

Article

Strengthening of RC Slabs with Symmetric Openings Using GFRP Composite Beams

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Abstract: This paper describes the results of experimental testing of glass fiber reinforced plastic (GFRP) composite beam strengthened reinforced concrete (RC) slabs with two symmetrical openings. Specimens, one-half scale, have been designed and fabricated to reflect the most common RC bathroom slab used in school buildings. The specimen had dimensions of 2000 mm (width) × 150 mm (thickness) × 3000 mm (length) were used with the two openings of 300 mm × 400 mm. The aim of this study is to investigate the most effective strengthening method using GFRP composite beams in slabs with openings for enhancing the load-carrying capacity and stiffness. Test results showed that the strengthened slabs seems to increase the load-carrying capacity by 29%, 21% and 12% over that of the control specimen for diagonal, parallel and surround strengthening respectively. Furthermore, test results showed that the diagonal-strengthened system is one of the most effective methods for strengthening an RC slab with openings in terms of load-carrying capacity, stiffness and crack patterns.

Keywords: RC slab; RC-strengthening; GFRP composite beam; crack pattern; failure mode

1. Introduction

In general, building codes are intended to protect people by preventing severe damages or collapse of that building in use. Furthermore, collapse or severe damages of critical buildings such as schools,

hospitals and public buildings may cause more devastating effects, with possible life loss. Seismic codes in buildings did not apply in Korea until 1988 since the country was considered to be located in a zone with low seismicity. Due to that fact, buildings constructed prior to 1988 in Korea cannot be considered safe, and need upgrading or strengthening of their structural elements. In particular, strengthening efforts of structural elements in school buildings that do not comply with current building codes have been made by the government or the local government in Korea since 2010 after the earthquake of Sichuan (2008) in China. During the strengthening of school reinforced concrete buildings, there were many cases when the strengthening of openings in RC slabs to accommodate new bathrooms, elevators or utility ducts were necessary. It is well known that creating openings in slabs can significantly decrease the load-carrying capacity of RC structure due to the discontinuity of structural elements including the cutting of both concrete and reinforcing steel [1,2]. The strengthening methods using fiber-reinforced polymer (FRP) composites have been widely used because FRP strengthening methods seem to have easy handling, lighter equipment for installation and less concern about corrosion in strengthened surface as compared to conventional methods such as steel jacketing method [3–5]. However, the performance or data of FRP-strengthened RC slabs with opening is rather limited although the performance of FRP-strengthened RC has received significant attention. Several analytical or experimental researches have reported on FRP-strengthened RC slab with openings [6–10]. Seliem *et al.* [11] reported the results of CFRP strengthened RC slab after having large openings cut out at the center of a slab in the positive moment region. In their investigation, three different strengthening types, externally bonded CFRP laminates, externally bonded CFRP laminates with CFRP anchors and near surface mounted (NSM) CFRP strips, were used for the evaluation of flexural capacity of slab. From the tests, it was found that the slab strengthened with NSM CFRP strips had a higher load-carrying capacity in comparison to the slab strengthened with externally bonded CFRP laminates if using significantly less area of FRP, and the use of externally bonded CFRP laminates slightly increased the flexural strength of the slab with opening while significantly enhancing its stiffness. Test results also showed that creating an opening in the slab reduces its strength by as much as 18% up to failure. Casadei *et al.* [12] presented the experimental results of one-way slabs with openings from an existing parking garage externally strengthened with CFRP laminates. In this study, a total of six one-way square slab specimens were used from an existing parking garage building as a test bed. Test results showed that the use of CFRP laminates in slab strengthening was effective with an increase in load by approximately 30%, and shear failure was found to be the controlling mechanism when cutouts were placed in the negative moment region of one-way slabs. Smith *et al.* [13] reported the experimental and analytical results of fiber reinforced polymer (FRP) strengthened one-way spanning reinforced concrete (RC) slabs with central cutouts for four wide slabs with cutouts and two narrow slabs without cutouts. Test results indicated that all FRP-strengthened slabs achieved a higher load-carrying capacity than their unstrengthened control counterparts. In addition, all strengthened slabs failed by debonding initiating at intermediate cracks (IC debonding) and in the case of the slabs with cutouts, the critical cracks were diagonal and originated from the corners of the cutout. Also, an analytical model was able to capture the different slab bending actions in addition to the debonding failure of the strengthened slabs. Enochsson *et al.* [14] showed the results of laboratory tests on 11 slabs with openings including six slabs strengthened with carbon fiber reinforced polymers (CFRPs) sheets. Strengthened slabs were compared with traditionally steel reinforced slabs and

without openings in slab. The results showed that slabs with openings can be strengthened with externally bonded CFRP sheets and the performance was even better than for traditionally steel reinforced slabs. Also, the numerical and analytical evaluations showed a good agreement with the experimental results.

