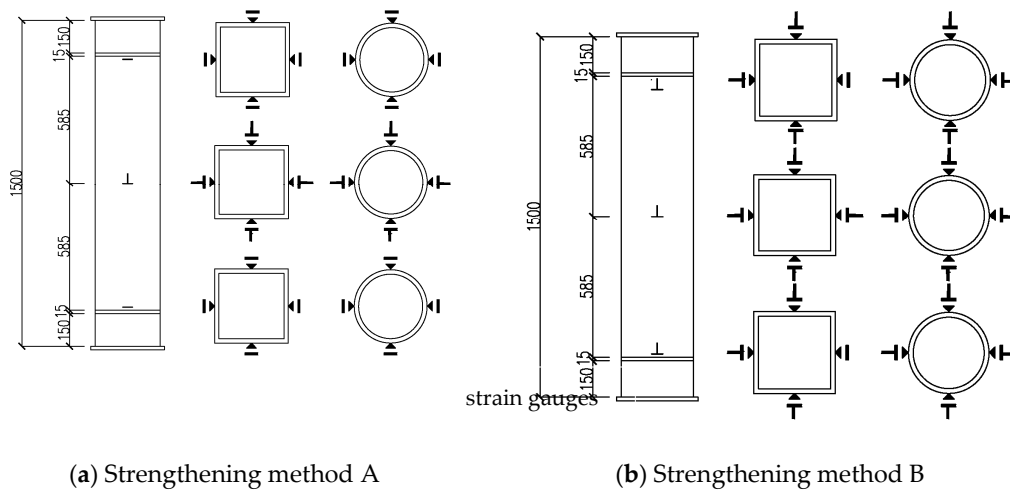


Figure 10. Arrangements of strain gauges for steel tubes.

3. Result Discussion

The failure modes of each test specimen are shown in Figure 11. It is observed that when the thin-walled circular steel tube was subjected to the ultimate load, a diagonal crack of about 50 degrees to horizon occurred in the concrete, and then the steel tube was inclined and failed by shear. While the thin-walled square steel tube RC column was buckling, the column reached the ultimate load. After steel tube was cut off, the concrete near the buckling was crushed, and the other parts were not damaged.

**Figure 11.** Failure mode of Specimens.

Table 4 gives the main characteristic points and test results of the post-fire RC columns strengthened using thin-walled steel tubes. All specimen N- Δ curves are given in Figure 12a,b.

Table 4. The principal characteristic points of the specimens.

Specimen	N_m (kN)	Δ_m (mm)	N_u (kN)	Δ_u (mm)	G_N (%)	G'_N (%)	G_u (%)	G'_u (%)
SC1	2883	8.53	2451	9.94	-	-	-	-
SC2	2249	10.70	1912	12.50	-	-	-	-
SC3	3500	11.68	2975	15.35	21.40	55.62	54.43	22.80
SC4	3601	13.00	3061	15.82	24.90	60.12	59.15	26.56
SC5	3651	11.85	3103	14.54	26.64	62.34	46.28	16.32
SC6	3762	10.25	3198	13.89	30.49	67.27	39.74	11.12
S-C1	4753	19.91	4040	41.82	64.86	111.34	320.72	234.56
S-C2	4684	15.98	3981	23.35	62.47	108.27	134.91	86.80
C1	3060	9.04	2601	10.75	-	-	-	-
C2	2409	10.58	2048	12.51	-	-	-	-
C3	5065	18.63	4305	29.78	65.52	110.25	177.02	138.05

C4	4803	21.05	4083	34.42	56.96	99.38	220.19	175.14
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Notes: N_m : The ultimate strength; Δ_m denotes the displacement at the ultimate strength; Δ_u : The ultimate displacement, when the loading is reduced to 85% of the ultimate load; N_u : The strength corresponding to the ultimate displacement; G_N : The ultimate strength difference between enhanced specimens and unfired ones; G'_N : The ultimate strength difference of the strengthened specimens compared with the unenhanced specimens after the fire; G_u : The ultimate displacement post-fire enhanced specimens compared with the values before fire; G'_u : the difference between the ultimate displacement of enhanced specimens and the unenhanced ones after the fire.