This investigation provides additional data to the investigations conducted by the other researchers. Specimens, one-half scale, have been designed and fabricated to reflect the most common RC bathroom slab used in school buildings. Also, the investigation described in this paper supports the use of an FRP composite beam as a means of strengthening or repairing of RC slabs with openings, when RC slabs do not satisfy load-carrying capacity or current design codes requirements.

2. Experimental Program

The experimental program is focused on examining the structural performance of RC slabs strengthened with glass fiber reinforced plastic composite beam near openings. Four simply-supported RC slabs, one with no strengthening and three that were strengthened with the GFRP system, were carefully investigated as a measure of load-carrying capacity and stiffness.

2.1. Material

The following materials were used in this study: type I/II Portland cement which meets the Korean standards KS L5201 [15], 19 mm maximum size of crushed coarse aggregate with a specific gravity of 2.64, natural sand with a specific gravity of 2.56 and a fineness modulus of 2.42. The mix design of concrete was prepared with the following: ordinary Portland type I/II cement 410 kg/m³, aggregate 1629 kg/m³, water-cement ratio of 0.48. The used mix proportions were intended to have a normal weight and a target average compressive strength of 21 MPa at 28 days, respectively. For strengthening, commercially available glass fiber containing (GFRP) composite beam as shown in Figure 1 was used. Table 1 shows the mechanical properties of the GFRP composite beams provided by the manufacturer. Glass fiber containing composite beam has dimensions of 130 mm (W) × 12 mm (H).

Figure 1. Glass fiber reinforced plastic (GFRP) composite beam.

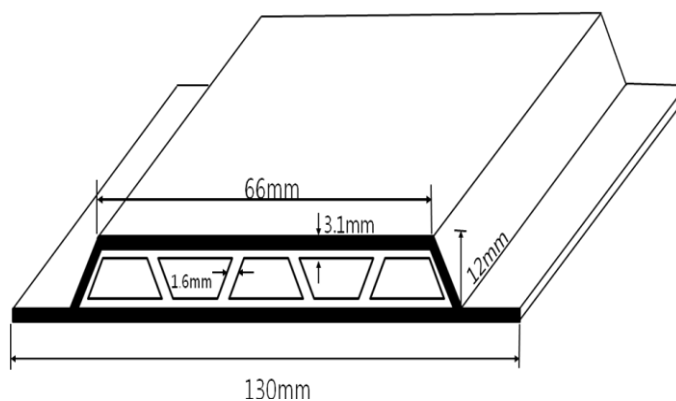
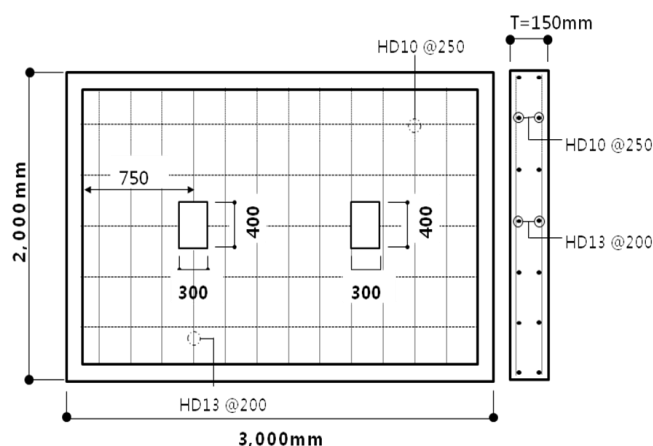


Table 1. Mechanical properties of the Glass fiber reinforced plastic (GFRP) composite beams.