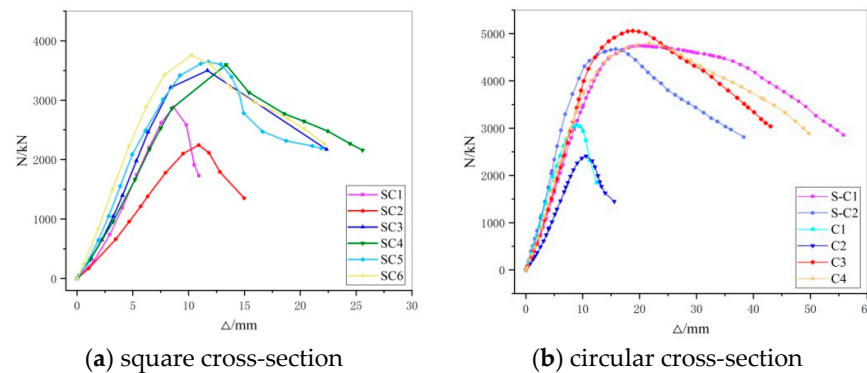


Figure 12. N - Δ of the specimens.

From the experimental observation and data, the main findings are summarized as follows: (1) The failure mode of the post-fire RC square column wrapped by a thin-walled square steel tube is the local bulking, and the column wrapped by the round steel tube failed by shear. (2) The test specimens appear to be “softened” after fire, with decreased axial stiffness and strength. The axial stiffness, axial compressive strength, and deformation capacity of the post-fire RC columns are significantly improved in strength by the thin-walled steel tubes. (3) Within the range of designed tube thickness (2 and 3 mm), the axial strength of the RC square column wrapped by the thin-walled square steel tube increases with the thickness of the tubes. (4) Different strengthening modes have no significant difference on the axial strength of the enhanced elements, it is more sensitive to axial deformation. Except the C3 specimen with a damaged welding, the rest of the specimens enhanced using mode A have larger axial deformations than the specimens enhanced by method B. (5) For the square column, the axial strength and the axial compression deformation of the columns whose cross-section were changed to a circle after strengthening are larger than those which remained as square cross-sections.

Eight strain gauges were installed at 1/2 height of the test specimen, and the average longitudinal strain and shear strain measured under the same load denote as ε_v and ε_h . Figure 13 compares the load–concrete strains of the unconfined test specimens; Figure 14 gives the load–steel line strain of the strengthened test specimens. Figure 15 shows a longitudinal load–strain diagram of test specimens; and the load–strain diagram of the stirrup is shown in Figure 16.

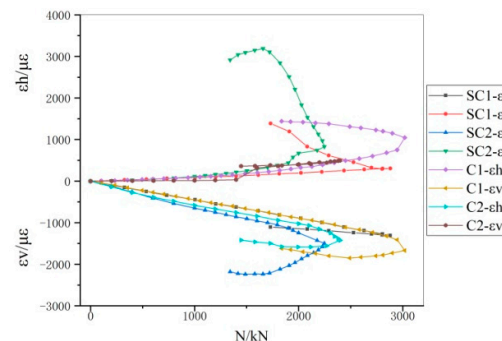
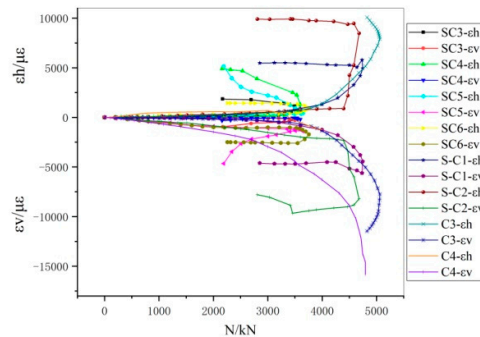
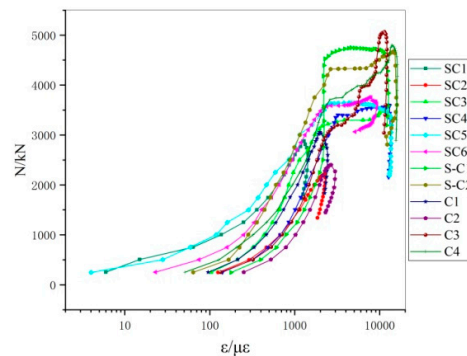
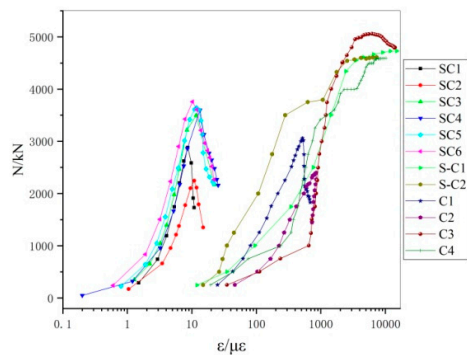


Figure 13. Diagram of load–concrete strain.**Figure 14.** Diagram of load–steel tube strain.**Figure 15.** Diagram of load–longitudinal bar strain.**Figure 16.** Diagram of load–stirrup strain.

It is observed from the Figure13–16 that (1) the behaviors of the steel tubes are greatly affected by the enhancement methods. The steel tubes using strengthening mode A, which is merely subjected to circumferential strain, almost in belt tension and provide a strong restraint to the core concrete. The tubes using strengthening mode B, are mainly subject to compression, which are likely to buckled under with a resulting strain along the longitudinal direction. (2) The middle part of the square tube for strengthening yielded until the ultimate strength, and the middle part of the circular tube yielded before the ultimate strength was achieved. Compared with the square steel tube, the circular tube can achieve better material efficiency of the steel, with larger axial strength and axial compression deformation value. (3) The longitudinal reinforcement and stirrup of the strengthened specimen yielded before the ultimate load. Then most of the reinforcements are in post-yield stiffening status, indicating that the test specimen obtained good ductility after enhancement.

3.1. Analysis Method on the Structural Strength

The experimental research indicates that the confining coefficient of a thin-walled circular steel tube wrapping a concrete column is very small, and shear failure occurs under axial compression. The thin-walled square steel tube concrete column failed by buckling.

In the calculation of the axial strength of the post-fire RC column strengthened using thin-wall tubes, the following assumptions are made:

- (1) The strengthened component still conforms to the Bernoulli hypothesis of flat sections.
- (2) The post-cast concrete, the original concrete after the surface treatment and the steel tube, can be effectively bonded together, and no interface slip occurs before the ultimate strength is approached.
- (3) The confining effect in the original column from the stirrup is small and negligible.
- (4) The concrete section of the original column after high thermal action is assumed to be an equivalent cross-section of uniform strength. The temperature field of the cross-section column subjected to fire is determined [13], and the uneven temperature field is simplified into an equivalent cross-section of uniform strength [14], as Equations 1 and 2.

$$f_{cur}(T) = f_{cu} (0 \leq T \leq 300^\circ\text{C}) \quad (1)$$

$$f_{cur}(T) = 0.5f_{cu} (300 \leq T \leq 800^\circ\text{C}) \quad (2)$$

In the Equation: f_{cur} —the cubic compressive strength of concrete after high thermal action; f_{cu} —the cubic compressive strength of concrete in air temperature.

- (5) The thin-walled steel tube is under a uniformly distributed load across the wall thickness, ignoring the radial direction stress along.

- (6) When the column reaches the ultimate axial strength, the lateral stress of the thin-walled steel tube is assumed to reach the yield strength f_y .

3.2. Post-Fire RC Column Stngthened by Thin-walled Circular Steel Tube

Assuming that when the specimen reaches the ultimate axial strength, the longitudinal stress of the wrapping steel tube is zero, and the hoop stress reaches the yield strength f_y of the steel. At this time, the wrapping steel tube provides maximum restraint on the core concrete.