Type	Unit	Property
Cross Section	mm ²	1312
Tensile Strength	MPa	418
Compressive Elastic Modulus	GPa	28
Ultimate Elongation	%	2.1

2.2. Slab Specimens

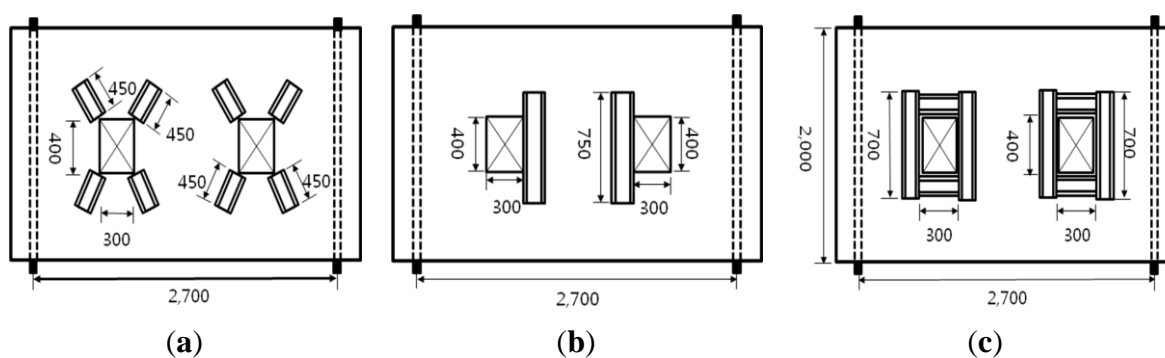
A total of four RC slabs having dimensions of 2000 mm (W) × 150 mm (T) × 3000 mm (L) were fabricated using the 28-day average concrete compressive strength of 23.4 MPa and the average concrete elastic modulus of 25.63 GPa. The average 28-day concrete compressive strength was obtained from the four standard cylinder specimens (150 mm × 300 mm). The slab dimensions were chosen based on one-half scale of the typical bathroom slabs of school buildings. Each slab has two symmetric openings of the same size, with dimensions of 300 mm (W) × 400 mm (L), as shown in Figure 2. The openings were created with a concrete saw-cutting machine, and then four cut-edges were smoothed out by 100 mm thickness of cement mortar. Actually, these openings were intended for use in bathroom installations or utility purposes in the existing building slabs. Each slab had 12 and 8 HD (high tension deformed) 10mm bars as bottom and top reinforcement in longitudinal and transverse directions, respectively. Details of slab reinforcement are given in Figure 2. All steel bars used in the slabs have yield strength of 400 MPa. Before bonding the GFRP composite beams around the openings, the slab surface was first sandblasted, cleaned and dried, and the epoxy was uniformly applied. The properties of the used epoxy resin have tensile strength of 44.9 MPa, flexural strength of 97.0 MPa and density of 1.21 g/cc. To improve the bond between the GFRP and concrete in this study, commercially available power pins were used in the lips of GFRP composite beams. The application of GFRP composite beams was undertaken in accordance with the requirements of the manufacturer of the GFRP composite system. All the strengthened slabs were tested approximately 90 days after casting, and two weeks after applying the GFRP composite beams.

Figure 2. Details of reinforcement.

In this study, four different strengthening configurations of GFRP composite beam were used for comparison of their effectiveness. Details of strengthened slabs are shown in Figure 3 with the

positioning of the FRP composite beams around the openings. The strengthening configurations used in this study were chosen based both on economic considerations and technical aspects of engineering. Also, these types of strengthening were selected to examine the potential use of GFRP composite beams to enhance the load-carrying capacity of RC slabs with openings. The first slab (S_U) without FRP strengthening served as the control slab. The other three slabs (S_D , S_P and S_S) were strengthened with FRP composite beams in diagonal, parallel and surround at the near openings respectively.

Figure 3. Strengthened configurations of (a) diagonal; (b) parallel; (c) surrounding slabs with openings.



2.3. Test Setup and Procedure

In this experiment, each slab specimen was tested under simply-supported conditions with a clear span of 2.7 m. All slab specimens were subjected to monotonic compressive loading using a 500 kN of Material Testing System (MTS) actuator (MTS, Eden Prairie, MN, USA) with maximum stroke of 150 mm. The load was applied at the center of the specimen at a rate of 2.0 mm/min. Also the midspan displacement of each specimen was measured using linear variable differential transformer (LVDT). Furthermore, in order to trace the crack development and propagation, two people were placed directly under the slab during tests to mark up the crack propagations as they appeared at the bottom of the slabs with a frequency of every 1 kN of the load changed, as shown in Figure 4a. The configurations of the slab tests are shown in Figure 4. First, unstrengthened slab was tested to failure to investigate the load-carrying capacity, crack pattern and stiffness as a control specimen.

Figure 4. Test setups of (a) unstrengthening; (b) surround strengthening; (c) diagonal strengthening; (d) parallel strengthening for the various strengthening configuration.



Figure 4. Cont.