Considering the influence of the high temperature on the strength of concrete and steel, and the enhancement of the core concrete by the thin-walled circular steel tube, the ultimate strength of the RC column N_{ucir} consists of the confined concrete strength and the longitudinal reinforcement strength. The equation as follows:

$$N_{ucir} = f_{cc}A_c + \sum_{i=1}^n f_{sri}A_{si} \quad (3)$$

where f_{cc} is the constrained concrete axial compressive strength [15]. According to the Willam–Warnke failure criterion [16] of the concrete under uniform lateral stresses, the equation of the concrete strength is given as

$$f_{cc} = f_{co} \left(-1.254 + 2.254 \sqrt{1 + 7.94 \frac{f_r}{f_{co}}} - 2 \frac{f_r}{f_{co}} \right) \quad (4)$$

where f_{co} is the axial compressive strength of unconstrained concrete; f_r is uniform pressure, i.e., the restraint stresses on the concrete; calculated following Equation 5

$$f_r = \frac{2tf_y}{D - 2t} \quad (5)$$

where f_y is the yield strength of the wrapping steel tube; D is the cross-sectional diameter of the strengthened specimen; t is the wall thickness of the wrapping steel tube; A_c is the cross-sectional area of the core concrete; A_{si} is the cross-sectional area of the No. i longitudinal reinforcement in the

original column; f_{sri} is the yield strength of No. i longitudinal reinforcement in the original column after exposed to fire, using the temperature field during the fire test, reduced according to the literature [17].

The axial compressive strength of the unconstrained concrete in the strengthened test specimen is taken as the combined strength of the concrete exposed to high temperature, and post-cast concrete, as Equation 6.

$$f_{co} = (0.5f_{c1}A_{c1} + f_{c2}A_{c2}) / (A_{c1} + A_{c2}), \quad (6)$$

where A_{c1} and f_{c1} denote the cross-sectional area of the post-fire concrete after removing damaged concrete outside from the longitudinal reinforcements and the concrete axial compressive strength after the high temperature, respectively; A_{c2} and f_{c2} denote the cross-section area of the newly poured concrete and its axis compressive strength, respectively.

The comparison between the test results N_t and estimate values N_u from Equation 3 for the axial strength of enhanced RC column is shown in Table 5. The calculated results match well with the test results, indicating that Equation 3 can be used as a reference for the strengthened post-fire specimen design.

Table 5. Comparison between experiments and computed results of RC columns with circular steel tubes.

Specimen	f_y/f_{co} (MPa)	$\sum_{i=1}^n f_{sri}A_{st}$ (kN)	N_t (kN)	N_u (kN)	N_u/N_t
S-C1	256.7/20.9	1034	4753	4777	1.005
S-C2	256.7/20.9	1034	4684	4777	1.020
C3	256.7/20.1	1100	5065	4751	0.938
C4	256.7/20.9	1100	4803	4845	1.009

3.3. Post-Fire RC Column Enhanced by Thin-Walled Square Steel Tube

The force model shown in Figure 17 [18] indicates that the square steel tube exerts non-uniform constraints on the core concrete, and can effectively restrain the corner and cross-section center of the concrete, but not the edge part between the corners. In Figure 17, b is the side length of the core concrete. The concrete within the range of $0.1b$ from the corner is the zone under boundary confinement, and the concrete in the range of the edge $0.8b$ is the unconstrained zone, and the curve of the unconstrained zone is a quarter circle. The shaded parts in the figure are unconstrained areas. The calculation based on the geometry indicates the effective confinement area of the core concrete is 63.5% of the total cross-sectional area, and the effective constraint coefficient of the defined square steel tube to the core concrete is

$$k_e = \frac{A_e}{A_c} \quad (7)$$

In the equation, A_e is the effective confined area of the core concrete; A_c is the cross-section geometry of the concrete.

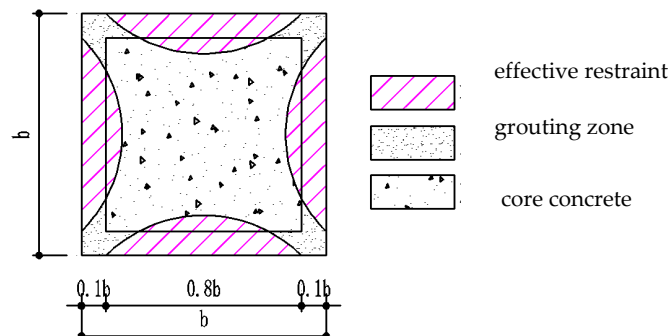


Figure 17. Constraint model of core concrete filled square steel tube.

The effective constraint stress of the square steel tube to the core concrete f_{r1} is obtained by the effective constraint coefficient:

$$f_{r1} = k_e f_{rsqu} = 0.635 f_{rsqu} \quad (8)$$

where f_{rsqu} is the restraint stress of the square steel tube on the core concrete, calculated following Equation 9.

$$f_{rsqu} = \frac{2t f_y}{b - 2t} \quad (9)$$

where f_y denotes the yield strength of the wrapping steel tube; b denotes the cross-section side length of the specimens; and t denotes the wall thickness of the wrapping steel tube.

According to William's five-parameter failure criterion of concrete under uniform lateral stresses, the core concrete strength inside the thin-walled square steel tube is given as

$$f_{cc} = f_{co} \left(-1.254 + 2.254 \sqrt{1 + 7.94 \frac{f_{r1}}{f_{co}}} - 2 \frac{f_{r1}}{f_{co}} \right) \quad (10)$$

The effective confined area of the core concrete is basically the cross-section of the specimen after removing the damaged surface, and the unconstrained area is the newly poured concrete section after strengthening. In order to simplify the calculation without compromising it significantly, the cross-section of the specimen removed from the damaged surface is defined as the effective constraint zone, and the newly poured concrete section is assumed to be the unconstrained zone. The axial strength N_{usqu} of the RC column wrapped square steel tube consists with the strength of confined concrete, unconstrained concrete, and the longitudinal reinforcement strength. Equation 11 is as follows:

$$N_{usqu} = f_{cc} A_{c1} + f_{c2} A_{c2} + \sum_{i=1}^n f_{sri} A_{si} \quad (11)$$

where f_{cc} denotes the axis compressive strength of the RC column enhanced using a square tube, according to the Mander model [15] (see Equation 4); The computed axial strength of the enhanced post-fire RC columns with square sections and the experimental results are shown in Table 6.

Table 6. Comparison between experiments and computed results of RC columns with square steel tubes.

Specimen	f_y/f_{co} (MPa)	$\sum_{i=1}^n f_{sri} A_{si}$ (kN)	N_t (kN)	N_{usqe} (kN)	N_{usqe}/N_t
SC3	250.0/14.8/32.6	1034	3500	3515	1.004
SC4	256.7/14.8/32.6	1034	3601	3729	1.036
SC5	250.0/14.8/32.6	1034	3651	3515	0.963
SC6	256.7/14.8/32.6	1034	3762	3729	0.991

3.4. Comparison and Validation

In China and other countries, there are many design standards [19–26] and research [27] on the axial strength calculation of concrete filled steel tubes.

In order to investigate whether the existing calculation methods of concrete filled steel tubes are suitable for the post-fire RC columns wrapping thin-walled steel tubes, in accordance with JCJ01-89 [19], CECS28:90 [20], DL/T5085-1999 [21], DBJ13-51-2003 [22], ACI318-08 [23], AIJ-1997 [24], EC 4-2004 [25], GJB 412-2000 [26] and other literature [27], the axial strength of the circular specimens and the square specimens were calculated, as shown in Table 7.

Table 7. Comparison between experiments and computed results based on design standards.