3. Test Results and Discussions

3.1. Load-Deflection

The measured maximum load and the corresponding deflections are given in Table 2. The maximum load of the control specimen was 71.91 kN. The maximum loads of all tested slabs were 92.59 kN, 86.80 kN and 80.39 kN for diagonal, parallel and surround strengthening respectively. Therefore, the strengthened slabs for diagonal, parallel and surround strengthening seem to raise the load-carrying capacity by 29%, 21% and 12% over that of the control specimen, respectively. The use of the GFRP composite beam has relatively increased the load-carrying capacity of slab because of the rigidity of the GFRP composite beams. However, the strengthening slabs with FRP system reduced its deflection at the maximum load by 28%, 13% and 21% over that of the control specimen for diagonal, parallel and surround strengthening respectively. The load-carrying capacity of strengthened slabs from this investigation was similar to that found in the one-way slabs with openings strengthened with CFRP laminates by Casadei *et al.* [12].

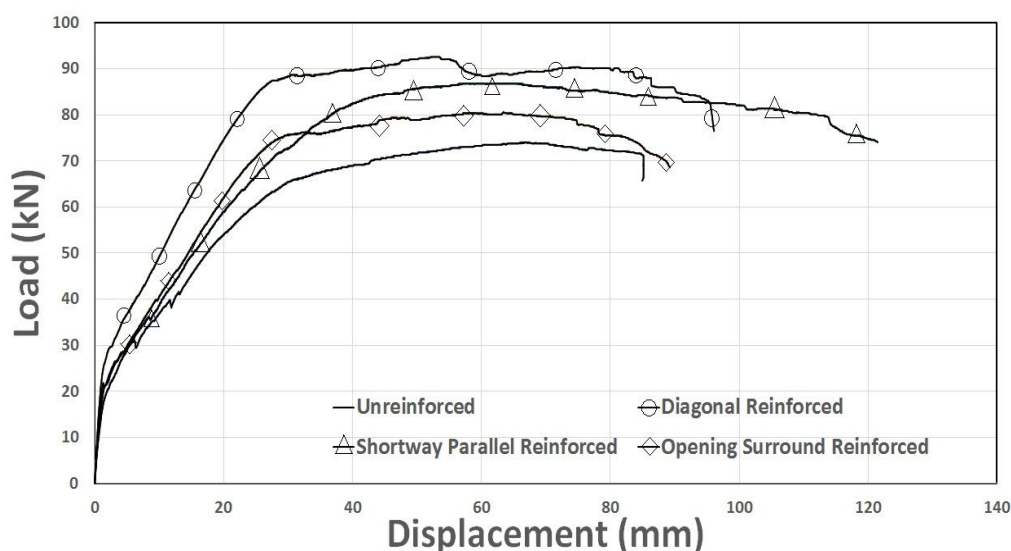
Table 2. Test results of loads and deflection.

Specimen	Maximum Load (kN)	Deflection at the Maximum Load (mm)	First Cracking Load (kN)
S _U	71.91	69.05	22
S _D	92.59	52.16	25
S _P	86.8	62.4	19
S _S	80.39	57.18	22

The observed load-deflection curves up to failure are shown in Figure 5. The linear relationship between load and deflection can be observed up to the first cracks which opened at the bottom of slabs. The relationship continues nearly linearly until deflection increases without a substantial increase of load. At this point, de-bonding was initiated at the one of corners of opening between GFRP composite beam and concrete interface. Under this condition, it was found that the tension steel reinforcement already yielded and major cracks became wider. Test results indicated that the strengthened slabs may have an increased stiffness of 15.30%, 18.6% and 9.87% over that of the control specimen for diagonal, parallel and surround strengthening respectively. Also, all the strengthened slabs exhibited higher ductility than the control specimen, sometimes as much as 23%. This type of behavior is caused

by the gradual steel yielding in the slab and it reached failure or strengthening FRP still had the ability to mitigate sudden failure of slabs. However, the test results by Smith [13] reported a loss of ductility when FRP system was added in RC slabs. This contrary result may come from the different span length and loading type for the test. Thus, it is necessary to verify such a controversial result through future research. Also, the initial stiffness of strengthened slabs, which is represented by a slope on the graph up to the moment when the first crack occurred, was higher for all FRP strengthened slabs than the control slab. This means that the use of the GFRP composite beam increased the stiffness of slabs with openings due to the rigidity of the GFRP composite beam itself. From the test results, it can be concluded that diagonally-strengthened slab is one of the most effective strengthening methods for RC slab with openings in terms of load-carrying capacity, stiffness and crack patterns.

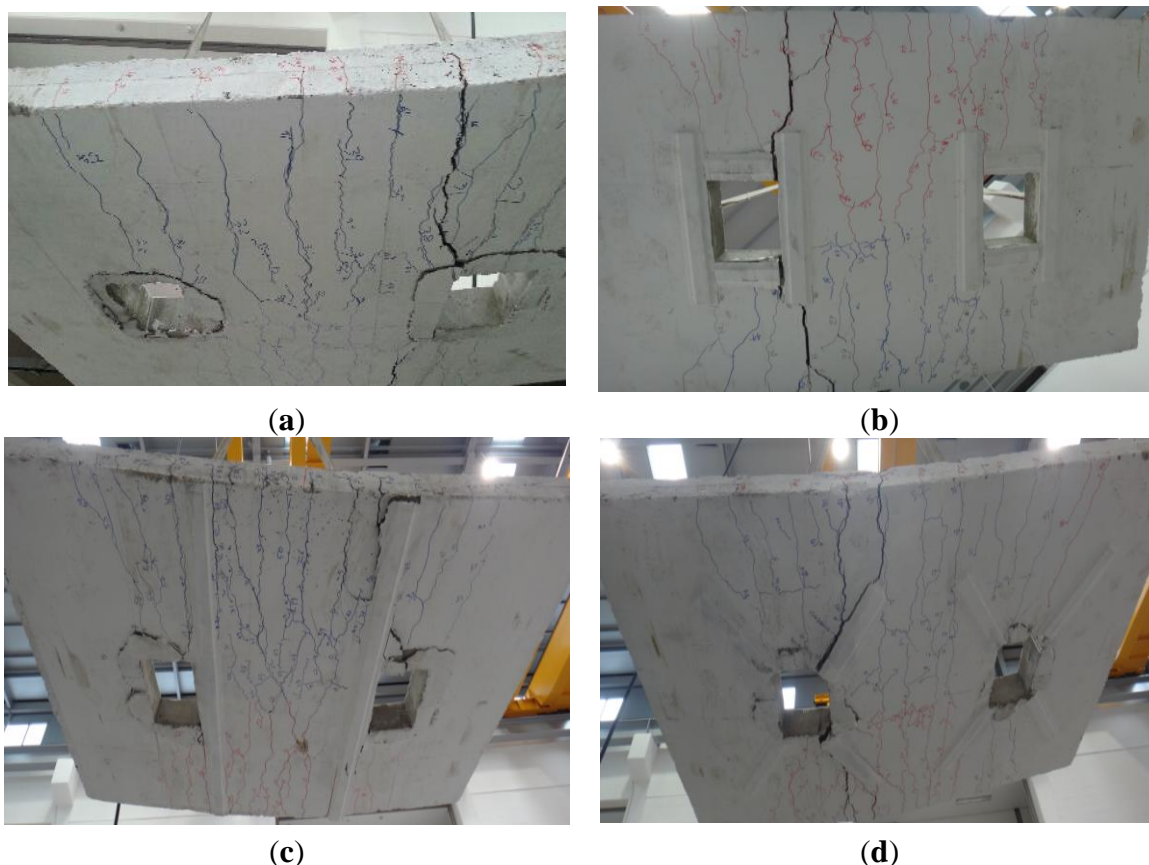
Figure 5. Load-deflection curves.