Specimen	Computed Results/Experiments									
	This Paper	JCJ [19]	CECS [20]	DL/T [21]	DBJ [22]	ACI [23]	AIJ [24]	EC [25]	GJB [26]	Ref [27]
S-C1	1.005	0.926	1.034	0.904	0.673	0.622	0.796	0.678	—	0.996
S-C2	1.020	0.940	1.050	0.918	0.683	0.631	0.808	0.688	—	1.011
C1	0.938	0.868	0.965	0.844	0.633	0.586	0.746	0.638	—	0.933
C2	1.009	0.930	1.038	0.909	0.680	0.629	0.802	0.685	—	1.000
SC3	1.004	1.139	1.285	—	1.075	0.796	0.981	0.860	1.102	1.115
SC4	1.036	1.265	1.404	—	1.045	0.773	0.954	0.836	1.071	1.083
SC5	0.963	1.092	1.231	—	1.030	0.763	0.941	0.824	1.056	1.069
SC6	0.991	1.211	1.344	—	1.082	0.800	0.971	0.876	1.112	1.179

In retrofit specifications, the strength calculation method of the CFST axial compression short column is used to calculate the axial strength of the post-fire RC column wrapped thin-walled steel tube. In JCJ01-89, the strength of columns with different slenderness ratios can be calculated by adjusting the stability factor of the axial compression column. In CECS28:90, the reduction factor of the slenderness ratio and the eccentricity of the component are taken into account, and the ferrule coefficient of the reinforced concrete is used to modify the calculated value. In DL/T5085-1999, DBJ13-51-2003, and GJB 412-2000, the design value combined with the steel tube concrete axial strength is used. In ACI318-08, the concrete-filled steel tube is simulated to equivalent reinforced concrete elements to calculate the structural strength. EC4-2004 is commonly used for the strength of the concrete wrapped by steel tubes of square, circle and rectangular shapes. The constraint from the circular steel tube onto core concrete is taken into account. The design code in both China and other countries are proposed mainly for ordinary concrete-infilled steel tubular columns. The strengthening elements on post-fire RC in this research are thin walled tubes. The enhanced column strength developed in this paper, was compared the design values calculated in accordance with ACI318-08 [23], then verified by the test results. The calculated results of DBJ13-51-2003 and AIJ-1997 agreed with the test results of average variations—5.8% and −3.8%, respectively. As shown in Table 7, the calculations following the Chinese Standards and ACI318-08 [23] seem higher than the test results because the formulas specified in the national codes are generally developed based on a consistent theory, considering the constraint effects of steel pipes on concrete. In the US code, Eurocode, and Japan code, superposition theory is generally adopted, and concrete strengthening from the steel tubes is not considered, therefore the calculated design resistance in accordance with the codes are lower than the experiment results.

4. Conclusions

The axial compression performance of reinforced concrete columns, their post-fire performance and strengthening technology using thin walled steel tubes were investigated based on tests. The main findings are summarized as follows:

1. The failure mode of post-fire RC columns enhanced using square thin-walled steel tubes is local buckling failure, and the enhanced circular column failed by shear. The test specimens appear to be “softened” after fire with decreased axial stiffness and strength. The strength and stiffness of post-fire RC columns enhanced using thin-wall steel tubes can be rehabilitated to the level of undamaged specimens, and the axial compression can be restored to or even exceed the ones of undamaged columns. Thin-walled steel tubes can effectively improve the ductility of the post-fire specimens.
2. The original cross-section geometry (square or circular) of the test specimen before enhancement has little effect on the final axial strength, which is more related to the final geometry. Both the axial strength and ductility of the specimens strengthened using the circular thin-walled steel tube are larger than the ones with the square steel tubes. Thickening the steel tube, would lead to a rise in the axial strength of the strengthened specimens.
3. The behaviors of steel tubes are greatly affected by the enhancement methods. If the steel tube only acts as a restraint, it is mainly subjected to circumferential strain in tension. It provides a

strong restraint on the core concrete. While the tube carries the longitudinal load together with concrete columns, it is mainly subject to compression, and likely to buckle along the longitudinal direction.

4. The practical calculation equations of the axial strength of post-fire RC columns enhanced by thin-walled steel tubes, including both square and circle sections, are proposed on the mechanism of composite structure. Besides the practical calculation equations, the effectiveness of axial compression equations and standards for concrete filled steel tubes are compared and analyzed. The practical calculation equations can be used to estimate the axial strength of post-fire RC columns enhanced by the thin-walled steel tubes.

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