3.2. Crack Patterns and Modes of Failure

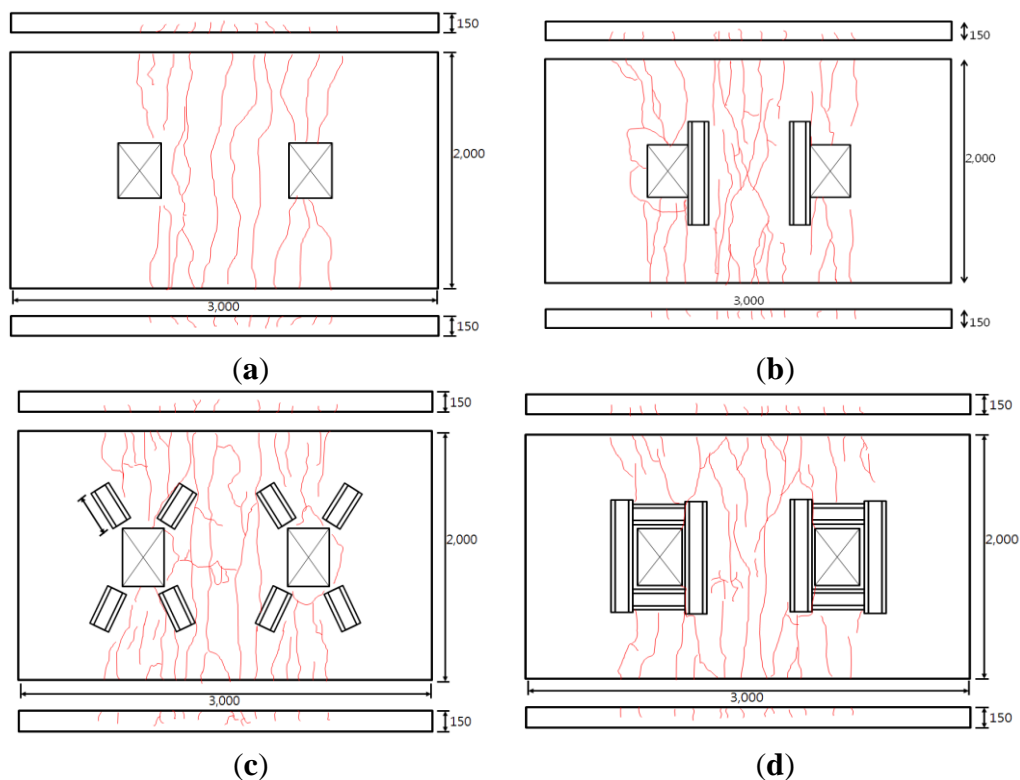
Figure 6 shows the final failure condition of the tested slabs. It was observed that the slab strengthened with GFRP composite beams failed by flexure due to the intermediate cracks between the two openings. As shown in Figure 6, all tested slabs had ultimately failed by debonding of GFRP composite beam with a wide-open crack, which originated from the corners of the opening and propagated towards the short length of the slab. During the tests, an almost identical failure sequence was observed in all strengthened slabs as compared to the control slab. Also, crack propagation was increased gradually until the load approached almost its maximum, and after that point, crack width became wider.

Figure 6. Failure Modes of (a) unstrengthened; (b) surround strengthening; (c) parallel strengthening; (d) diagonal strengthening.



The detailed schematic final crack patterns are given in Figure 7. For the control specimen, first cracks were detected in the midspan region at the load of 22 kN into the transverse direction. At the load of 65 kN, the main transverse cracks reached approximately 0.5–1.0 mm in width for the control slab. The first cracks of the strengthened slabs also detected in the midspan region at the loads of 25, 19 and 22 kN for diagonal, parallel and surround strengthening, respectively. The main transverse cracks of the strengthened slabs reached the width of approximately 0.3–0.4 mm at an average load of approximately 65 kN. Unexpectedly, the slight differences between the control and GFRP composite beam strengthened slabs for the first cracking load were probably due to the relatively long span or the loading configuration used in this investigation. Diagonal cracks were also detected from the near openings of the strengthened slab as the load increased. Compared to the control slab, the strengthened slabs showed more and longer diagonal cracks. Also, there were no significant differences in crack pattern and the extent of cracks between different strengthening types in this study. A similar result was found in the result of one-way spanning RC slabs with cutouts using FRP composites by Smith *et al.* [13]. Furthermore, strengthening the slab with FRP system resulted in a multi-direction crack mechanism than that of the control specimen. This result may indicate that crack propagation in RC slabs could be controlled with FRP strengthening technique for a particular case when required.

Figure 7. Crack patterns of (a) unstrengthened; (b) parallel strengthening; (c) diagonal strengthening; (d) surround strengthening.



4. Conclusions

This paper described the results of RC slabs with two openings strengthened with GFRP composite beam. One control slab without FRP strengthening and three slabs with FRP strengthening were investigated for their effectiveness. Based on the test results, it can be concluded that the investigated strengthening types can be used for strengthening or upgrades of structural capacity of existing RC slabs with openings, since all strengthened slabs seem to raise the load-carrying capacity approximately by an average of 20%. Also, diagonally-strengthened slab showed to be one of the most effective ways for load-carrying capacity, stiffness and crack patterns. It was also observed that the slab strengthened with GFRP composite beams failed by flexure due to the intermediate cracks and debonding of GFRP composite beam with a wide-open crack. Also, there were no significant differences in crack patterns and the extent of cracks between different strengthening types in this study.

Acknowledgments

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Conflicts of Interest

The authors declare no conflict of interest.

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